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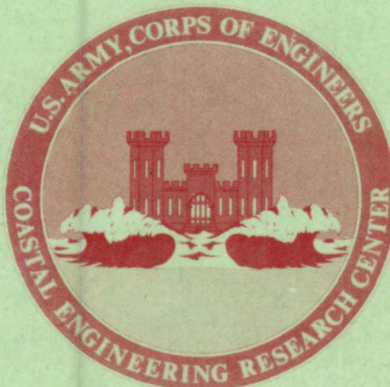
TP 77-13

**Development of Surge II Program With  
Application to the Sabine-Calcasieu Area  
for  
Hurricane Carla and Design Hurricanes**

by

**Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid**

**TECHNICAL PAPER NO. 77-13  
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**Prepared for  
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Design hurricane	Inland flooding model							
Hurricane Carla	Sabine-Calcasieu region							
Hurricane surge model	Surge II program							
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) SURGE II is a program for calculation of storm surges and tides in a bay or estuary of the type where frictional resistance dominates over Coriolis force. It includes the provision for subgrid scale barriers and channels as well as allowing for overtopping of barriers and flooding of and recession from normally dry regions adjoining the bay or estuary. The theory and numerical algorithm is discussed in detail. A user's guide for the program is also provided. Application of the program, in respect to astronomical tides and (continued)								

hurricane surges, is made for the Sabine-Calcasieu region which straddles the Texas and Louisiana boundary. For normal tide conditions, cities such as Beaumont, Orange, and Lake Charles are connected to the sea via rivers, which in the numerical model must be represented as subgrid scale channels as long as the basic grid scale is of the order of a nautical mile. Under hurricane surge conditions, however, the overland flooding can greatly expand their connection to the sea.

Calibration of channel friction is carried out via the astronomical tide simulation. Calibration of the block friction is carried out using data on a previous storm of record, Hurricane Carla. An example application is provided for standard project hurricanes (SPH). The response for a large radius SPH of slow speed and one of moderate speed of translation is examined. Also, the effect of rainfall is examined by running the latter storm with and without rainfall.

## PREFACE

This report is published to assist coastal engineers in the study of storm surge and inland flooding for use in the planning and design of protective coastal works. The work was carried out under the coastal processes program of the U.S. Army Coastal Engineering Research Center (CERC).

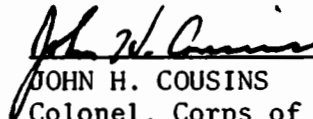
The report was prepared by Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid, Coastal Studies, Inc., who are also on the staff of the Department of Oceanography, Texas A & M University, College Station, Texas, under CERC Contract No. DACW64-74-C-0015 to the U.S. Army Engineer District, Galveston.

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G. Marinos was the Galveston District contract monitor for the report under the general supervision of S. Tanner, Chief, Coastal Planning Section. Dr. Jon Hubertz was the CERC technical monitor of the report under the general supervision of Dr. D.L. Harris, Chief, Coastal Oceanography Branch, Research Division.

Comments on this publication are invited.

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JOHN H. COUSINS  
Colonel, Corps of Engineers  
Commander and Director

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CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI)  
UNITS OF MEASUREMENT

U.S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	by	To obtain
inches	25.4	millimeters
	2.54	centimeters
square inches	6.452	square centimeters
cubic inches	16.39	cubic centimeters
feet	30.48	centimeters
	0.3048	meters
square feet	0.0929	square meters
cubic feet	0.0283	cubic meters
yards	0.9144	meters
square yards	0.836	square meters
cubic yards	0.7646	cubic meters
miles	1.6093	kilometers
square miles	259.0	hectares
knots	1.8532	kilometers per hour
acres	0.4047	hectares
foot-pounds	1.3558	newton meters
millibars	$1.0197 \times 10^{-3}$	kilograms per square centimeter
ounces	28.35	grams
pounds	453.6	grams
	0.4536	kilograms
ton, long	1.0160	metric tons
ton, short	0.9072	metric tons
degrees (angle)	0.1745	radians
Fahrenheit degrees	5/9	Celsius degrees or Kelvins <sup>1</sup>

<sup>1</sup>To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use formula:  $C = (5/9) (F - 32)$ .

To obtain Kelvin (K) readings, use formula:  $K = (5/9) (F - 32) + 273.15$ .

## SYMBOLS AND DEFINITIONS

A	cross-sectional area of a channel
$A_b$	effective surface area of a block
$A_c$	cross-sectional area of a channel
$A_s$	surface area of an estuary at MSL
$a_0$	amplitude of input tide to an estuary
B	$8/3\pi m(A_s\omega)^2 a_0$ , a parameter which determines the phase lag of tidal response in an estuary
BN	right-hand side of equation (48)
BP	right-hand side of equation (46)
b	$(\partial A/\partial s)_H$ const., a characteristic of a channel
$C_d$	dimensionless discharge coefficient characterizing a constricted opening between bay and sea
$C_g$	admittance coefficient (with dimensions of velocity); nominally represents the wave speed in the sea
$C_o$	dimensionless overflow coefficient (generally less than 0.5 for a broad-crested barrier)
$C_s$	dimensionless discharge coefficient for a submerged barrier (generally less than $\sqrt{2}$ )
D	total depth of water at position $x,y$ at time $t$
$\bar{D}$	a mean depth for the effective fetch across a block; also mean depth for a channel $(D_N + D_p)/2$
$D_b$	depth of water over the crest of a barrier
$D_c$	effective depth of a channel $A_c/w$
$D_{max}$	maximum depth to be expected anywhere in the system during a storm surge
$F_L$	contribution to the forcing term in equation (17) due to lateral transfer of mass and momentum
f	dimensionless bed resistance coefficient for blocks
$f_c$	channel bed friction coefficient

SYMBOLS AND DEFINITIONS--Continued

G	damping factor for channels, see equation (44)
$G_1$	damping factor for x-transport on blocks, see equation (35)
$G_2$	damping factor for y-transport on blocks, see equation (36)
g	acceleration due to gravity
H	water level elevation relative to local MSL datum
HB	water elevation on the water-connected block of a channel
HC	common water elevation for a channel junction
HM	mean water level anomaly of connected channel and blocks
HX	water level at the lower end of an x-channel
HY	water level at the left end of a y-channel
$H_A$	H at point B in a channel
$H_b$	water level on the high side of a barrier
$H_g$	input tide level at time t outside a bay entrance
$H(i,j)$	water level anomaly H for block identified by x and y indexes i,j
$H^*$	tentative predicted H for a ponding block in the absence of any contribution by longitudinal discharge to or from the channel which terminates adjacent to that block
$H'$	value of H at new time level
$H'_p$	new H value at point P in channel
$H_1$ & $H_2$	water levels on the two sides of a barrier (both of which exceed $Z_b$ ), equation (10)
i	x-index for grid blocks
j	y-index for grid blocks
K	dimensionless wind-stress coefficient, equation (6)
L	effective fetch length

SYMBOLS AND DEFINITIONS--Continued

$L_f$	net time rate of gain of water volume per unit distance along the channel by lateral transfer and rainfall
$L_m$	net time rate of gain of momentum (divided by water density) per unit distance along channel
$m$	$fL/gD_cA_c^2$ or $1/g(C_dA_d)^2$
$N$	denotes negative characteristic
$n$	time index
$P$	wind "push" term $X\Delta t$ or $Y\Delta t$ ; also denotes positive characteristic
$Q$	volume transport through cross-sectional area of a channel
$\bar{Q}$	mean $Q$ value for channel, equation (45)
$QCXP_K$	flow at the upper end of an x-channel for channel block $K$
$QCYN_K$	flow at the left end of a y-channel for channel block $K$
$QCYP_K$	flow at the right end of a y-channel for channel block $K$
$QCXN_K$	flow at the lower end of an x-channel for channel block $K$
$Q_A$	$Q$ at point $A$ of positive characteristic
$Q_B$	$Q$ at point $B$ of negative characteristic
$Q_d$	discharge from channel to ponding block
$q_f$	the flow (per unit length of channel) from the channel to the adjacent block
$q_i$	lateral volume flux per unit length into the channel
$q_n$	outward component of volume flux at a boundary
$q_o$	lateral volume flux per unit length out of the channel
$q_t$	flow (per unit length of channel) from the channel block to the channel (across the interior side of the channel)
$Q'$	new $Q$ value
$Q'_N$	new $Q$ at point $N$

SYMBOLS AND DEFINITIONS--Continued

$Q_p$	new Q at point P
$Q'_r$	specified river discharge
R	rainfall rate
$R(i,j)$	rainfall rate for block i,j
r	relative amplitude response
s	distance along the axis of a channel
T	tidal period
$T_s$	longitudinal component of wind stress (divided by water density)
	or
	appropriate wind-stress component (X or Y) corresponding to time level t for the associated channel block
t	time
U	vertically integrated x-component of volume transport per unit width
UCF(K)	lateral transport, per unit width per unit time, nominally from an x-channel of block K to an adjacent block; also denoted $UCF_K$
UCT(K)	lateral transport, per unit width per unit time, nominally to an x-channel from the interior of block I; also denoted $UCT_K$
UN	U value on left side of block
$U(i,j)$	value of U at the left side of block i,j
$U(i+1,j)$	value of U at the right side of block i,j
u	typical fluid speed in the bay
U'	value of U at new time level
V	vertically integrated y component of volume transport per unit width
VCF(K)	lateral transport per unit width per unit time, nominally from an y-channel of block K to an adjacent block; also denoted $VCF_K$



SYMBOLS AND DEFINITIONS--Continued

VCT(K)	lateral transport per unit width per unit time, nominally to an y-channel from the interior of block K; also denoted $VCT_K$
$VN_I$	value of V at the lower side of a block
$V(i,j)$	value of V at the lower side of block i,j
$V(i,j+1)$	value of V at the upper side of block i,j
$V'$	value of V at new time level
W	windspeed at 10-meter elevation over the water
$W_c$	a critical speed taken as 14 knots (7 meters per second)
w	surface width of a channel (conveyance width)
X	x-component of the wind stress divided by the density of the water
$X(i+1,j)$	value of X for right side of block i,j
x	horizontal Cartesian coordinate nominally alongshore, positive to the right when facing shore
Y	y-component of the wind stress divided by the density of the water
$Y(i,j+1)$	value of Y for top side of block i,j
y	horizontal Cartesian coordinate nominally normal to shore, positive landward
Z	elevation of the seabed relative to MSL datum
$Z(i,j)$	value of Z for block i,j
$Z_b$	barrier crest elevation
$Z_c$	channel bed elevation
$\alpha$	$(gD)^{1/2} \Delta t / \Delta s$ (Courant number); also $L_c / D_c A_c$ , equation (77)
$\Gamma$	$L(C_b D_b)^2 / \bar{D} \Delta t$
$\Delta H$	a head differential dependent upon barrier type
$\Delta q$	net lateral flow to the channel per unit length of channel

SYMBOLS AND DEFINITIONS--Continued

$\Delta s$	grid size for blocks (distance between successive H values in both the x and y directions); also written $\Delta S$ or DELS
$\Delta t$	time step (time interval between successive H values at given location); also written DELT
$\theta$	the angle between the wind velocity vector and the x-axis
$\lambda$	$w (g\bar{D})^{1/2}/G$
$\pi$	3.14159 ...
$\sigma$	$wf Q /A^2$
$\phi$	latitude
$\Omega$	absolute angular speed of the earth
$\omega$	radian frequency $2\pi/T$

# DEVELOPMENT OF SURGE II PROGRAM WITH APPLICATION TO THE SABINE-CALCASIEU AREA FOR HURRICANE CARLA AND DESIGN HURRICANES

by  
Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid

## I. INTRODUCTION

Numerical techniques for the solution of equations representing storm surges in coastal areas were significantly augmented in 1966 by the development of a two-dimensional model (referred to in this study as the SURGE I program) for the U.S. Army Engineer District, Galveston (Reid and Bodine, 1968). At about the same time a number of bay models emerged. Notable among these are the models of Leenderste (1967) and Masch, et al. (1969), which have been applied to problems of both surge and circulation in bays. These models include the Coriolis force which is neglected in the Reid-Bodine model. However, the Reid-Bodine model produced the first successful inclusion of flooding, recession, barriers, and flow over barriers in the study of inundation of low-lying coasts. The actual model is a nonlinear system of equations and boundary conditions solved by numerical integration of time-dependent, forced motion. Its use produces the water response to stormwinds over the region for a given storm tide at the seaward boundary. The initial application was a hindcast of the Hurricane Carla surge generated in Galveston Bay during 9 to 12 September 1961.

During Hurricane Carla, the wetted perimeter of Galveston Bay essentially doubled, as accurately reproduced in the hindcast computations. Serial observations of water levels for the storm period available from stations throughout the bay were compared to levels computed with the numerical algorithm. These records produced a standard deviation of less than 4 inches, overall. The maximum deviation of the water level prediction was 1.5 feet and occurred at the grid square corresponding to the location of the Pelican Island Bridge which spans the channel between Galveston and the Pelican Islands. Although this disparity was relatively large, its effect on the computations was effectively reduced by the smoothing operation of the numerical integration. However, this difference points out a basic problem confronting any model--the minimum definition of topographic features.

The basic problem of indicating subgrid scale effects in numerical modeling is normally solved by parameterization of the omitted physical mechanism. Often, an analytic relationship is introduced that requires the specification of empirically derived constants; e.g., the wind-stress equation for the transfer of momentum from wind to water. Another simple and pertinent instance is the *a priori* rotation of wind vectors over certain grid squares in the Hurricane Carla computations for Galveston Bay. The model Galveston entrance channel was not in the proper orientation on the Cartesian numerical grid system and, as a result, did not admit a realistic amount of water to the bay. A programmed shift in the wind vectors indicated this subgrid scale feature.

SURGE I has been applied to the study of Texas coastline surge susceptibility. The topographic features of this region are characterized by barrier islands and shallow, river-fed bay systems surrounded by near sea level land and marshes. The specific applications of the program have therefore centered interest on the immediate environs of a bay. The requirement for surge studies of appreciable distances inland from the bay system has only recently been placed on the numerical model. The propagation of the surge to higher ground through necessary subgrid scale topographic features has required an extension of the basic algorithm.

The new algorithm developed for the study of the Sabine-Calcasieu region is referred to as the SURGE II program. This program incorporates all the features of SURGE I with the further option of representing variable depth and width channels along the sides of each grid square. The flow computations for the channels interact with the normal grid square computations and permit a complete suite of flooding conditions for overtopping of levees. In this manner SURGE II provides a time-dependent, subgrid scale transport of water through the model.

## II. THEORETICAL DEVELOPMENT FOR SURGE II

### 1. Summary of Two-Dimensional Theory.

The development of SURGE II was based on the SURGE I concept by Reid and Bodine (1968). A part of this study is presented here to provide a complete description of SURGE II.

The advection of momentum (or field acceleration) is considered negligible except at singular regions of the bay (submerged barriers and narrow channels) where the effect is included implicitly through the use of appropriate nonlinear discharge relations. The effect of the earth's rotation is also neglected; this approximation appears justifiable for systems of small spatial scale and shallow depth where frictional forces are more dominant.

Within the normal domain of the bay and immediate adjoining sea, the vertically integrated equations of motion and of continuity appropriate to the problem are taken as follows:

$$\frac{\partial U}{\partial t} + gD \frac{\partial H}{\partial x} = X - fqUD^{-2} \quad (1)$$

$$\frac{\partial V}{\partial t} + gD \frac{\partial H}{\partial y} = Y - fqVD^{-2} \quad (2)$$

$$\frac{\partial H}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = R, \quad (3)$$

where

x and y = horizontal Cartesian coordinates;

t = time;

U and V = vertically integrated x and y components, respectively, of transport per unit width;

g = gravity;

H = water level elevation relative to the local mean sea level (MSL) datum;

D = depth of water at position x, y at time t;

q = magnitude of the transport per unit width;

f = dimensionless bed-resistance coefficient;

R = rainfall rate;

X and Y = x and y components of the wind stress divided by the density of the water (the density assumed constant).

Normal values of f are in the range  $10^{-3}$  to  $10^{-2}$  for typical seabed conditions.

The value of q is obtained from U and V by

$$q = (U^2 + V^2)^{\frac{1}{2}} \quad (4)$$

which is a positive quantity.

The kinematic forms of the wind-stress components in the absence of rainfall are taken as

$$\begin{aligned} X &= K W^2 \cos \theta \\ Y &= K W^2 \sin \theta, \end{aligned} \quad (5)$$

where W is the windspeed at a 10-meter elevation over the water, and  $\theta$  is the angle between the wind velocity vector and the x-axis. The dimensionless coefficient, K, used in the calculations is presumed to be a function of windspeed as implied by the van Dorn (1953) relation for wind stress. Specifically, it is assumed that

$$\begin{aligned} K &= K_1 && \text{for } W \leq W_c \\ K &= K_1 + K_2 \left(1 - \frac{W_c}{W}\right)^2 && \text{for } W \geq W_c, \end{aligned} \quad (6)$$

where the constants  $K_1$  and  $K_2$  are taken as  $1.2 \times 10^{-6}$  and  $1.8 \times 10^{-6}$ , respectively, and  $W_c$  is a critical speed which is taken as 14 knots (7 meters per second). For large windspeeds,  $K$  approaches the limiting value of  $3.6 \times 10^{-6}$  which corresponds to a resistance coefficient of about  $3.0 \times 10^{-3}$  if the ratio of air density to water density is taken as  $1.2 \times 10^{-3}$ .

In the presence of rainfall an added flux of momentum proportional to  $RW$  occurs (van Dorn, 1953). The effect can be included by augmenting  $K$  by  $R/W$ . For heavy rainfall, the resulting  $K$  is increased about 10 per cent.

The variables  $H$  and  $D$  are related by the simple expression,

$$D = H - Z , \quad (7)$$

where  $Z$  is the elevation of the seabed relative to the MSL datum. Presumably,  $Z$  is a function of  $x$  and  $y$  only; i.e., the time-dependent scour of the seabed is ignored.

The above equations ignore the direct effect of variable atmospheric pressure which is relatively minor in a small, shallow bay. The effect over the sea is included implicitly through the specification of an appropriate surge height versus time in the adjoining sea where the combined effects of winds and differential atmospheric pressure give rise to a coastal storm surge. This is presumed to be determined independently of the detailed calculations for the bay and enters as a boundary condition.

a. Boundary Conditions. Four different types of boundary conditions are used in this system of computations. Two of these conditions apply to the water-land boundary, one condition applies to the artificial boundary representing the seaward end of the bay system, and one applies at partial barriers internal to the system. (Additional internal conditions are needed in the presence of imbedded channels as discussed later in Section III,2.) All four conditions relate the normal component of flow at the boundary to the state of the water level at the boundary.

In general, the boundary between bay water and land depends on the water elevation and the land topography. The shoreline for different uniform elevations of the surface of the bay is readily established from a knowledge of the topography. For a bay with low-lying terrain, the rate of increase of surface area of water per unit increase of water level can be considerable. In the actual rising stage of storm tide the amount of inundation is controlled by the rate at which the water can flow into the potential ponding areas. In the present scheme, which uses a representation of the bay in terms of a discrete grid, the elevation of the seabed or land is regarded as uniform over each grid square, thus forming a two-dimensional, stairstep-type approximation of the actual topography. The boundary condition on the normal component of flow,  $q_n$ , at the juncture of a flooded square and a dry square is taken as

$$q_n = 0, \quad (8)$$

if the elevation,  $H$ , of the water is less than that of the adjacent dryland. However, if the water level is greater than that of the dryland, then the rate of flooding,  $q_n$ , per unit length of land barrier, is given by

$$q_n = \pm C_o D_b (g D_b)^{1/2}, \quad (9)$$

where  $D_b$  is water depth over the crest of the barrier, and  $C_o$  is an appropriate dimensionless overflow coefficient, generally less than 0.5 for a broad-crested barrier. The choice of sign depends on whether the flooding is from bay to land or from flooded land back to the bay during the recession stage.

Equation (9) is considered valid for any barrier within or at the boundary of the system for which the water level on one side of the barrier is greater than the barrier crest elevation,  $Z_b$ , and for which the water level on the other side is less than  $Z_b$ . Moreover,  $D_b$  is simply  $H_b - Z_b$ , where  $H_b$  is the water level on the high side.

In the case where the water level on both sides of an internal barrier exceeds the barrier-crest elevation, the discharge is taken as that for a submerged wier,

$$q_n = \pm C_s D_b (g |H_1 - H_2|)^{1/2}, \quad (10)$$

where  $D_b$  is the water depth over the crest of the barrier,  $H_1$  and  $H_2$  are the water levels on the two sides of the barrier (both of which exceed  $Z_b$ ), and  $C_s$  is an appropriate dimensionless discharge coefficient for the submerged barrier (generally less than  $\sqrt{2}$ ). In this case,  $D_b$  is taken as  $(H_1 + H_2)/2 - Z_b$ . Again, the sign is taken such that the flow is directed toward the low-head side of the barrier. Both equations (9) and (10) presume that the velocity of approach to the barrier is much less than the velocity over the barrier.

In the numerical computational scheme, emphasis is placed on the evaluation of flow and water levels within a bay which is connected to a sea of essentially unlimited extent. An appropriate boundary condition is required either at the mouth of the bay system or along some line within the sea which delineates the outer limit of the computational grid. The correct approach would be to treat the development of the surge in the sea and bay as a single problem. However, the difference in spatial resolution required for the two different regions of the system, as well as computer storage limitations, makes this impractical. The assumption is made that the effect of the conditions in the bay has only a minor influence on the development of the surge in the sea and over the Continental Shelf. The evaluation of the latter can be determined independently of the bay problem or obtained from observation and used as an outer boundary condition for the bay.

The simplest condition at the seaward boundary is of the form

$$H = H_g , \quad (11)$$

where  $H_g$  is the prescribed water level which would exist in the absence of the bay at time  $t$  at the outer boundary of the bay system. SURGE II presently uses this condition at the seaward boundary and at lateral boundaries on the limited shelf part of the system. An alternative condition for the lateral boundaries on the shelf is to prescribe that  $\partial U/\partial x = 0$  at these boundaries where  $x$  is taken alongshore (Jelesnianski, 1966, 1967). An alternative condition for the seaward boundary is one which allows for radiation of energy to the sea. The latter condition is of the form

$$H = H_g + q_n/C_g , \quad (12)$$

where  $q_n$  is taken positive outwards from the bay to the sea, and  $C_g$  is an appropriate admittance coefficient (with dimensions of velocity). Nominally,  $C_g$  represents the wave speed in the sea. The generalized condition (eq. 12) is nearly equivalent to the simplest condition (eq. 11) if  $C_g$  greatly exceeds the wave speed for the bay.

b. Initial Conditions. Since the system includes allowance for frictional dissipation as well as radiation of energy, the solution for given fields of  $X$  and  $Y$  and given boundary function,  $H_g$ , should be reasonably insensitive to the nature of the initial conditions after a suitable lapse of time from the initial state. Thus, the initial conditions can be somewhat arbitrary. As in the laboratory model experiments, it is reasonable to start from a state of equilibrium in which  $U$  and  $V$  are zero and  $H$  is uniform throughout the system, in order to minimize the introduction of transient oscillations related to the starting conditions. Moreover, a reasonable period (depending on the characteristic decay time) can be allowed for the system to reach that state where its response reflects only the effect of the forcing functions.

## 2. Theory of Embedded Channels.

Let  $s$  denote distance along the axis of a channel whose cross-sectional area is  $A$  and surface width is  $w$  at position  $s$  and time  $t$ . Let  $Q$  be the volume transport through  $A$  in the positive sense of  $s$ , and let  $H$  be the water elevation above MSL datum at the same section. In general,  $A$  and  $w$  are known functions of  $H$  for a given cross section, as determined by the geometry of the cross section (Fig. 1). In particular,  $\partial A/\partial H = w$  for given  $s$ . The width  $w$  is to be the "conveyance" width, as used by Dronkers (1964).

The channel is considered an "open system" in the sense that water and momentum may enter or leave the channel laterally; i.e., exchange of fluid with adjacent bay area or flooded land can exist. If the longitudinal velocity in the channel is considered uniform for evaluating the



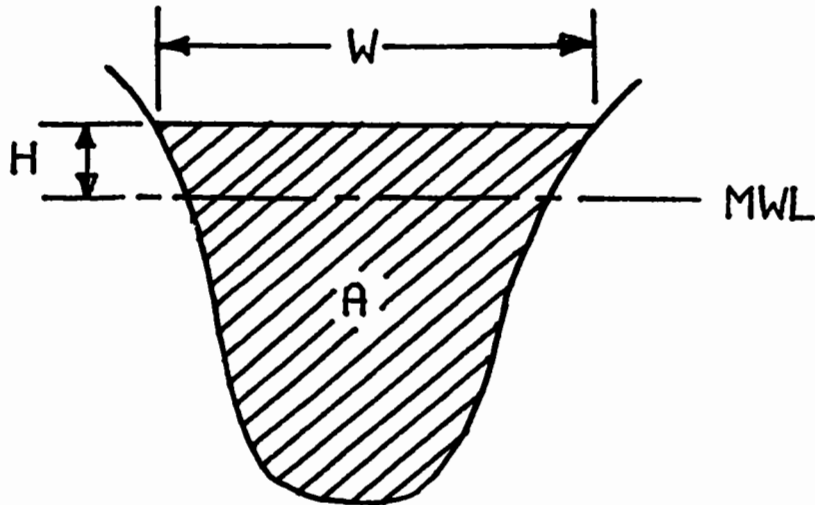


Figure 1. Schematic channel cross section showing pertinent parameters.

longitudinal transport of momentum, then the equations of motion and continuity for a given channel reach are (Stoker, 1957, Ch. 11; Dronkers, 1964, Ch. 9)

$$\frac{\partial Q}{\partial t} + \frac{\partial}{\partial s} (Q^2/A) + gA \frac{\partial H}{\partial s} = wT_s - \sigma Q + L_m \quad (13)$$

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial s} = L_f, \quad (14)$$

where

$T_s$  = longitudinal component of wind stress (divided by water density);

$\sigma = wf|Q|/A^2$  where  $f$  is a dimensionless channel-friction coefficient;

$L_f$  = net time rate for gain of water volume per unit distance along the channel by lateral transfer and rainfall;

$L_m$  = associated net time rate of gain of momentum (divided by water density) per unit distance along channel.

The units of  $L_f$  are square feet per second;  $L_m$  has the units cubic feet per second squared.

It is convenient in the analysis of the channel dynamics to transform the above equations into a characteristic form. There are several different possible characteristic forms. The approach used by Stoker (1957) is to work with  $u$  and  $H$  (where  $u \equiv Q/A$ ) as the dependent variables. Dronkers (1964) works with either  $Q$  and  $H$  directly or with  $Q$  and total head  $(H + (Q/A)^2/2g)$ . Each method has certain

advantages and disadvantages. In the present analysis, the variables  $Q$  and  $H$  are used to be as consistent as possible with the computations in the two-dimensional regions of the system.

In transforming equations (13) and (14) to characteristic form, it is noted that

$$\frac{\partial A}{\partial t} = w \frac{\partial H}{\partial t}$$

$$\frac{\partial A}{\partial s} = w \frac{\partial H}{\partial s} + b, \quad (15)$$

where

$$b \equiv \left( \frac{\partial A}{\partial s} \right)_{H \text{ const.}} \quad (16)$$

(For a channel of uniform cross section the latter quantity would be zero.) It can be shown, following Dronkers' (1964) analysis and considering equation (15), that a characteristic form of equations (13) and (14) is

$$\frac{dQ}{dt} + w \left( -\frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \right) \frac{dH}{dt} = \left\{ wT_s - \sigma Q + L_m + b \left( \frac{Q}{A} \right)^2 + \left( -\frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \right) L_f \right\} \quad (17)$$

along the path  $s(t)$  where

$$\frac{ds}{dt} = \frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \quad (18)$$

The path line where the plus or minus sign is taken in equation (18) is referred to as the positive  $P$  characteristic or the negative  $N$  characteristic path, respectively. These are illustrated in Figure 2 where  $x$  corresponds to  $s$ , the two paths having point  $C$  in common. Equation (17) with the upper sign applies along  $P$  and equation (17)

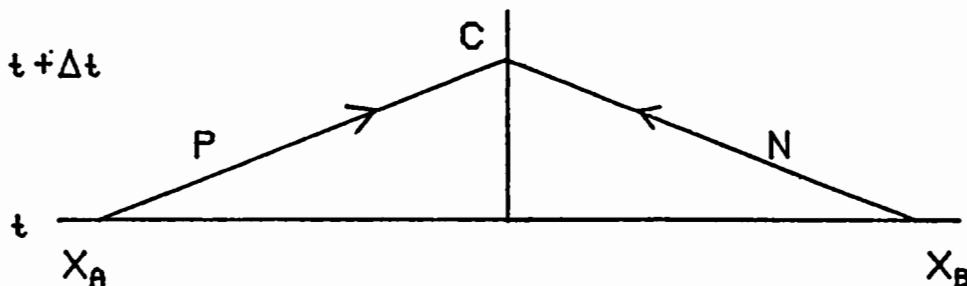


Figure 2. Schematic positive and negative characteristic paths to a common point in the  $x, t$  diagram.

with the lower sign applies on path N. Thus, information with regard to Q and H at points  $x_A$  and  $x_B$  at time t and along the two paths can, in principle, be used to predict the values of Q and H at point C from two equations.

For a laterally closed channel ( $L_f, L_m = 0$ ) of a uniform cross section ( $b = 0$ ) without friction ( $\sigma = 0$ ), in the absence of wind stress ( $T_s = 0$ ), then the quantity in braces on the right-hand side of equation (17) vanishes. In this case only the information at points A and B of Figure 2 is needed to predict values of H and Q at C. To show that equation (17) is consistent with Stoker's (1957) analysis for this special case, let  $u = Q/A$  and  $D = A/w$ . For a uniform cross section at given H,  $dH/dt = dD/dt$ , so equation (17) reduces to.

$$\frac{d(DU)}{dt} + (-u \pm \sqrt{gD}) \frac{dD}{dt} = 0 \quad (19)$$

along

$$\frac{ds}{dt} = u \pm \sqrt{gD}. \quad (20)$$

Equation (19) simplifies further to

$$wD \frac{d}{dt} (u \pm 2\sqrt{gD}) = 0. \quad (21)$$

Thus, for this special case ( $u + 2\sqrt{gD}$ ) is conserved along P where  $dx/dt = u + \sqrt{gD}$ , while ( $u - 2\sqrt{gD}$ ) is conserved along N where  $dx/dt = u - \sqrt{gD}$ . Thus, u and D (hence, Q and H) can readily be evaluated at C.

In the more general case the time integral of the right-hand side of equation (17) must be estimated in a rational way. This is considered later in Section III,2. Also, in the general case it is usually not possible to put the left-hand side of equation (17) in the simple form shown in equation (21).

a. Lateral Transfer Terms. In the absence of direct rainfall,  $L_f$  must equal the net gain of volume per unit length per unit time due to lateral flow into the channel on either or both sides. Let  $q_i$  and  $q_o$ , respectively, represent the volume fluxes per unit length into and out of the channel. Then,  $L_f = q_i - q_o$  in the absence of rainfall, or

$$L_f = q_i - q_o + wR \quad (22)$$

with rainfall. The corresponding lateral transfer of momentum (divided by water density) is

$$L_m = q_i u_i - q_o u_o, \quad (23)$$

the transfer from rainfall being included in the wind-stress term as discussed in Section II,1. In equation (23) the quantity  $u_o$  is simply  $Q/A$  for the channel while  $u_i$  is the channel-directed component of velocity of fluid from the adjoining block water area. In equation (17) the terms  $L_m$  and  $L_f$  contribute to the right-hand side the quantity,

$$F_L \equiv L_m - \frac{Q}{A} L_f \pm \sqrt{\frac{gA}{w}} L_f . \quad (24)$$

Using equations (22) and (23) yields

$$F_L = q_i (u_i - u_o) - wRu_o \pm \left(\frac{gA}{w}\right)^{\frac{1}{2}} (q_i - q_o + wR). \quad (25)$$

The lateral flows into or out of the channel can be evaluated by relations such as equations (8), (9), and (10). This is also discussed in Section III,2.

b. Simplifications. The SURGE II program uses certain simplifications of the above equations. For normal conditions, the propagational speed  $(gA/w)^{\frac{1}{2}}$  significantly exceeds the speeds  $u_i$  or  $u_o$ ; i.e.,  $Q/A$ . Accordingly,  $F$  is approximated by

$$F_L = \pm \left(\frac{gA}{w}\right)^{\frac{1}{2}} L_f . \quad (26)$$

Elsewhere in equations (17) and (18),  $Q/A$  is neglected compared with  $(gA/w)^{\frac{1}{2}}$ . Moreover, each channel reach within a grid block is considered of uniform width and bottom elevation  $Z_c$ ; however,  $w$  and  $Z_c$  vary from one reach to another. Thus,  $b = 0$  for each reach and

$$A/w = D = H - Z_c . \quad (27)$$

Under these conditions equations (17) and (18) take the form,

$$\frac{dQ}{dt} \pm w\sqrt{gD} \frac{dH}{dt} = \{wT_s - f|Q|Q/(D^2w) \pm \sqrt{gD} (q_i - q_o + wR)\} \quad (28)$$

along

$$\frac{ds}{dt} = \pm \sqrt{gD} \quad (29)$$

where  $T_s = X$  or  $Y$  as  $s = x$  or  $y$ , depending on channel orientation. Equation (28) can also be expressed in the form,

$$\frac{d}{dt} \left( Q \pm \frac{2}{3} wD\sqrt{gD} \right) = F \quad (30)$$

for a given channel reach where  $F$  is the right-hand side of equation (28). The neglect of  $Q/A$  relative to  $\sqrt{gD}$  in the above approximate channel equations is tantamount to neglect of longitudinal advection of momentum in the original equation (13), an approximation already made in the two-dimensional equations in Section II, 1.

### III. SURGE II PROGRAM

Numerical algorithms for two-dimensional blocks and subgrid scale channels are given in this section, and the coupling between these is discussed. A complete listing of the SURGE II program is in Appendix A. A description of the program, as adapted for the GE-400 computer, and the required input and output options are discussed in Appendix B. Appendix C is a user's guide to the SURGE II program. The block algorithm is essentially as discussed by Reid and Bodine (1968) except for a change in the barrier computation and incorporation of coupling with the subgrid scale channels.

#### 1. Block Algorithm.

In the numerical analog of the prognostic equations (1), (2), and (3), values of  $H$  are evaluated on a uniform Cartesian mesh at spacing,  $\Delta s$ , for uniform time steps,  $\Delta t$ . The values of  $H$  are representative of the water level for the grid square  $i, j$  which is centered at  $x = (i - 1/2) \Delta s$ ,  $y = (j - 1/2) \Delta s$ , at time  $n\Delta t$ , in which  $i, j$ , and  $n$  are integers. Values of  $Z$  are specified as permanent storage for the same locations as  $H$  so that  $D$  can be evaluated as needed at these locations. Values of  $U$  are evaluated at even half steps of  $x$ , odd half steps of  $y$ , and odd half steps of  $t$  (Fig. 3). This staggered system gives the least storage consistent with a given spatial resolution. It corresponds to the simplest scheme discussed by Platzman (1958) and requires only half the storage compared with the coupled scheme used by Miyazaki (1963).

The variables  $X$  and  $Y$  are supplied at spatial locations consistent with  $U$  and  $V$ , respectively, but at even half steps of  $t$ . Values of  $H_g$  are supplied for positions and times on the outer boundary of the bay consistent with the locations and times for the  $H$  values on that line. Values of  $R$  are supplied at locations consistent with  $H$  but at a one-half time step out of phase with  $H$ . Arrays of  $X, Y$ , and  $R$ , for a single value of  $j$  and  $n$ , and the array of  $H_g$  values for given  $n$  are read from tape as required. The fields of  $X$  and  $Y$  are generated from a coarse spatial and temporal array evaluated from the basic meteorological data and then evaluated for the detailed mesh by linear interpolation.

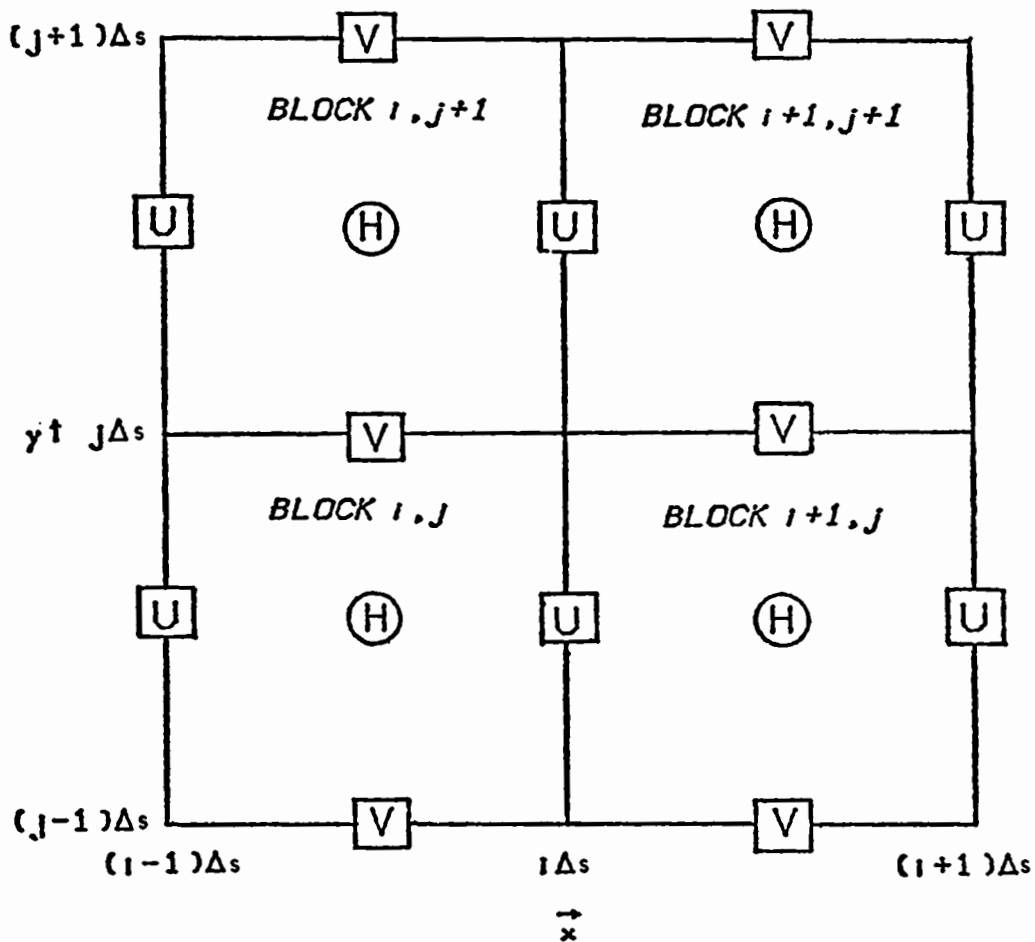


Figure 3. Example of grid blocks showing staggered arrangement of variables U, V, and H.

Information pertinent to the position, elevations, and discharge coefficients for barriers (those not resolved by the limitations of the grid system) is stored as permanent storage along with the field of Z.

The numerical analogs of equations (1), (2), and (3) use values of U, V, H, Z, X, Y, and R at locations shown in Figure 4 for a typical calculation. In the present application a common value of R for given time is used for the whole spatial array. The following notation is used in the recursion equations:  $H(i,j)$  represents H centered in block i, j at  $t = n\Delta t$ ;  $U(i,j)$  represents U for the left side of block i, j at  $t = (n - 1/2) \Delta t$ ;  $v(i,j)$  represents V for the lower side of block i, j at  $t = (n - 1/2) \Delta t$ .

Primed symbols are used to denote values of these variables at time step  $\Delta t$  later. Thus, the difference  $U' - U$  is centered in time at the level of H, and the difference  $H' - H$  is centered in time at the level of  $U'$  or  $V'$ . The notation for Z or D is consistent with that of H.

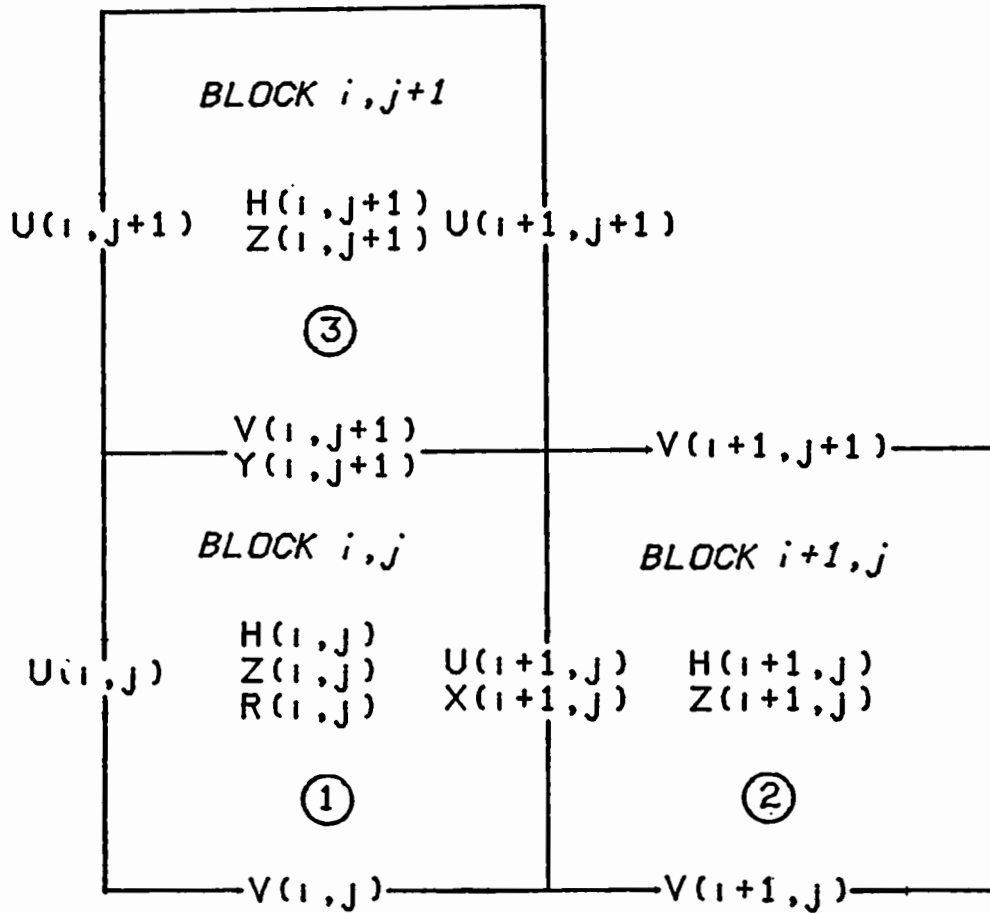


Figure 4. Basic block triad showing variables used in computation of U, V, and H for block 1.

The frictional terms in equations (1) and (2) are represented by  $fAU'D^{-2}$  and  $fQV'D^{-2}$ , respectively, where the estimation of Q and D is centered spatially at the position for U' or V'. Since U, V, and D are not available at common locations, this requires a suitable spatial average in order to obtain centered values of Q and D. The resulting recursion equations for U, V, and H, using centered differences for the spatial derivatives, are as follows:

$$U'(i+1, j) = \frac{1}{G_1(i, j)} \left\{ U(i+1, j) + \frac{g\Delta t}{2\Delta s} [D(i+1, j) + D(i, j)] [H(i, j) - H(i+1, j)] + X(i+1, j)\Delta t \right\} \quad (31)$$

$$V'(i, j+1) = \frac{1}{G_2(i, j)} \left\{ V(i, j+1) + \frac{g\Delta t}{2\Delta s} [D(i, j+1) + D(i, j)] [H(i, j) - H(i, j+1)] + Y(i, j+1)\Delta t \right\} \quad (32)$$

$$H'(i,j) = H(i,j) + \frac{\Delta t}{\Delta s} [U'(i,j) + V'(i,j) - U'(i+1,j) - V'(i,j+1)] + R(i,j)\Delta t, \quad (33)$$

where

$$D(i,j) = H(i,j) - Z(i,j), \quad (34)$$

and  $G_1$  and  $G_2$  are the factors which incorporate the effect of the friction. These are given by:

$$G_1(i,j) = 1 + f\Delta t \{ [4U(i+1,j)]^2 + [V(i,j) + V(i+1,j) + V(i,j+1) + V(i+1,j+1)]^2 \}^{\frac{1}{2}} [D(i,j) + D(i+1,j)]^{-2} \quad (35)$$

and

$$G_2(i,j) = 1 + f\Delta t \{ [4V(i,j+1)]^2 + [U(i,j) + U(i+1,j) + U(i,j+1) + U(i+1,j+1)]^2 \}^{\frac{1}{2}} [D(i,j) + D(i,j+1)]^{-2}. \quad (36)$$

The latter factors are always somewhat greater than unity unless the flow or friction factor vanishes.

The prediction relation for  $H$  given by equation (33) does not consider any possible contribution of flow to or from the block due to the presence of a subgrid scale channel. This will be considered in a subsequent section.

It should also be emphasized that the effect of Coriolis force is not considered. The relative importance of the Coriolis force compared with bottom friction can be estimated in terms of the ratio,  $r$ , of these two forces which is of the order,

$$r = \lambda D / fu, \quad (37)$$

where

$\lambda$  = Coriolis parameter ( $2\Omega \sin \phi$ ,  $\Omega$  being the absolute angular speed of the earth and  $\phi$  the latitude);

$D$  = mean depth;

$f$  = bottom-friction coefficient;

$u$  = typical fluid speed in the bay.



For 30° latitude  $\lambda = 7.3 \times 10^{-5}$ ; typical D and f for gulf coast bays are 10 feet and  $2 \times 10^{-3}$ , respectively. For  $u = 3$  feet per second, which is reasonable for storm conditions, r is only 1/10. However, for normal circulatory regimes u may be only a fraction of 1 foot per second and r is of order unity. Hence, while it may be justifiable to neglect the Coriolis term for short-duration storm surge studies for shallow bays of limited horizontal dimensions it cannot be neglected in long-term circulatory studies.

Although it does not appear difficult to add the effect of Coriolis force, it can be shown (Platzman, 1958) that a different scheme for the U, V, and H arrays is necessary for numerically stable computations using an explicit time-marching procedure as used here. The coupled scheme required for stable explicit computations at least doubles the computing time. The present scheme could be used with an implicit time-marching procedure to maintain stability and similar accuracy, but this too can be achieved only at the cost of an increase in computing time by a factor of at least two. In the presence of friction, the destabilizing effect of the Coriolis terms in an explicit scheme such as that used by Masch (1969) is suppressed; however, this is accomplished only at the sacrifice in rendition of the frictional terms. Thus, the omission of the Coriolis force from a program intended primarily for gulf coast estuaries is motivated primarily for reasons of economy of operation, in respect to surge calculations.

a. Stability. Numerical stability requires that  $\Delta t$  be taken at less than the value  $\Delta S / (2gD_{\max})^{1/2}$ , where  $D_{\max}$  is the maximum depth to be expected anywhere in the system during the storm surge (Platzman, 1958).

b. Barrier Algorithm. Equations (9) and (10) are assumed to apply for values of  $q_n$ ,  $D_b$ , and  $\Delta H$  at the same time and in the immediate vicinity of the barrier. In the grid scheme used, however, the flow and the water level are staggered in time; moreover, the water levels like  $H_1$  and  $H_2$  represent in effect the spatial average for blocks 1 and 2, respectively, at a given time rather than local values in the vicinity of a given barrier, which in the schematization are presumed to occur on lines separating two blocks. As a consequence the above relations cannot be applied directly. Instead, the evaluation of U or V across a barrier (if the water level allows such flow) is carried out by a modified version of the predictive equations (1) and (2), or their numerical counterparts, equations (31) and (32), where f is replaced by an effective value related to the barrier discharge coefficient so as to be consistent with equations (9) or (10). The effect is to maintain proper time phasing and to consider possible tilt of water level across the block; i.e., difference of H at barrier relative to the mean value for the block.

Specifically, the frictional terms in equation (1) or (2) are taken as  $(\bar{D}/LC_b^2) |q_n'|/q_n'/D_b^2$  where  $C_b$  is the barrier discharge coefficient

( $C_0$  or  $C_s$ , depending on type of barrier),  $q_n'$  is the transport per unit width normal to the barrier (either  $U'$  or  $V'$ , depending on barrier orientation),  $D_b$  is the water depth over the barrier, and  $\bar{D}$  is a mean depth for the effective fetch  $L$  across the blocks. The gravitational slope term involves the same scale length,  $L$ , and mean depth,  $\bar{D}$ . The resulting relation for prediction of  $q_n'$  at a barrier, given  $q_n$  at the previous time step, is:

$$|q_n'|q_n' + \Gamma q_n' = F, \quad (38)$$

where

$$\Gamma \equiv \frac{L(C_b D_b)^2}{\bar{D}\Delta t} \quad (39)$$

and

$$F \equiv g(C_b D_b)^2 \Delta H + \Gamma \cdot (q_n + P), \quad (40)$$

$P$  being the wind "push" term ( $X\Delta t$  or  $Y\Delta t$ ), and  $\Delta H$  a head differential dependent on barrier type. For steady state ( $q_n' = q_n$ ) and no wind ( $P = 0$ ), the above reduces to

$$q_n' = \pm C_b D_b \sqrt{g|\Delta H|}, \quad (41)$$

which is consistent with equation (9) or (10) with  $C_b$  and  $\Delta H$  taken as  $C_0$  and  $D_b$  or  $C_s$  and  $(H_1 - H_2)$ , respectively, depending on the barrier. The more general relation (eq. 38) provides an added effect of the wind and of the inertia of the water on the blocks. For a submerged barrier,  $L$  is taken equal to  $\Delta S$ ; i.e., from the center of block 1 to the center of block 2. For an overflow barrier,  $L$  is taken as half this distance since the inertia and wind setup are effective only on the higher of the two blocks.

Thus,  $C_b$ ,  $L$ ,  $H$ , and  $D_b$  are taken as follows:

Submerged barrier ( $H_1 > Z_b$  and  $H_2 > Z_b$ )

$$C_b = C_s$$

$$L = \Delta S \quad \Delta H = H_1 - H_2$$

$$D_b = \left[ (H_1 + H_2) / 2 \right] - Z_b \quad (42)$$

Overflow barrier ( $H_1 > Z_b$  or  $H_2 > Z_b$ )

$$C_b = C_0$$

$$L = \Delta S / 2$$

$$D_b = |\Delta H|$$

$$\Delta H = \begin{cases} H_1 - Z_b & \text{(a)} \\ \text{or} \\ Z_b - H_2 & \text{(b)} \end{cases},$$

where  $Z_b$  is the elevation of the barrier crest, relation (a) being for  $H_1 > Z_b$  and (b) for  $H_2 > Z_b$ . If  $Z_b$  exceeds both  $H_1$  and  $H_2$ , then  $q_n' = 0$ . The meaningful solution of the quadratic equation (38) is

$$q_n' = \pm \{ [|F| + (\Gamma/2)^2 ]^{1/2} - \Gamma/2 \}, \quad (43)$$

where the sign is taken as that of  $F$ , as verified from equation (38).

The above relations for barriers differ from that used in Reid and Bodine (1968) and in the original SURGE I program. The present barrier relations have a more realistic response when applied to the numerical simulation of a natural oscillation of a bay having a submerged barrier across it.

c. Barrier Specification. Since only certain blocks contain barriers, they must be identified by  $I, J$  location; specifically, the program identifies the  $K$ th barrier block by location  $I = IB(K)$  and  $J = JB(K)$ ,  $K = 1, 2 \dots KM$ . A given barrier block potentially has a barrier on the right and upper side of the block in an  $x, y$  plot. These are designated  $x$  and  $y$ , respectively; i.e., an  $x$  barrier is one normal to the  $x$ -axis (the flow over it being in the  $x$  sense). For both potential barriers on a barrier block, values of  $Z_b$ ,  $C_0$ , and  $C_s$  must be prescribed. A real barrier is one where  $Z_b$  is larger than the  $Z$  value for either of the adjoining blocks. A null barrier is one where  $Z_b$  equals the larger of the  $Z$  values for the adjoining blocks (thus, in effect, the higher block is a potential barrier). The program requires that information pertinent to both null barriers ( $Z_b$ ,  $C_0$ , and  $C_s$ ) and real barriers be provided.

d. Volume Check. During the recession stage of flooding when water is draining off flooded blocks (via the barrier overflow relation), it is possible for the volume leaving in one time step as computed from  $q_n' \Delta t$  to exceed the available volume. Therefore, a test is included in the program such that if this occurs, the flow is adjusted to only drain the block dry ( $D = 0$ ), and the flow to adjacent blocks adjusted to be consistent.

e. Depth Check. When the water depth is very shallow the effect of the wind is such that a given block could become partially dry unless the fluid is flowing fast enough for the bottom stress to balance the wind stress. To avoid anomalous computations for very small  $D$  (e.g., in areas where rainfall is occurring over regions above the surge level), the wind stress is arbitrarily set zero when  $D$  is less than 0.1 foot.

## 2. Channel Algorithm.

a. Channel Specification. As in the case of barriers, those blocks on which channels occur are identified by the  $I$  and  $J$  values; for channel block  $K$  these are denoted by  $ICG(K)$  and  $JCG(K)$ , respectively, where  $K = 1, 2 \dots KCM$ . Also each "channel block" may contain two channels, one on the right denoted the  $x$  channel and one on the upper side denoted the  $y$  channel. Each of these channel reaches is characterized by a

channel width ( $w$ ), a channel-bed elevation ( $Z_c$ ), and a channel-friction coefficient ( $f_c$ ). Figure 5 shows a schematic of a channel block indicating nomenclature for dimensions as used in the SURGE II program. Figure 6 shows the dependent variables pertinent to the channels as used in the program and stored for the channel block  $K$ . These include the channel flows,  $Q$ , at each end of the channel, one end designated  $N$ , the other  $P$  (corresponding to the negative and positive characteristic ends of the channel, respectively). Also included is the height,  $H$ , of the water level at the point in common to the two channels for block  $K$  ( $HC(K)$ ). The lateral transport (per unit width per unit time) nominally to the channel from block  $K$  and from the channel is also indicated:  $UCT(K)$  and  $UCF(K)$ , respectively, for the channel normal to the  $x$ -axis, and  $VCT(K)$  and  $VCF(K)$ , respectively, for the channel normal to the  $y$ -axis. In the formulas in this study, these are referred to as  $q_t$  and  $q_f$ , respectively. Note that  $UCF(K)$  and  $VCF(K)$  correspond to  $U$  and  $V$ , respectively, on the right and upper sides of the general block flow. Also, the quantity  $HP(K)$  corresponds to the block (pool) height for the channel block. Values of  $H$  at the "negative" ends of the channels for channel block  $K$  are stored as  $HC$  values in adjacent channel blocks to minimize duplication of storage.

b. Computation of Channel Variables. The time phasing of block variables versus channel variables is indicated in Table 1. The  $H$  values occur at common times thus facilitating evaluation of head differentials used in determining lateral flow between channel and adjacent blocks.

Table 1. Time phasing of computations for blocks and channels.

Time	Block	Channel
$t + \Delta t$	$H$	$H, Q$
$t + \Delta t/2$	$Q$	
$t$	$H$	$H, Q$
$t - \Delta t/2$	$Q$	
$t - \Delta t$	$H$	$H, Q$

For a given channel reach, application of equations (28) and (29) can be made for two characteristic paths, as shown schematically in Figure 7. As in the case of the block computations, the friction term in equation (28) is taken proportional to the product of a new  $Q$  and

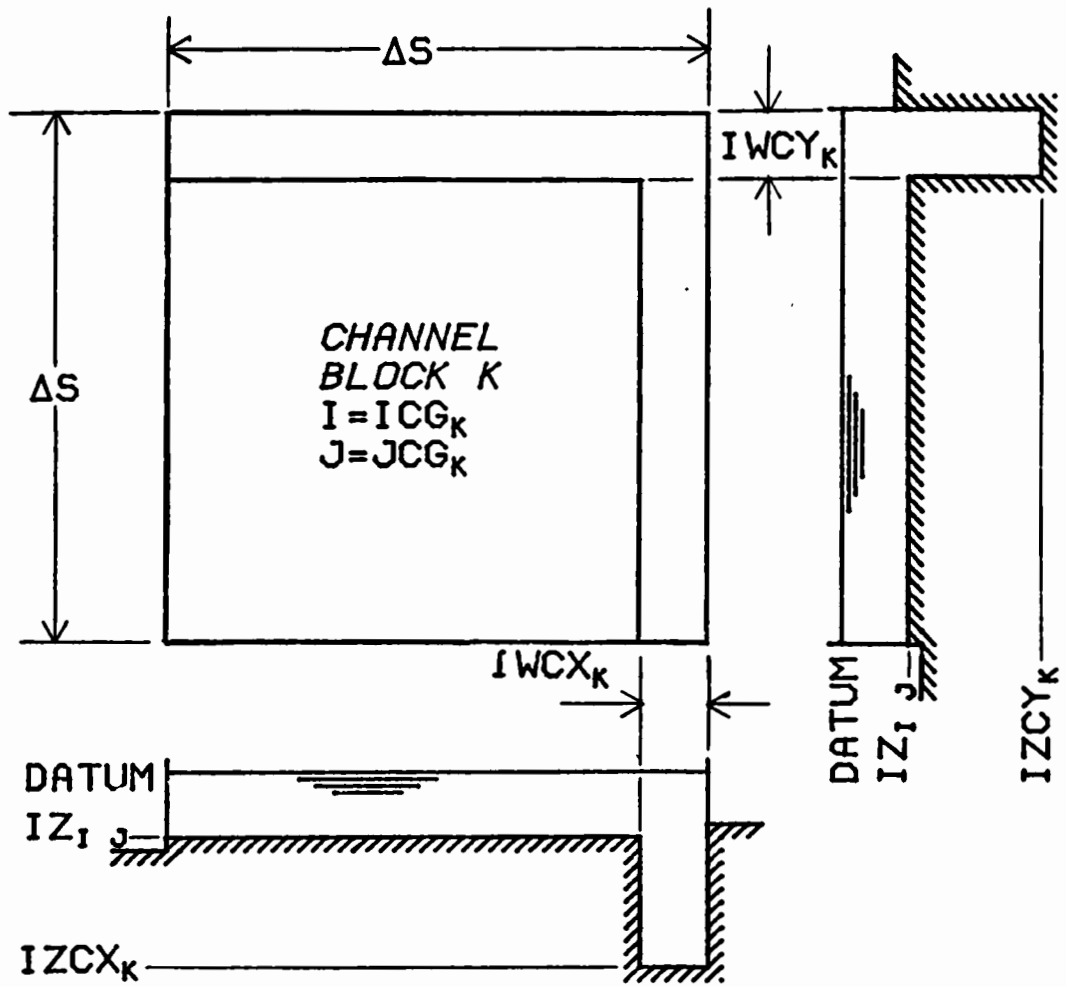


Figure 5. Channel block, showing channels and their dimensions.

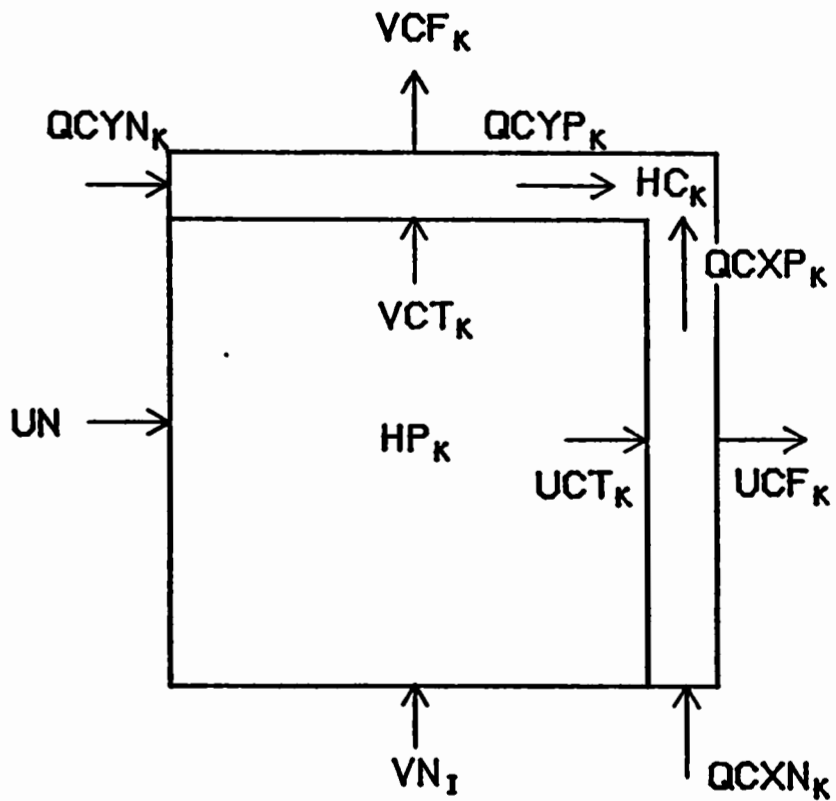


Figure 6. Channel block K at coordinates  $I = ICG(K)$  and  $J = JCG(K)$ , showing associated flows and water level variables.

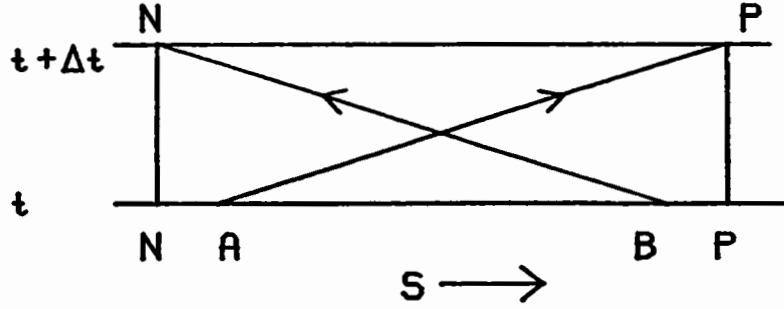


Figure 7. Characteristic paths on the time-distance diagram for an individual channel reach.

the absolute value of the old  $Q$ . Specifically, for the positive characteristic path from  $A$  to  $P'$  in Figure 7, equation (28) is approximated by

$$(Q'_P - Q'_A) + w\sqrt{g\bar{D}}(H'_P - H'_A) = [WT_s - f_c|\bar{Q}|Q'_P/(\bar{D})^2w + \sqrt{g\bar{D}}\Delta q] \Delta t, \quad (44)$$

where  $\bar{D} = (D_N + D_P)/2$ ,  $T_s$  is the appropriate wind-stress component (X or Y) corresponding to time level  $t$  for the associated channel block,  $\Delta q$  is the net lateral flow per unit width, and  $\bar{Q}$  is taken as

$$\bar{Q} = [(Q_N^2 + Q_P^2)/2]^{1/2}. \quad (45)$$

The subscripts on  $Q$ ,  $H$ , and  $D$  designate the points at which these apply (see Fig. 7) and primes denote new time level.

After regrouping terms, equation (44) can be written as

$$Q'_P + (w\sqrt{g\bar{D}}/G)H'_P = [(Q'_A + w\sqrt{g\bar{D}}H'_A) + (WT_s + \sqrt{g\bar{D}}\Delta q) \Delta t]/G, \quad (46)$$

where

$$G \equiv 1 + f_c \Delta t |\bar{Q}| / (\bar{D})^2 w. \quad (47)$$

Similarly, for the negative characteristic from  $B$  to  $N'$ ,

$$Q'_N - (w\sqrt{g\bar{D}}/G)H'_N = [(Q'_B - w\sqrt{g\bar{D}}H'_B) + (WT_s - \sqrt{g\bar{D}}\Delta t)/G], \quad (48)$$

where  $\bar{D}$  and  $G$  are as defined for the positive characteristic.

The values of  $Q$  and  $H$  at points  $A$  and  $B$  are determined by interpolation from values at  $N$  and  $P$  at time  $t$ , using equation (29) for the path. The distance from  $A$  to  $P$  or  $B$  to  $N$ , using the mean wave speed for the channel at time  $t$  is  $\sqrt{g\bar{D}}\Delta t$ . The interval  $N$  to  $P$  is equal to  $\Delta s$ . Let

$$\alpha \equiv \sqrt{g\bar{D}} \Delta t / \Delta s; \quad (49)$$

this should always be less than or at most unity for stability of computation. The linearly interpolated values at A and B are then

$$Q_A = \alpha Q_N + (1 - \alpha) Q_P$$

$$Q_B = (1 - \alpha) Q_N + \alpha Q_P,$$
(50)

and similarly for  $H_A$  and  $H_B$  in terms of  $H_N$  and  $H_P$ .

The evaluation of  $\Delta q$  is the most sensitive part of the computations and is discussed in a subsequent section. Presuming  $\Delta q$  is known, the problem of evaluating the new  $Q$  and  $H$  individually at the channel-end points is considered. Note that equations (46) and (48) yield predictions for linear combinations of  $Q$  and  $H$  at two different points. Thus, information from adjoining channels, or other information in the case of channel end points, is needed to solve for the new channel  $Q$  and  $H$ . For a simple continuous channel without branches and consisting of a series of reaches of length  $\Delta s$  but not necessarily of equal width or depth, then  $Q$  and  $H$  are readily solved at a common junction, using the information from the positive characteristic from one channel and the negative characteristic from the adjoining channel. However, branches do occur and it is therefore desirable to use a sufficiently general procedure which will accommodate either branching channels or continuous channels.

In the scheme chosen for representing channels in SURGE II it is possible to have four channels merging at a common junction. Figure 8 shows this junction with four different volume transports, but with a common  $H$ . The designation of the different  $Q$  shown in this figure is that used in the coded program (see App. B);  $Q_C$  for channel transport,  $X$  or  $Y$  denoting the channel (not the direction of flow), and  $N$  or  $P$  denoting whether the flow is at the negative or positive end of a given channel reach. Each is identified by a channel block index  $K$ .

For any given channel reach equations (46) and (48) predict, for a given point, values of the quantities

$$BP \equiv Q' + \lambda H'$$

$$BN \equiv Q' - \lambda H' ,$$
(51)

where

$$\lambda \equiv w \sqrt{gD}/G .$$
(52)



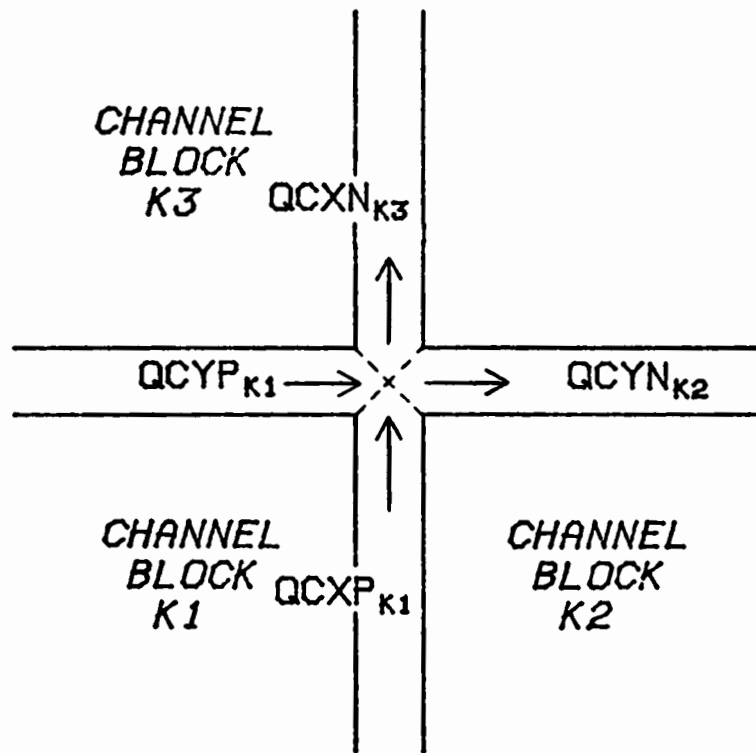


Figure 8. General channel junction, showing flows and channel identification.

For simplicity of notation let 1, 2, 3, and 4 denote the merging channels with 1 being the lower channel, 2 the left channel, 3 the upper channel, and 4 the right channel (Fig. 8). Then, with this notation

$$\begin{aligned}
 Q1' + \lambda 1 \cdot H' &= BP1 \\
 Q2' + \lambda 2 \cdot H' &= BP2 \\
 Q3' - \lambda 3 \cdot H' &= BN3 \\
 Q4' - \lambda 4 \cdot H' &= BN4 .
 \end{aligned}
 \tag{53}$$

Now, continuity requires that

$$Q1' + Q2' - Q3' - Q4' = 0 \tag{54}$$

at a common junction. Thus,

$$(\lambda 1 + \lambda 2 + \lambda 3 + \lambda 4) H' = BP1 + BP2 - BN3 - BN4 \tag{55}$$

from which  $H'$  can be calculated at the junction. With  $H'$  known, the values of  $Q1'$ ,  $Q2'$ ,  $Q3'$ , and  $Q4'$  are readily evaluated from equation (40).

For those cases where one or two of the above merging channels do not exist (i.e., null channels), then their width and  $\lambda$  value are zero. Moreover, the program yields zero for the BP or BN values of any null channel. Thus, equations (55) and (53) can apply for a general junction consisting of from two to four real merging channels.

c. Net Lateral Flow. The net time rate of water accumulation in the channel per unit length due to lateral exchange with blocks and by rainfall is

$$\Delta q = q_t - q_f + wR , \tag{56}$$

where  $q_t$  corresponds (if positive) to the flow (per unit length of channel) from the channel block to the channel (across the "interior" side of the channel, Fig. 6) and  $q_f$  (if positive) is the flow (per unit length of channel) from the channel to the adjacent block. These flows can be positive, negative, or zero. To allow for channels which have widths  $w$  much smaller than the block grid size  $\Delta s$ , and since the above  $q$  values are comparable to those which exist across the sides of blocks, the change in channel water level can be very sensitive to the difference  $q_t - q_f$ . Hence, special care must be taken in the model to avoid possible instabilities caused by improper calculation of these transverse flows. However, there is no particular difficulty with the rainfall term in equation (56) which is generally at least one order of magnitude smaller than that of the "net" lateral flow. In a sense, the potential difficulty with the transverse flows,  $q_t$  and  $q_f$ , arises because the  $\Delta t$  chosen for stable calculation on the blocks is usually

too large for stable calculation for narrow channels, unless the coupling with blocks exists only in respect to longitudinal flow from the channels to blocks at end points of such channels.

On a given side of a channel, basically four physically distinct situations can occur: (a) a barrier (levee) or block ground level of sufficient height exists to prevent lateral flow; (b) overflow exists from an adjacent flooded block into a channel where the water level is less than the adjacent barrier or ground level; (c) overflow of adjacent barrier (levee) exists from the channel to an adjacent dry block or one where the water level is lower than the barrier elevation; or (d) both the channel water level and the water level on the adjacent block exceed the height of any intervening barrier and the lateral flow depends on the difference of water level. These four situations are illustrated in Figure 9. In the fourth situation, the water level could also be lower on the channel side with the associated lateral flow reversed.

For situation (a) there is no problem, the appropriate lateral flow ( $q_t$  or  $q_f$ ) being constrained to zero value. For situation (c), the predictive-type barrier relation (eq. 55), with auxiliary relations (eqs. 39 and 40), could be used. In principle, the above predictive barrier relations should apply for situation (b) as well, provided that  $L$  in equation (39) is taken as the channel width  $w$ . However, since  $w$  can be much less than  $\Delta s$  for many applications,  $\Gamma$  can be so small that the relation for  $q_n'$  reduces virtually to a diagnostic-type relation of equation (40), or more specifically of equation (9) for barrier overflow. Since situation (b) might occur on one side of the channel and situation (c) on the other, and since both should be evaluated by relations compatible with a common time level, the simple diagnostic relation (eq. 9) has been adopted for both situations in the SURGE II program. This, however, still demands special checks and possible adjustments, as will be discussed later. Finally for situation (d), a submerged barrier-type calculation might seem appropriate if the depth over such a barrier is small compared with that of the channel or adjacent block; however, use of such relations in preliminary versions of the program proved to be very vulnerable to numerical instability. The reason for this is related to the above discussion concerning the usual case where  $w/\Delta s$  is very small. As a consequence, for situations of type (d), a special calculation is required which treats the channel as essentially an integral part of the associated channel block or the adjacent block.

As stated above, for overflow situations (b) or (c), i.e., to or from the channel, the relation,

$$q_n = \pm C_o D_b (g D_b)^{1/2}, \quad (57)$$

is used where  $D_b = H - Z_b$ ,  $H$  being the water level on the high side of the barrier. While this relation gives a valid value of  $q_n$  ( $q_t$  or  $q_f$ ) at the time  $t$ , the value of  $q_n$  may change significantly over the prediction interval  $\Delta t$  if  $(g D_b)^{1/2} > w/\Delta t$ .

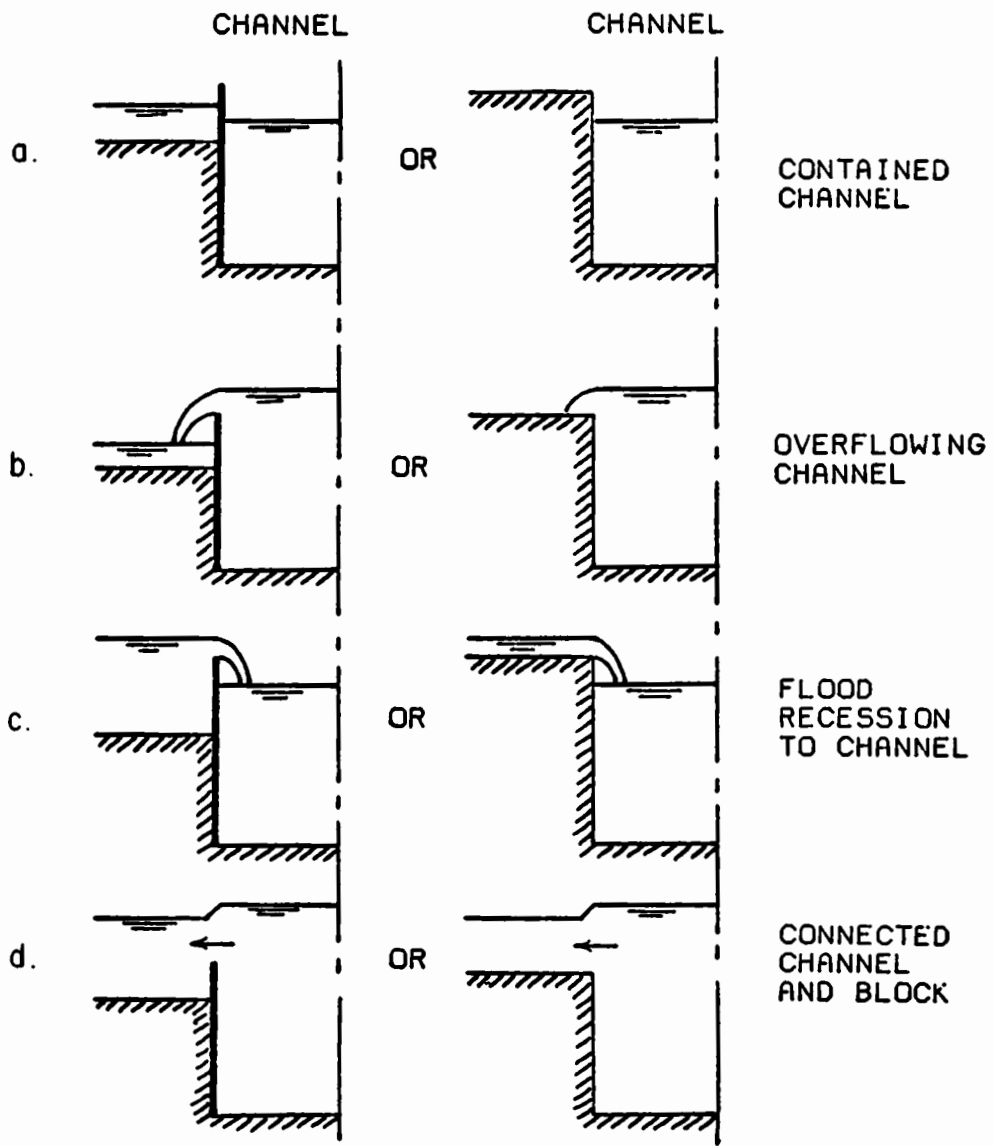


Figure 9. Different situations along a given side of a channel.

Under such circumstances, an approximate prediction based on the initial values of  $q_n$  could lead to physically impossible changes of channel level. Thus, tests are included in the program to constrain the lateral flow, such that  $q_t - q_f$  alone will not cause the channel level,  $H_c$ , to fall below a minimum possible value nor rise above a maximum possible value, depending on the situation. Six different situations requiring tests are illustrated in Figure 10 (the "mirror" version of each is also a possible situation). Situations where one side of the channel is blocked are special cases of those indicated. For situations A, C, and E, outflow exceeds inflow and the horizontal dashline represents a minimum level based on the sill depth of the channel. On the other hand, for situations B, D, and F, the horizontal dashline represents a maximum possible level. In each case, the maximum possible change in  $H_c$  is indicated as  $\Delta H_c$ .

For any of the situations illustrated in Figure 10, the SURGE II program compares  $|q_t - q_f|$  with  $|wH_c/\Delta t|$ . If the latter is exceeded by the trial value of  $|q_t - q_f|$  then an adjustment is made in  $q_t$  or  $q_f$  such that  $|q_t - q_f|$  equals  $|w\Delta H_c/\Delta t|$ . For cases A, B, C, and D, both  $q_t$  and  $q_f$  are prorated by a common factor to satisfy the above constraint. For cases E and F, only the overflow  $q$  is adjusted to be consistent with the above constraint.

For situation (d) where the channel and block are connected by a continuous water surface (Fig. 10), the net lateral flow to the channel,  $\Delta q$ , is taken to be that which would be required to bring  $H_c$  to a value equal to the existing mean level,  $H_M$ , of the connected channel and block. For a channel connected to a block on one side only then,

$$H_M = \frac{H_B \cdot L + H_C \cdot W}{L+W}, \quad (58)$$

where  $H_B$  is the water elevation on the water-connected block,  $L$  is its width, while  $H_C$  and  $W$  are the water elevation and width for the channel. The block width  $L$  is  $\Delta S - W$  if the connected block is the channel block containing the channel, or is  $\Delta S$  for an adjacent water-connected block. If the channel is water connected on both sides, then the above relation is replaced by an appropriate average over both blocks plus the channel.

The  $\Delta q$  for either of these situations is taken as

$$\Delta q = (H_M - H_C)w/\Delta t. \quad (59)$$

To determine the individual  $q_t$  and  $q_f$  on either side of the channel, the mean of these is taken to be that which is calculated as the flow

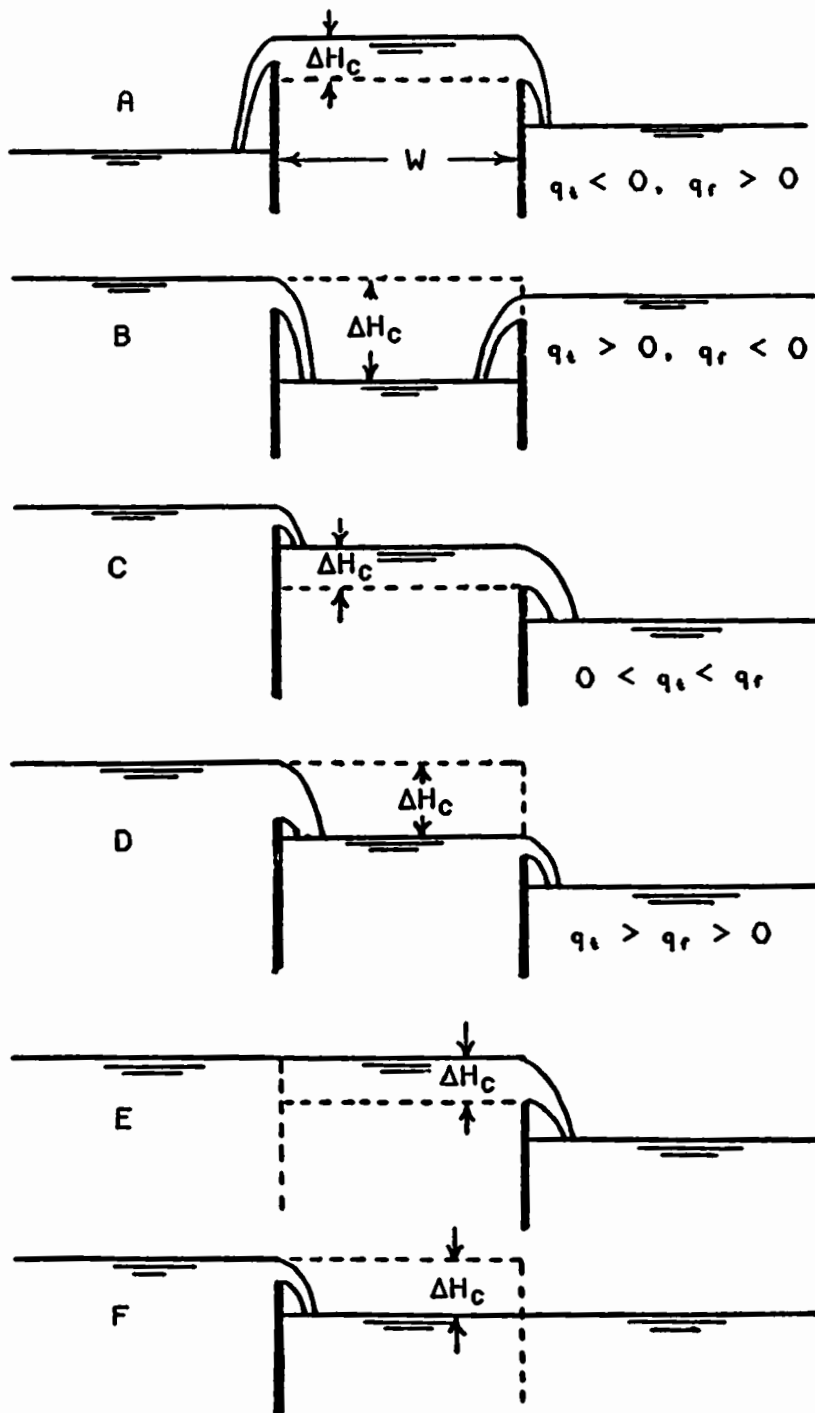


Figure 10. Situations involving overflow to or from a channel which require special checks.

between blocks, ignoring the presence of the channel (but considering barriers). Letting this be denoted  $q_m$ , then

$$q_t = q_m + \Delta q/2$$

and

$$q_f = q_m - \Delta q/2 .$$

This system of calculation leads to stable results.

d. Channel End-Point Computations. At the end point of a given channel system, special computations are required. Two types of end conditions are used: an "H-end condition" is used where a channel discharges into a lake, bay, or sea, in which case the channel H value at the end point is taken equal to the H of the adjacent channel block into which the channel discharges (or vice versa); a "Q-end condition" is used at the head of a channel or river at which point the discharge is specified.

For a Q-end point

$$Q' = \pm Q'_r$$

$$H' = (Q' - B)/\lambda , \quad (61)$$

where  $Q'_r$  is the specified river discharge (taken as zero if not specified); B equals BP or -BN, as defined by equation (51), for end points occurring at the positive or negative end of the channel reach, respectively, and  $\lambda$  is as defined in equation (52). The sign of  $Q'$  is taken such that  $Q'$  is directed into the channel, depending on the channel-end orientation. There are four possible orientations (see App. B, Fig. B-3).

The H-end points also have four possible configurations; these are depicted along with the associated adjacent "ponding" areas (i.e., a block with  $Z < 0$ ) in Figure 11. For an H-end point neither the longitudinal flow to or from the channel nor the H at the junction with the ponding block is specified *a priori*. It is required only that the predicted H at the channel-end point and that of the ponding block be the same. Let  $H^*$  be the (tentative) predicted H for the ponding block in the absence of any contribution by longitudinal discharge to or from the channel which terminates adjacent to that block. Thus,  $H^*$  corresponds to the H resulting from the routine block calculation using equation (33) with appropriate adjustments for contained channels as might occur for situations 3 and 4 shown in Figure 11. These adjustments are discussed in a subsequent subsection. The correct predicted H for the ponding block in the presence of longitudinal discharge from a channel is given by

$$H' = H^* + (Q'_d + Q_d)\Delta t/2A_b , \quad (62)$$

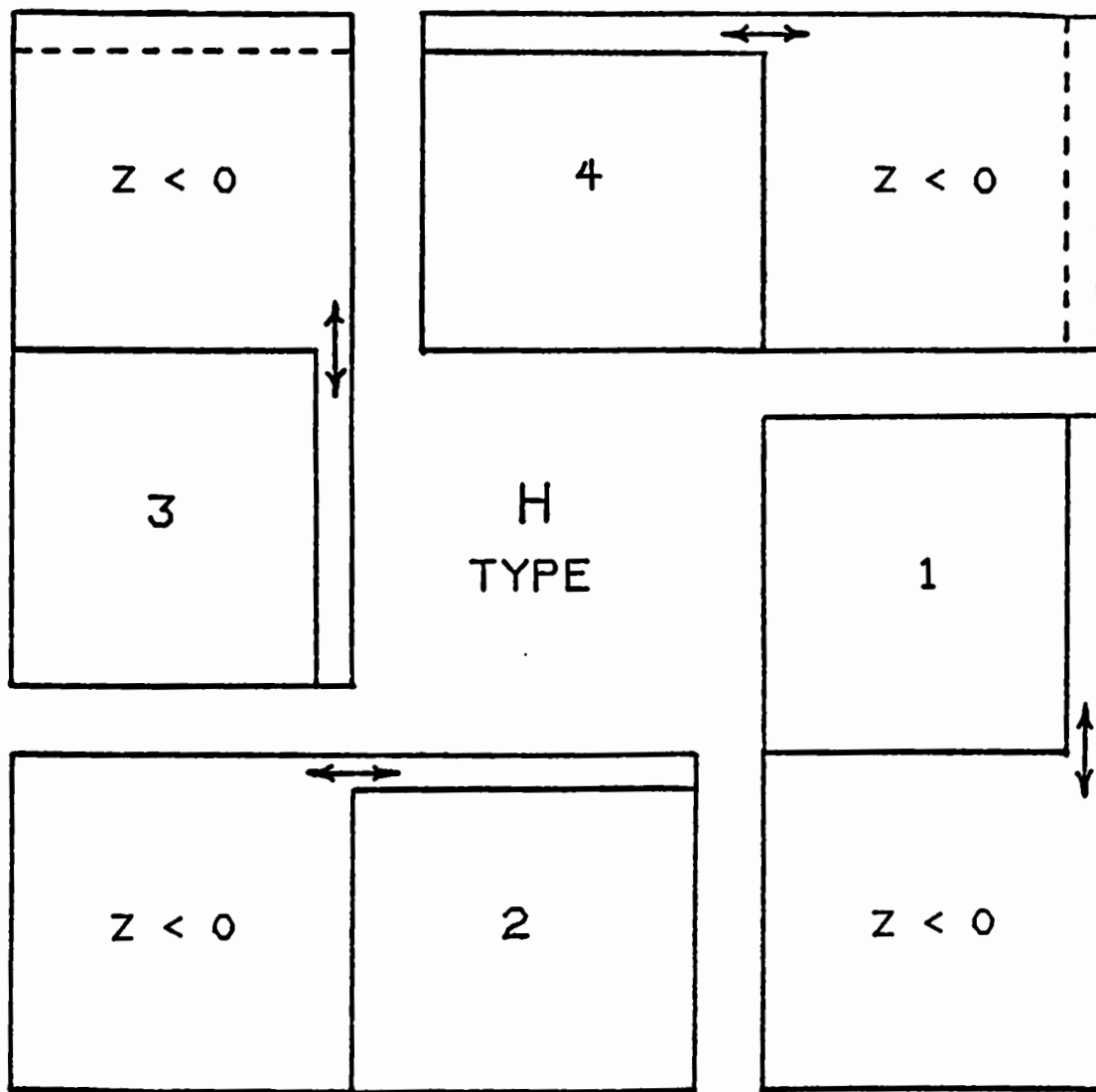


Figure 11. Possible end-point configurations and index identification (1 to 4).



where  $Q'_d$  and  $Q_d$  are the new and previous values, respectively, of the discharge from channel to ponding block, and  $A_b$  is the effective surface area of the block. For situations 1 and 2 in Figure 11,  $A_b = (\Delta s)^2$ , but for situations 3 and 4 a channel might exist on the ponding block in which case  $A_b = (\Delta s - w)\Delta s$ .

Equation (62) involves two unknowns  $H'$  and  $Q'_d$ . However, for the channel,

$$Q'_d + \lambda H' = B, \quad (63)$$

where  $B = -BN$  for end-point type 1 or 2 and  $B = BP$  for end-point type 3 or 4,  $BN$ ,  $BP$  and  $\lambda$  being those quantities defined by equations (46) to (52). Note that for end-point type 1 or 2,  $Q'_d$  is the negative of the QC value for the channel.

The resulting  $H'$  and  $Q'_d$  for an "H-end" condition are

$$H' = (F + B\Delta t/2A_b)/(1 + \lambda\Delta t/2A_b) \quad (64)$$

$$Q' = (B - \lambda F)/(1 + \lambda\Delta t/2A_b), \quad (65)$$

where

$$F \equiv H^* + Q_d\Delta t/2A_b. \quad (66)$$

e. Calculation of H on Channel Blocks. For blocks with  $D > 0$  and containing one or two channel reaches, the prediction relation for  $H$  given by equation (33) is not valid. The correct relation for a channel block  $k$  having location  $i,j$  is

$$\begin{aligned} H'(i,j) = & H(i,j) + [U'(i,j) - UCT'(k)]\Delta t/(\Delta s - wx) \\ & + [V'(i,j) - VCT'(k)]\Delta t/(\Delta s - wy) \end{aligned} \quad (67)$$

where  $UCT$  and  $VCT$  are as shown in Figure 6 and correspond to the  $q_t$  discussed previously. If only one channel exists (i.e., if  $wx$  or  $wy$  is zero), then

$$UCT'(k) = U'(i + 1, j) \text{ if } wx = 0$$

or

$$VCT'(k) = V'(i, j + 1) \text{ if } wy = 0.$$

#### IV. APPLICATION TO THE SABINE-CALCASIEU SYSTEM

##### 1. Adopted Grid and Simulated Topography.

The Sabine-Calcasieu system geographically bridges the Texas-Louisiana border and is physically linked by a system of manmade channels and a low-lying region extending 25 miles between Sabine Lake and Lake Calcasieu.

A local chart of the region is shown in Figure 12. The rectangular border indicates the region included in the numerical analog. The selection of the size of this rectangle is dictated by the basic hydrodynamic features required to adequately represent the region and then the logistical and economic limitations placed on the computations by the availability of computer storage. The region selected is 56 x 40 nautical miles. The grid size (DELX) is taken as 2 nautical miles, so that IM = 28 and JM = 20.

Figure 13 is a contoured plot of the schematized topography superimposed on the selected grid system. The offshore topography is regular with the exception of a shallow region adjacent to Sabine Pass and a slight embayment lying between Sabine Pass and the outlet from Lake Calcasieu at Cameron. Both lakes are adequately represented by the grid interval of 2 nautical miles. Figure 14 clearly delineates three high topographic areas in the numerical model: the Beaumont rise in the northwest, the Orange rise, and a more gradual rise northeastward to the Lake Charles area. The low-lying region between the lakes, immediately behind the shoreline barrier, and forward of the rises, forms a large ponding area during the inundation sequences. Between each rise a major channel is present, the Neches River, the Sabine River, and in the Lake Charles region, the Calcasieu River runs northeastward from Lake Calcasieu.

The deepest block in the system is -24 feet (MSL). Assuming a 10-foot surge, a value of DELT equal to or less than 260 seconds (Sec. III, 1,b) is required. The value chosen for DELT is 240 seconds.

## 2. Channel and Barrier Schematization.

The numerical discretization of the area shown in Figure 12 is given as an overlay in Figure 15. In this illustration the channel network (shown by full lines) shows the landward interconnection of Sabine and Calcasieu as well as the link with the Intracoastal Waterway as the lower left- and right-hand channels. Each channel segment has been provided the physical characteristics of width and cross-sectional area that best reproduce the pertinent information for the channel reach that was provided by the Corps of Engineers. The extent of the channel system was chosen on the basis of past inundation history and the judgment of the authors.

The barrier system, also shown in Figure 15, represents the major manmade and natural obstructions to flow above MSL. At the shoreline the major dune line is continuous with the exception of an apparent open area east of Sabine Pass. The block elevation of that area equals the adjacent barrier heights. Jetties are included at each of the openings to the Gulf of Mexico. Within the region the majority of barriers are manmade levees erected for protection. The heights of all barriers were chosen on the basis of data provided by the Corps of Engineers.

Appendix D has a listing of all data used for the Sabine-Calcasieu region in the simulation of the Hurricane Carla surge. The topography,

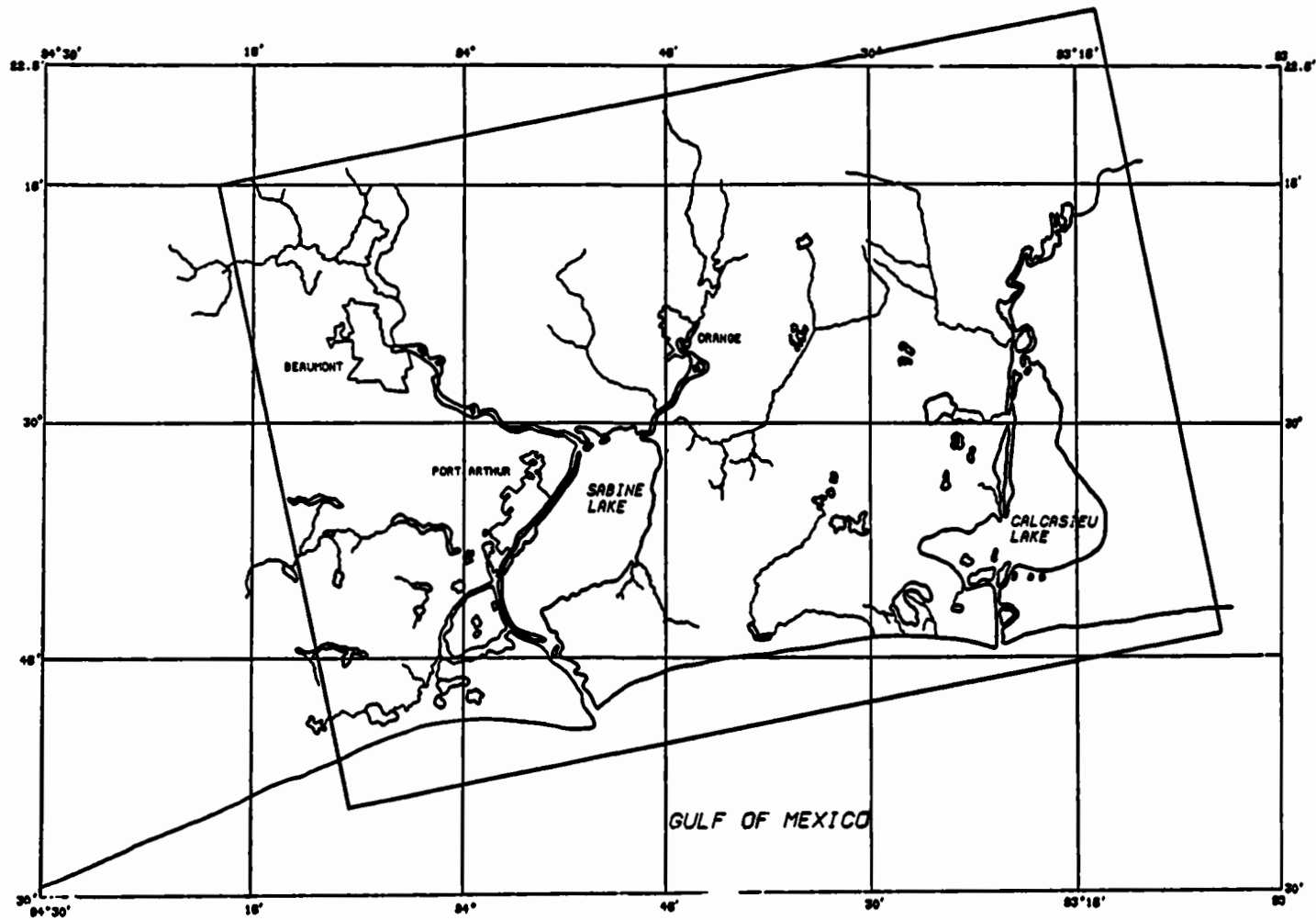


Figure 12. Map of Sabine-Calcasieu region showing grid boundary.

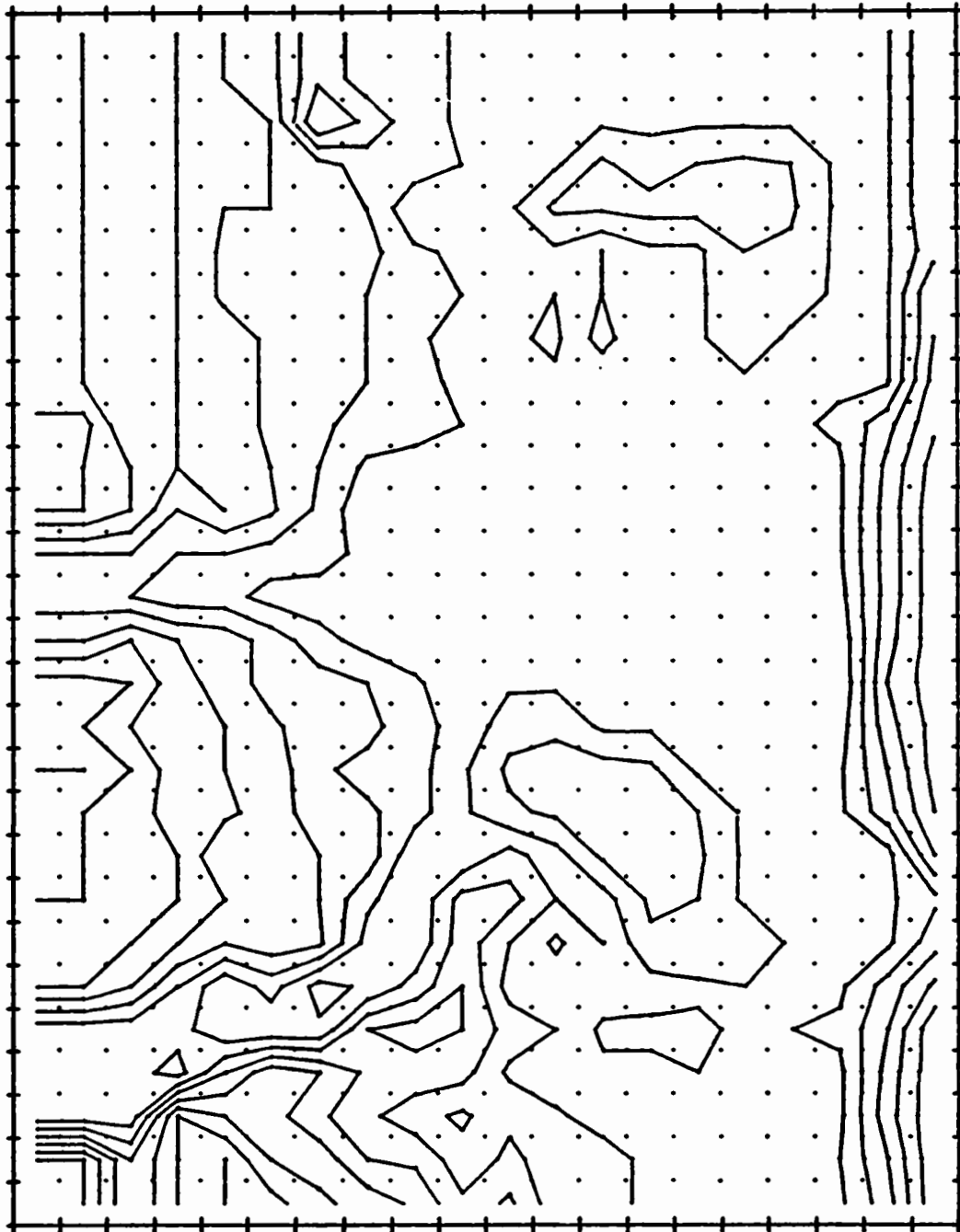


Figure 13. Topography contours at 5-foot intervals for Sabine-Calcasieu region (broad unoutlined area between Lakes Sabine and Calcasieu has elevations between 0 and 5 feet).

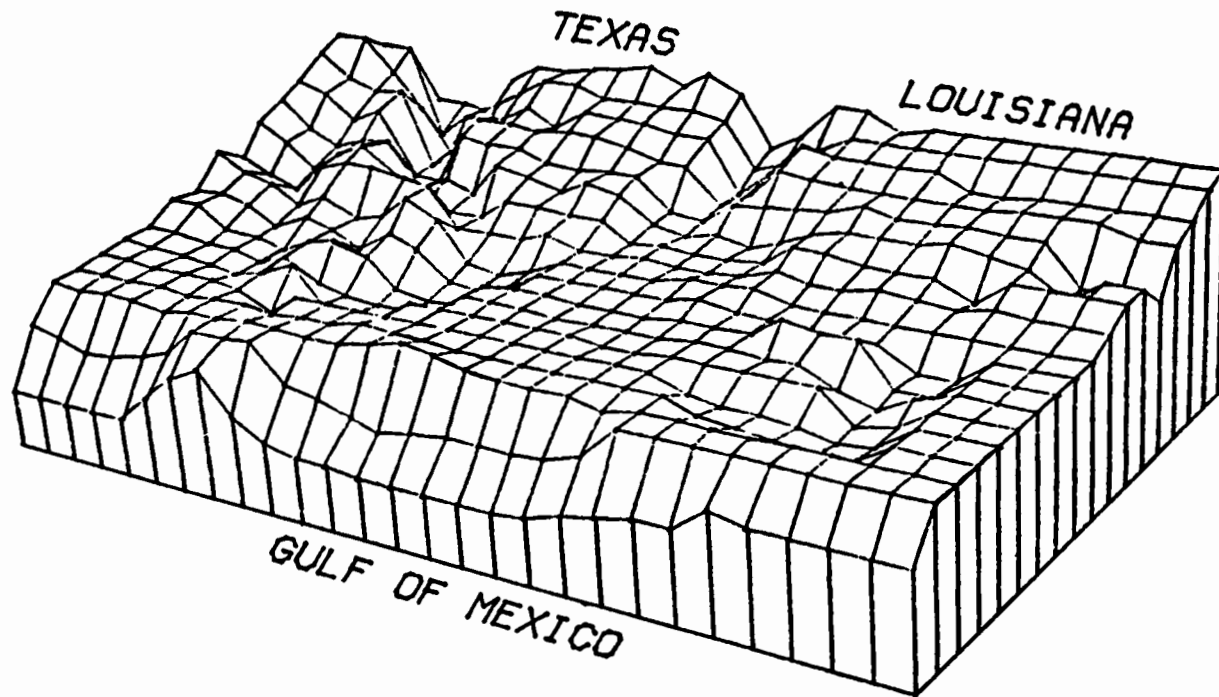


Figure 14. Topography in perspective for the Sabine-Calcasieu region.

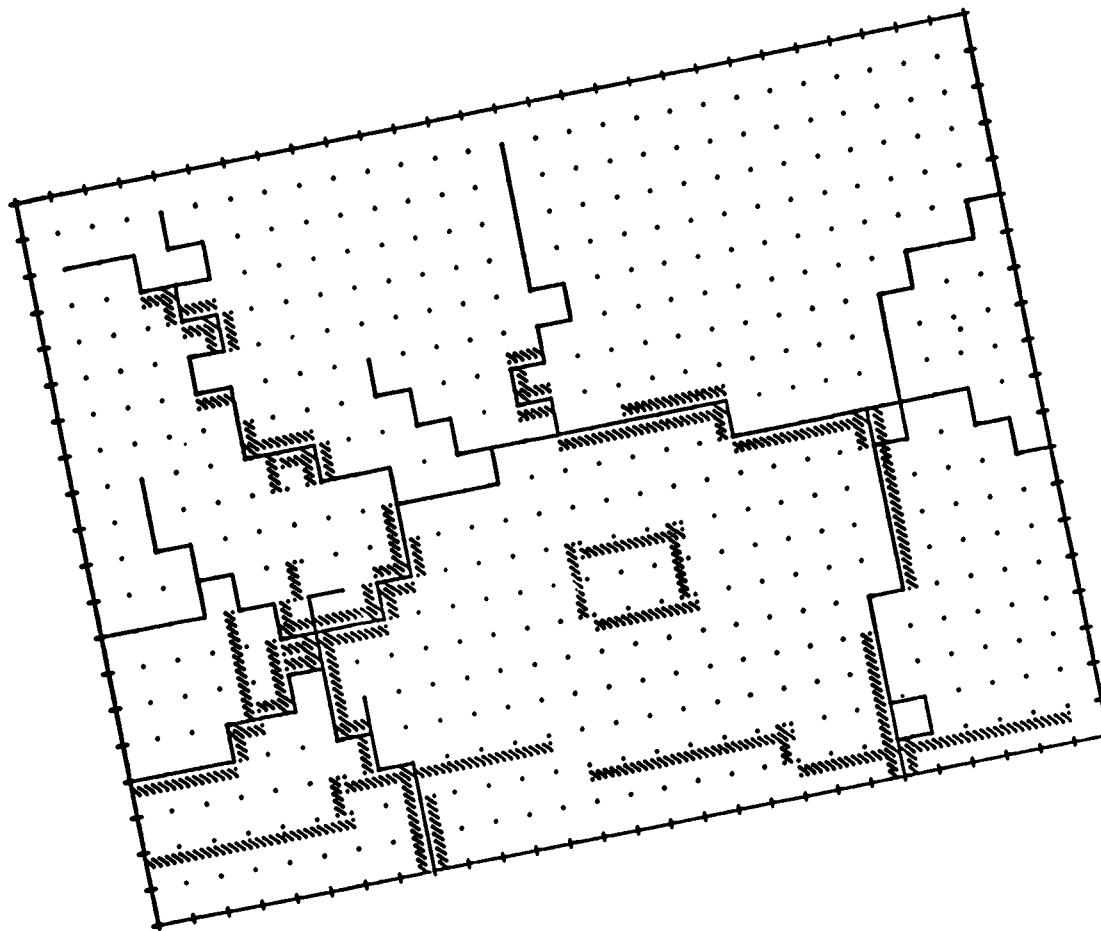


Figure 15. Overlay of grid (dots), channels (full lines) and barriers (hatched) on scale of Figure 12.

barrier data, and channel data are the same for the astrotide simulations and the standard project storms. There are 91 barrier blocks and 121 channel blocks of which 53 are common to barrier blocks. Examples of null channel blocks are for  $K = 4, 6, 18, 21, 24$ , etc., a total of 19.

Appendix E shows a plot of the block topography with the channel and barriers superimposed. This plot is given on two pages;  $x$  (or  $I$ ) runs from left to right and  $y$  (or  $J$ ) runs from bottom to the top of the page ( $I$  values are indicated along the top of both pages and  $J$  along the left side of the first page). Also in Appendix E is a listing of the key arrays for channels as generated by the program. Note that the final array size for channels is 128 (KCOMP), there being 6 channels which terminate on the boundary of the grid.

As an illustration of barrier input note from Appendix E that for block (2,2) a  $y$  barrier exists, but not an  $x$  barrier. The bed elevation of block (2,2) is -10 feet while that of block (3,2) is -13 feet. Thus, a value of  $ZX$  of -10 feet should have been input for this block. The listing of the barrier input data in Appendix D gives the information for block (2,2) at  $K = 12$  with  $ZX = -100$  (tenths of feet) which checks. The actual barrier on the upper side indicates a positive 6 feet. However, barrier block  $K = 13$  at the adjacent block (3,2) shows a  $ZX$  value of -12 feet. Reference to the topography in Appendix E indicates that this is the elevation of adjoining block (4,2) which is higher than block (3,2) and hence is the correct entry.

For an illustration of the sign coding concerning barriers along channels, refer to the channel input data in Appendix D and the plot in Appendix E. Channel block  $K = 1$  located at (8,1) shows a negative  $IWCX$  and a negative  $IZCX$  which is the coding for double levees of equal height with the channel in between. This is the location of the double jetty entrance channel for the Sabine region. Channel block 5 at location (7,4) shows a (+,-) signature for the  $x$  channel and a (+,+) signature for the  $y$  channel. Hence, the barrier for the  $x$  channel is on the inner lateral boundary while that for the  $y$  channel is on the outer lateral boundary (see App. C,6). Reference to Appendix E key array listings shows  $KCB = 37$  for channel block 5. Barrier block 37 has the same location (7,4) and indicates valid barriers of a 5-foot elevation above MSL for both the  $x$  and  $y$  channels.

### 3. River Input and Hydrograph Gage Locations.

There are three river discharge locations provided for the Sabine-Calcasieu region. These locations, as given in block 9 of the input (App. D), are (28,15), (4,19), and (14,19) which are respectively for the Calcasieu River near Lake Charles, the Neches River north of Beaumont, and the Sabine River north of Orange.

Nine gage locations for the astrotide calibration and Hurricane Carla simulation are shown as small circles in Figure 16. All of these with the exception of the North Sabine Lake gage are located on channels.

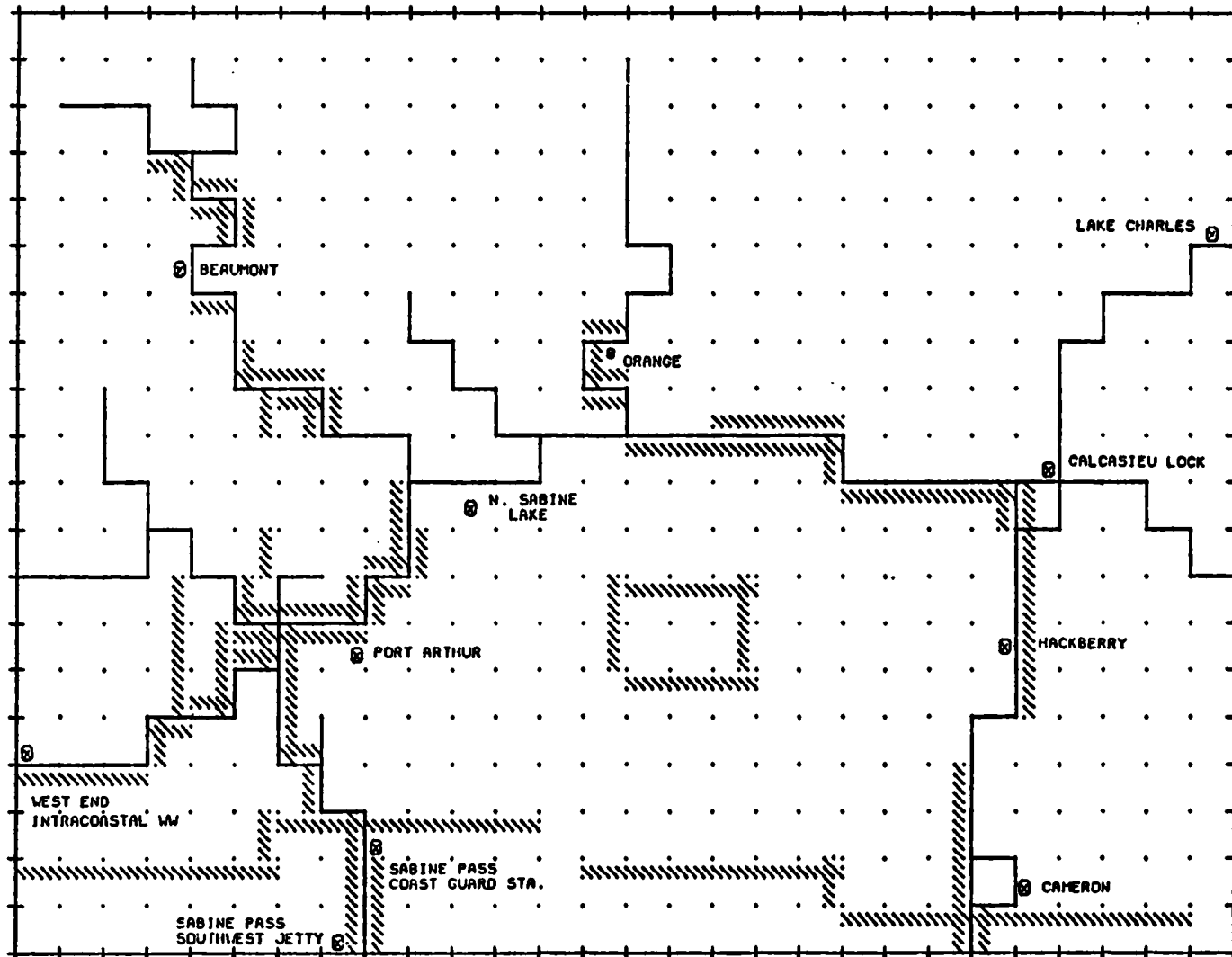


Figure 16. Plot of channels and barriers showing locations of hydrograph gages (⊙).



## V. TIDAL CALIBRATION

### 1. Tide Data.

The tidal calibration is a required step in the preparation of the numerical storm surge model. These computations permit the adjustment of the parameters representing the frictional effects in the channels and low-lying regions of tidal inundation. The calibration adjusts the tidal flows in order to adequately predict proper phasing and tidal excursions in the model region. Comparisons are made with actual tide records from geographical locations corresponding to blocks or channels in the grid.

Calibration of the Sabine-Calcasieu region was carried out for the springtide conditions that existed from 0000 hours, 22 August to 0000 hours, 27 August 1973. Tide recordings at nine locations in the region were furnished by two U.S. Army Engineer Districts (Fig. 16): Sabine Pass (southwest jetty), Port Arthur, north Sabine Lake, Brakes Bayou (Beaumont), and Orange, Texas, provided by the Galveston District; Cameron, Hackberry, Calcasieu Lock, and Lake Charles, Louisiana, provided by the New Orleans District.

The tidal calibration must be accomplished during a period when the tide is effectively the only forcing function operating on the system. This requires no abnormal riverflows into the region and winds which will not substantially alter the slope of the water surfaces. Such conditions existed for the first 96 hours of the 120-hour record period and this interval was used in the tidal calibration.

### 2. Estimation of $f_c$ for Entrance Channels.

Many of the bays or lagoons along the Texas coast are of such dimensions that their largest natural period is small compared with the tide period. Moreover, virtually all have narrow connections with the Gulf of Mexico. These two features conspire to produce a reduction of tidal range and a significant lag within the bay compared with the gulf tide. In addition, the tidal range is nearly uniform throughout the bay except possibly in some of the upper reaches of adjoining rivers. For these systems, the approximate response can be calculated in terms of the channel-friction coefficient,  $f_c$ , (or discharge coefficient) plus appropriate dimensions of the bay and entrance channel (Love, 1959). These relations can be used to get at least a preliminary estimate of  $f_c$  from the observed response.

Consider a bay of total MSL surface area,  $A_s$ , which is connected to the sea by a channel of cross-sectional area  $A_c$ , surface width  $W$ , effective depth  $D_c$  (defined as  $A_c/W$ ), length  $L_c$ , and channel-bed friction coefficient  $f_c$ .

Let  $H$  be the volumetric response in the bay at time  $t$  (where  $H \cdot A_s$  represents the impounded tidal volume above MSL at time  $t$ ); let  $Q$  be the tidal flux from the sea to the bay. Then,

$$A_s \frac{dH}{dt} = Q \quad (68)$$

Neglecting the inertia effects in the channel for the slow tidal variation, the slope force in the channel is balanced by friction at any time  $t$ ; thus,

$$H_g - H = m|Q|Q \quad (69)$$

where  $H_g$  is the given tide level at time  $t$  outside the bay entrance and  $m$  is a dimensional constant for the system given by

$$m = \frac{fL}{gD_c A_c^2} \quad (70)$$

This can also be written in the form,

$$m = \frac{1}{g(C_d \cdot A_c)^2},$$

where  $C_d$  is the discharge coefficient characterizing the constricted opening between bay and sea.

Assuming the input tide  $H_g$  is simple harmonic with period  $T$  and amplitude  $a_0$  then,

$$H_g = a_0 \cos \omega t \quad (71)$$

where  $\omega = 2\pi/T$ . Ignoring the second-order compound tide due to non-linearity in equation (69), the response will be roughly of the form,

$$H = r a_0 \cos (\omega t - \phi) \quad (72)$$

where  $\phi$  is a phase lag and  $r$  is the relative amplitude response. If these are substituted into equations (68) and (69) and the quantity  $|Q|Q$  expanded in the Fourier series form, it can be shown that

$$r = \cos \phi \quad (73)$$

and

$$\sin \phi = \frac{\sqrt{1 + B^2} - 1}{B} \quad (74)$$

where

$$B = \frac{8}{3\pi} m(A_s \omega)^2 a_0 \quad (75)$$

(Love, 1959). A plot of  $r$  and  $\phi$  versus the dimensionless parameter  $B$  is shown in Figure 17. The timelag of the high tide in the bay relative to that outside the bay is simply  $T = \phi/2\pi$ , for  $\phi$  in radians (or  $T = \phi/360$  for  $\phi$  in degrees).

Thus, if  $r$  or  $\phi$  is estimated from observations it is possible to get an estimate of  $B$ . Generally, the value obtained from the observed  $r$  will differ from that obtained from the observed  $\phi$ ; in this event an average of the values of  $B$  can be used to estimate  $f_c$ . In terms of  $B$ ,  $f_c$  is given by

$$f_c = \frac{3\pi}{8} \frac{gB}{\alpha^2 A_s^2 \omega^2 a_0} , \quad (76)$$

where

$$\alpha^2 = \frac{L_c}{D_c A_c^2} . \quad (77)$$

It is emphasized that the above analysis pertains to a bay system connected to the sea by a single channel of uniform dimensions. The results can be generalized for the case of a series of  $N$  channels of different dimensions or of  $N$  channels in parallel or combination of both (as in the Sabine-Calcasieu system) by using an effective value of  $\alpha^2$ .

Let  $\alpha_n^2$  designate the value of  $\alpha^2$  for an individual channel as evaluated by equation (77). Then, the effective value of  $\alpha^2$  for a series of  $N$  channels is simply

$$\alpha_s^2 = \sum_{n=1}^N \alpha_n^2 . \quad (78)$$

However, for  $N$  channels in parallel the effective  $\alpha^2$  is given by

$$\alpha_p^2 = \left( \sum_{n=1}^N \alpha_n^{-1} \right)^{-2} \quad (79)$$

For a series containing a parallel subset, the effective  $\alpha^2$ , for the latter is used in equation (78). If two or more complex entrance channels are in parallel then the effective values of  $\alpha$  are used in place of  $\alpha_n$  in equation (79).

The use of this procedure will be illustrated for the Sabine-Calcasieu system. In the numerical simulation scheme there is a total of 40 blocks of  $2 \times 2$  nautical miles covered with water and in communication with the sea. This represents a surface area of  $5.91 \times 10^9$  square feet. In addition, the channels contribute a total of  $0.64 \times 10^9$  square feet. Thus, the total surface area for the combined system is  $A_s = 6.55 \times 10^9$  square feet ( $3.77 \times 10^9$  square feet for the Sabine part and  $2.78 \times 10^9$  square

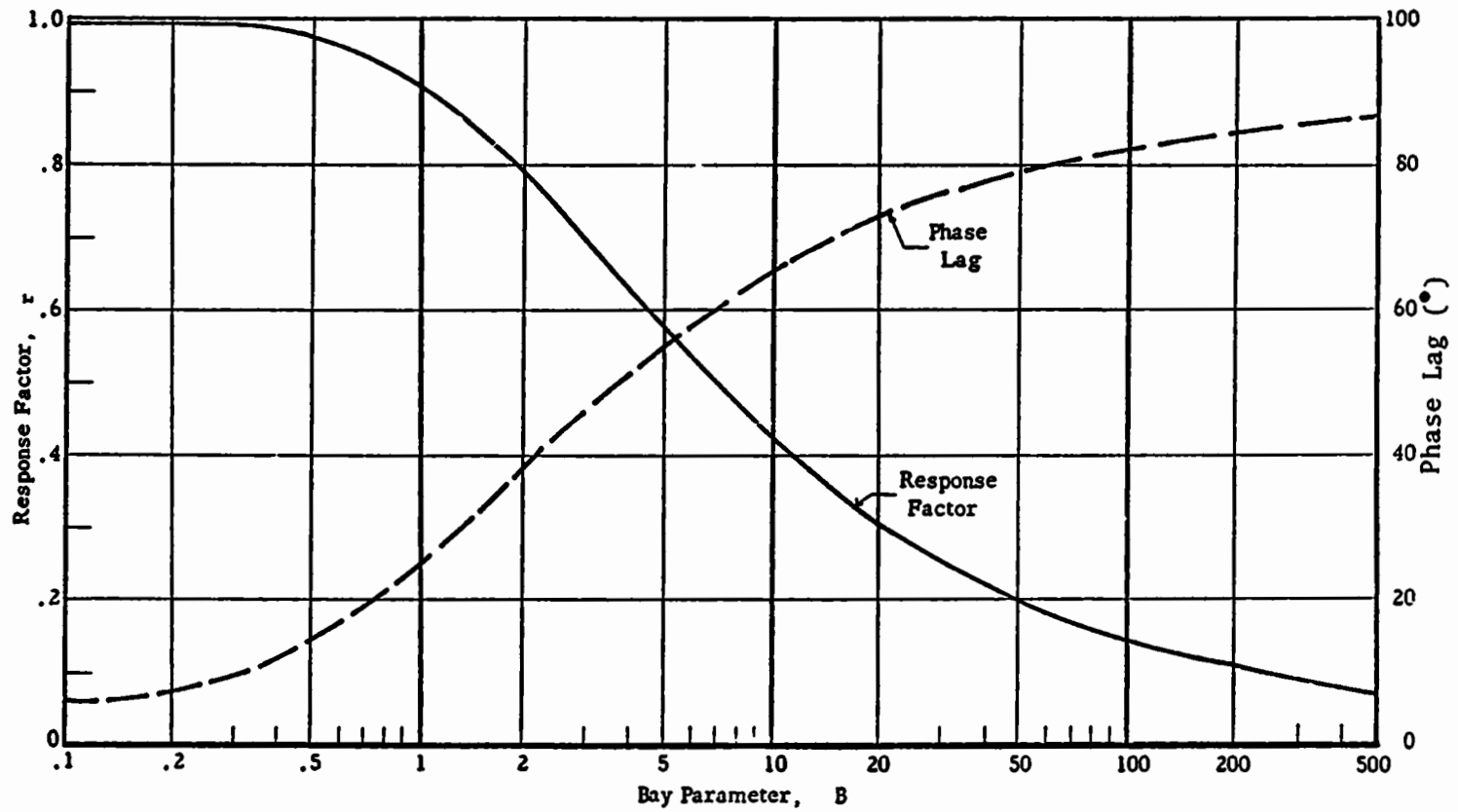


Figure 17. Amplitude response factor ( $r$ ) and phase lag ( $\phi$ ) versus the dimensionless parameter  $B$  characterizing a constricted bay.

feet for the Calcasieu part). The two parts of the system are coupled via the Intracoastal Waterway and their responses are about the same, so the combined system is treated as one.

A summary of data and calculations pertinent to the entrance channels for the Sabine-Calcasieu system is given in Table 2 (see also Fig. 15 and App. D). The simulated Sabine Pass between the gulf and Lake Sabine consists of two sections (1 and 2 in Table 2) of different dimensions in series. However, Calcasieu Pass consists of a pair of parallel channels (4 and 5 in Table 2) in series with a simple channel (3 in Table 2). The individual  $\alpha^2$  for each channel is also shown in Table 2. The effective  $\alpha^2$  for Sabine Pass is the first partial sum shown in the last column. The effective value of  $\alpha^2$  for the parallel part of Calcasieu Pass is shown in the last column, opposite entries 4 and 5. The effective value for Calcasieu Pass is the partial sum indicated in the last column. The effective value for the entire pass system is evaluated from the Sabine Pass and Calcasieu Pass values, using equation (79) for parallel systems:

$$\alpha^2 = 0.32 \times 10^{-6} \text{ (square feet)}^{-1} .$$

Table 2. Data on simulated Sabine Pass and Calcasieu Pass.

n	$W_c$ (ft)	$D_c$ (ft)	$A_c$ (ft <sup>2</sup> )	$L_c$ (ft)	$\alpha^2 \times 10^6$ (ft <sup>-2</sup> )	$\alpha^2 \times 10^6$ (ft <sup>-2</sup> )
Sabine Pass						
1	2,330	20	46,600	24,360	0.561	0.561
2	2,860	21	60,060	36,480	0.482	0.482
Subtotal						1.043
Calcasieu Pass						
3	800	32	25,600	24,360	1.162	1.162
4	500	40	20,000	12,160	0.760	0.455 <sup>1</sup>
5	1,000	16	16,000	34,480	8.960	
Subtotal						1.617

<sup>1</sup>Evaluated by parallel channel relation.

The observed ranges and times of minimum tide for 25 August 1973 for the Sabine-Calcasieu system are given in Table 3. Gage 1 is used as the input gulf tide. The average of all other gages is used as the response. The indicated amplitude response is

$$r = \frac{1.50}{2.59} = 0.58 .$$

Using a tidal period of 25 hours the indicated phase lag is

$$\phi = (20.8 - 17.5) \frac{360}{25} = 47^\circ .$$

Table 3. Ranges and times (c.d.t) of available observed tides in the Sabine-Calcasieu system for 25 August 1973.

Gage No.	Place	Range (ft)	Time (hr)
1	Sabine Pass, southwest jetty	2.59	17.5
2	Port Arthur	1.53	19.0
3	North Sabine Lake	1.40	21.5
4	Beaumont	1.52	21.5
5	Orange	1.40	23.0
6	Cameron	2.05	17.5
7	Hackberry	1.06	22.0
8	Calcasieu Lock, west	1.45	20.5
9	Lake Charles	1.60	21.5
Average of 2 to 9, inclusive		1.50	20.8

From Figure 17 the corresponding values of B are 4.7 and 3.6, respectively, with an average of 4.1. The tidal frequency is

$$\omega = \frac{2\pi}{25 \times 3,600} = 7.0 \times 10^{-5} \text{ radians per second}$$

and  $a_0 = 2.57/2$  or 1.3 feet. Consequently, the estimated  $f_c$  for the entrance channels is from equation (76):  $f_c = 0.0018$ .

The final selected value of  $f_c$  for the entrance channels is 0.0015 as determined by trial runs. This is somewhat less than the above estimate. The difference might be accounted for by the fact that the tidal hydrograph is not really simple harmonic but contains compound tides (of higher frequency) giving the sharp minimum and broad or double-peaked maxima. The effective frequency is consequently somewhat greater than the  $\omega$  given above, thus yielding a smaller  $f_c$  closer to 0.0015.

### 3. Final Calibration for Tide.

The major control on the response of the bay to the tides are the dimensions and friction factor for the entrance channels as discussed above. In this connection, it should be pointed out that channel dimensions (width and depth) were taken such that the average cross-sectional area (under MSL conditions) for a given reach is represented by the product of these dimensions. Thus, if the depth is taken as the mean for the reach, then the width will be somewhere between the width of the dredged channel and the surface width of the natural channel.

The values of channel friction for the remaining channels and of the block friction were selected by a trial-and-error procedure, starting with a uniform value throughout. The final values of channel friction for the upper reaches of the Neches and Sabine Rivers were taken as

0.0025 to give a reasonable agreement for the Beaumont and Orange tide response; it was necessary to use a low value (0.0005) for the upper reach of the Calcasieu River to reproduce the Lake Charles tidal hydrograph. The latter three gages (Beaumont, Orange, and Lake Charles) have connections to the inner bay areas only via channels, hence their responses are fairly sensitive to the channel friction. The low value for the Calcasieu River may be due to underestimates of the effective channel widths, which would demand a less than normal friction factor.

The block friction for the tide calculations was taken as 0.0015 to get a reasonable agreement for the north Sabine Lake gage. However, later calculations for the Hurricane Carla simulation (which is more sensitive to block friction than the astrotide) indicated that 0.0025 (as used in the Galveston Bay simulations) was more appropriate.

The results of the final astronomical tide simulations for a 96-hour period starting 0000 hours c.d.t., 22 August 1973, are given in Figures 18 to 26, and Appendix F. The input tide (Fig. 18) corresponds to the observed tide for the period at Sabine Pass (southwest jetty). In the subsequent eight figures the computed (full line) and observed (line with circles) are compared for the eight different gages within the system; the gages are identified in the figures. Note that the observed values for each gage have been adjusted with respect to a local datum, taken as the gage mean for a 120-hour period starting 0000 hours c.d.t., 22 August 1973. In all cases, the computed ranges are in fairly good agreement with the observed; however, there seems to be a consistent tendency for the computed to lag the observed. This might be due to a possible time-shift error for the input gage. Although a lowering of the frictional coefficient for the entrance channel would decrease the lag within the system, it would also increase the range of the tide everywhere in the system. It was felt that it was more important to reproduce the range than the times of high and low water, and hence the value of  $f_c = 0.0015$  for the entrance channels was retained.

For the upper Calcasieu River (Figs. 25 and 26) the computed water level (which refers to a common MSL datum for the system) and the observed water level display an apparent vertical shift. This could be related to possible wind effects in the second part of the record, which have been ignored in the computations.

The steady river discharges adopted in the astrotide runs were 800 cubic feet per second for Calcasieu River, 1,100 cubic feet per second for Neches River, and 1,500 cubic feet per second for Sabine River.

Serial listings of the computed water levels at the gages discussed above are given in Appendix F, along with listings of volume transport at six channel positions. Flow at points 1 and 2 correspond to input (if positive) to the system through Sabine Pass and Calcasieu Pass, respectively. Since the tide amplitude is less than the seaward barriers, the two passes represent the only source of water for normal conditions.

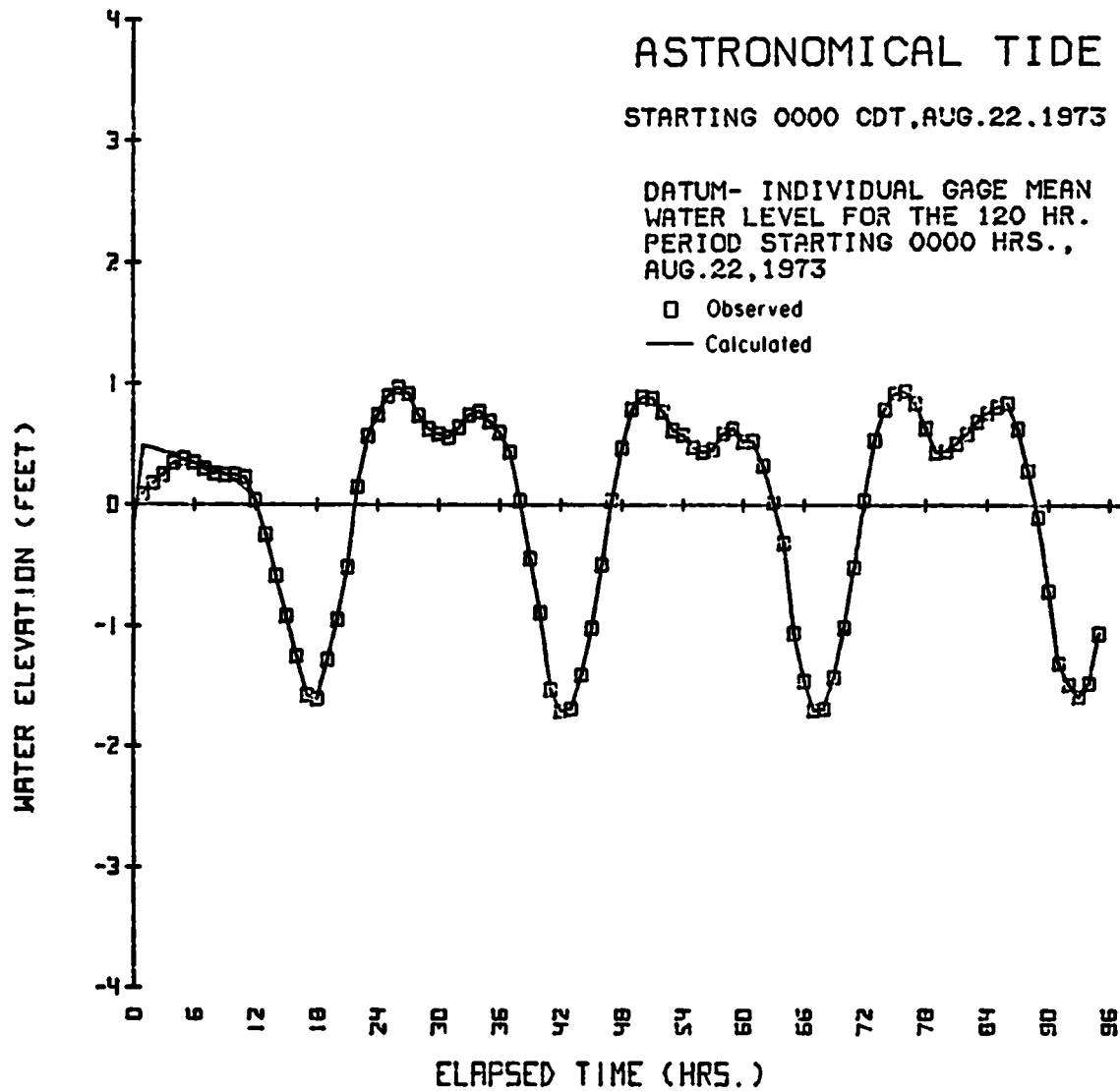


Figure 18. Astronomical tidal hydrograph for Sabine Pass, southwest jetty (input for tide calibration).



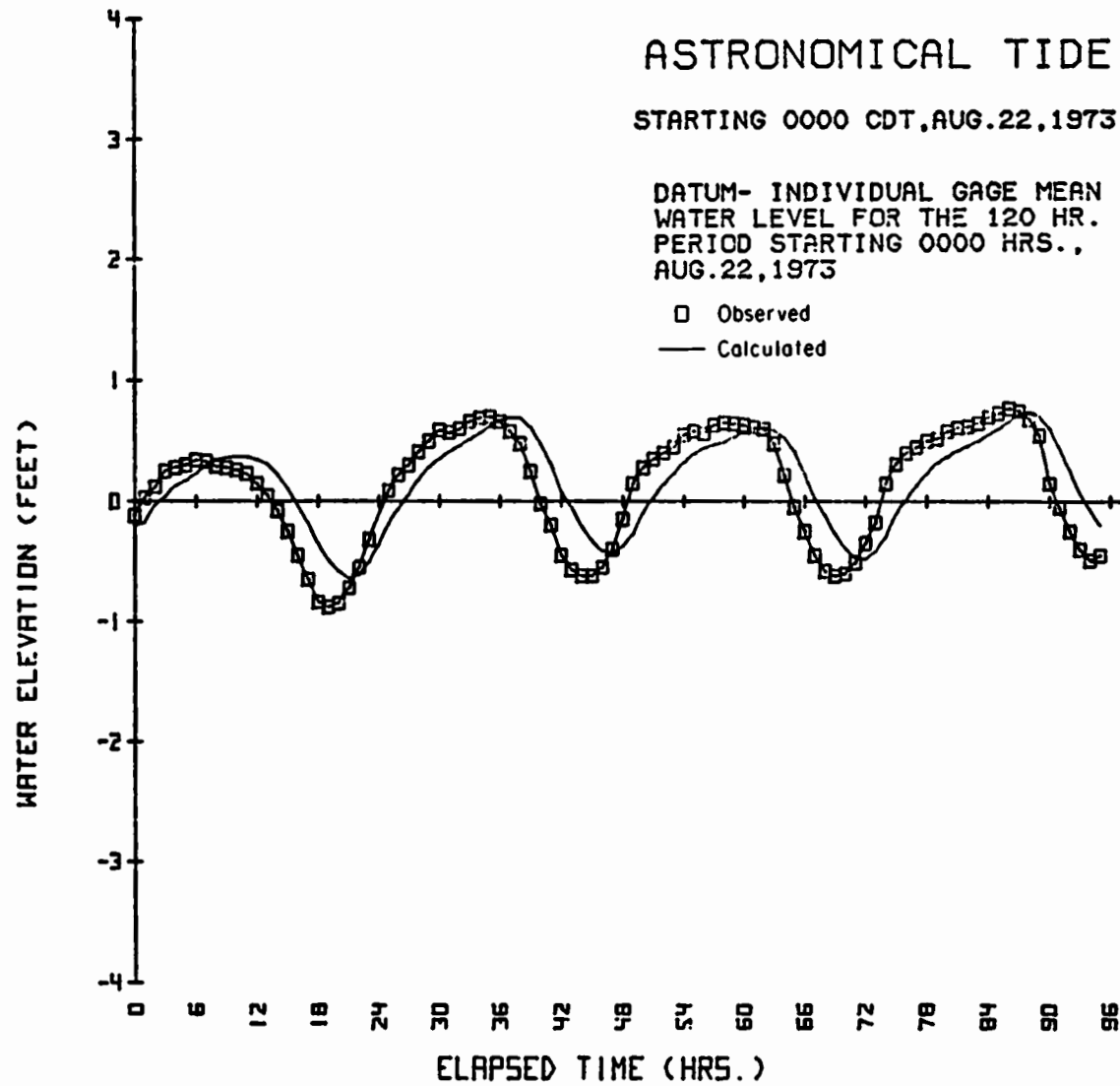


Figure 19. Astronomical tide for Port Arthur corresponding to input of Figure 18.

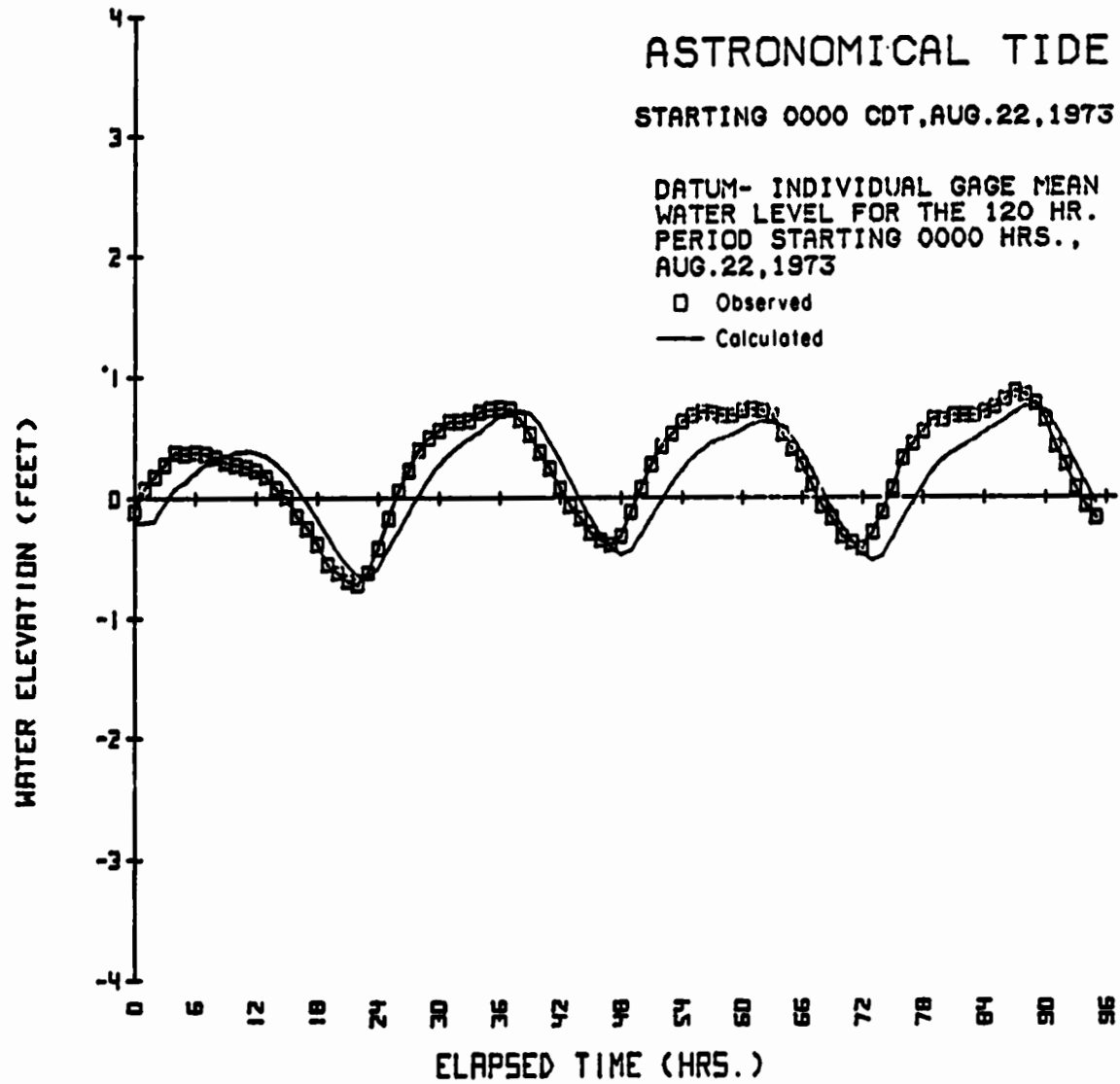


Figure 20. Astronomical tide for north Sabine Lake.

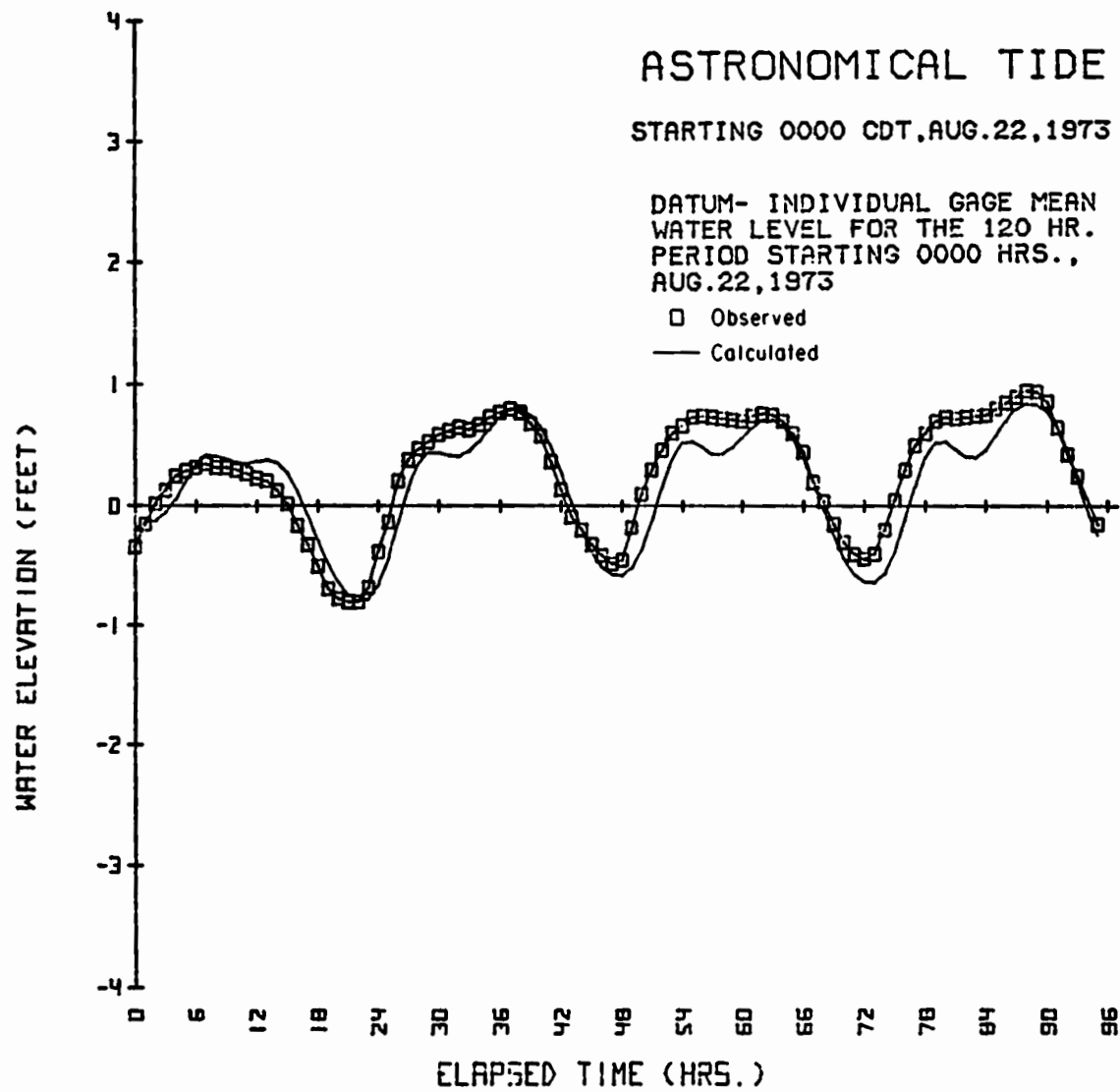


Figure 21. Astronomical tide for Beaumont, Neches River, and Brakes Bayou.

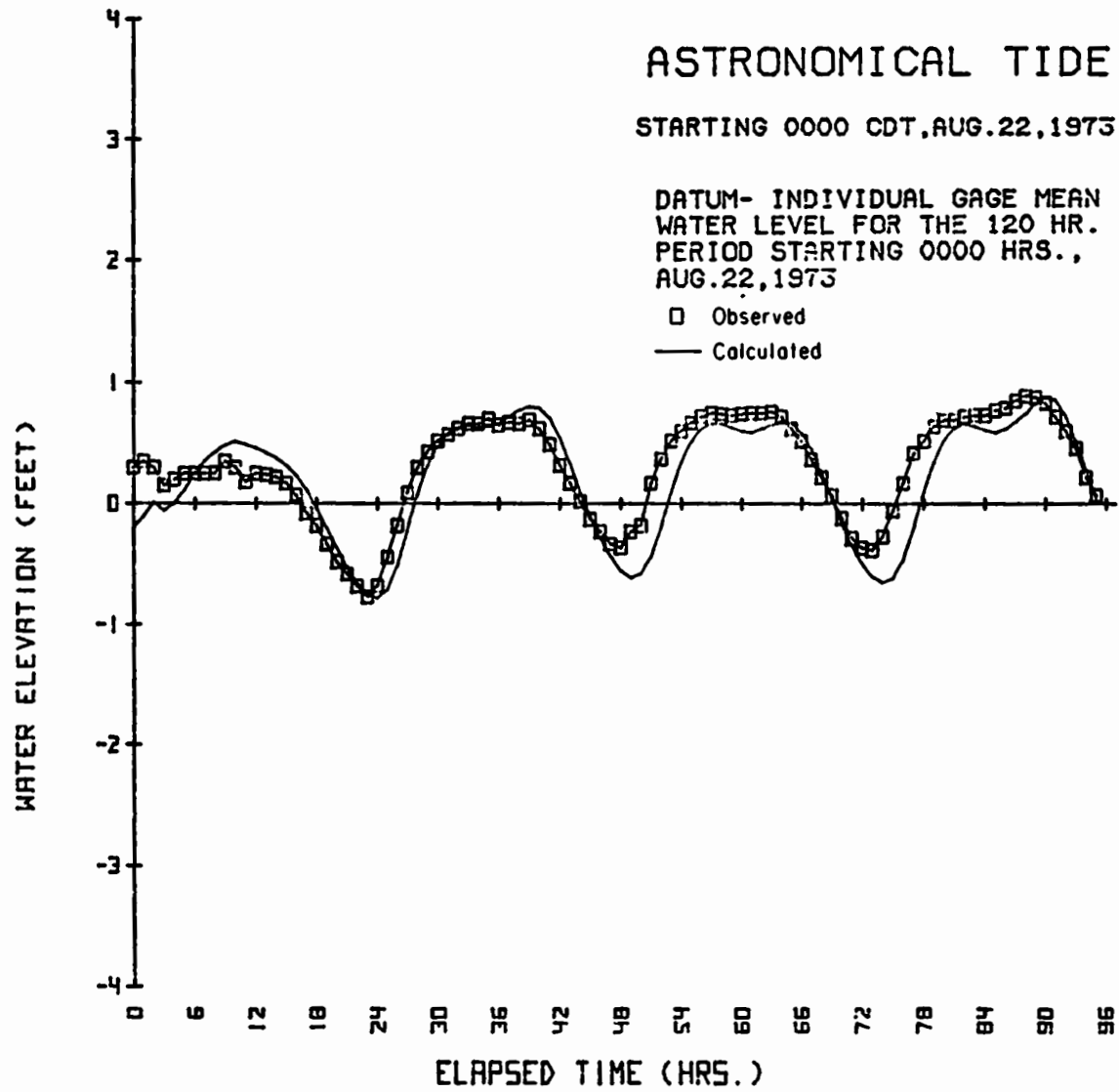


Figure 22. Astronomical tide for Orange Naval Station, Sabine River.

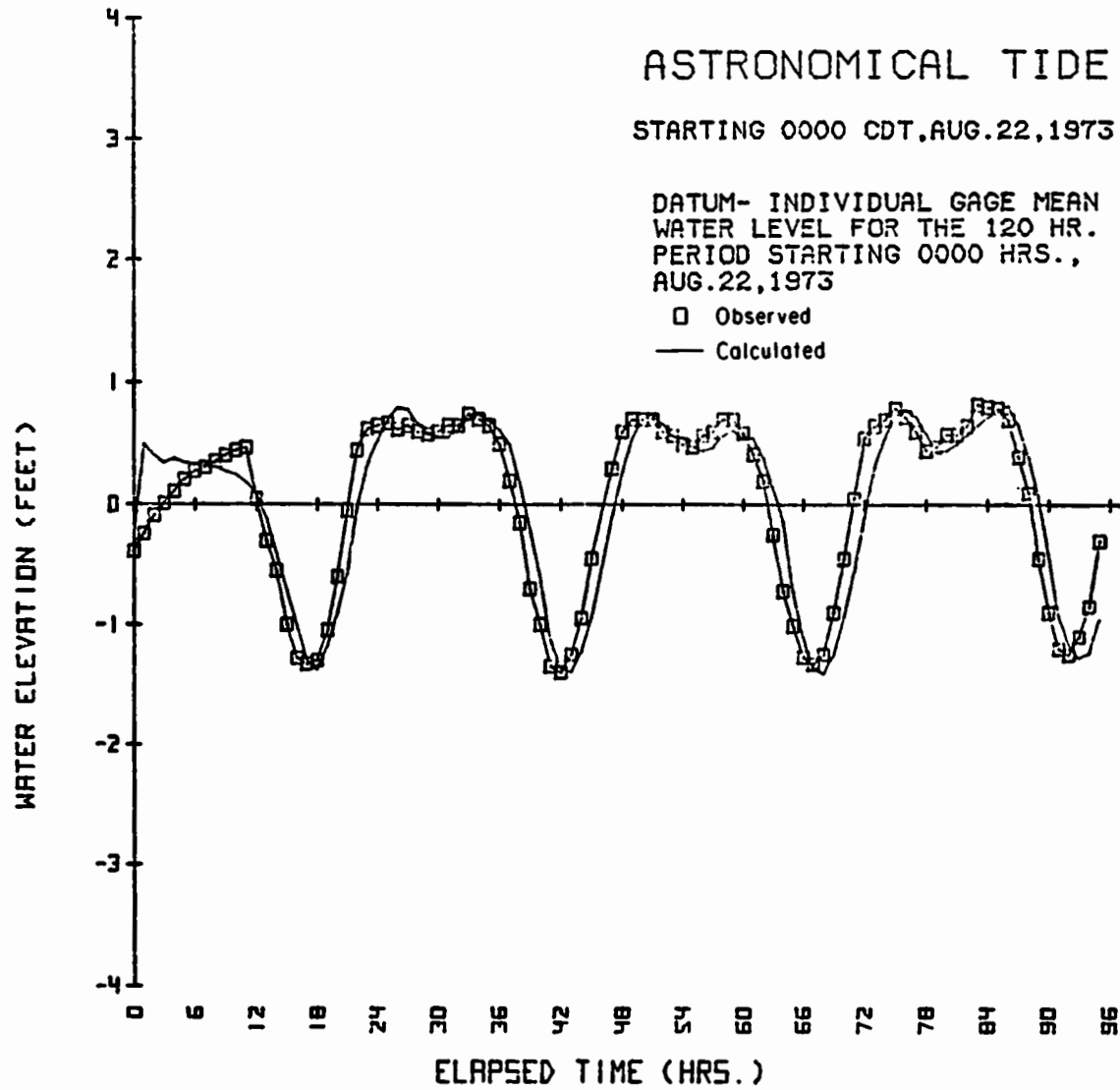


Figure 23. Astronomical tide for Cameron, Calcasieu Pass.

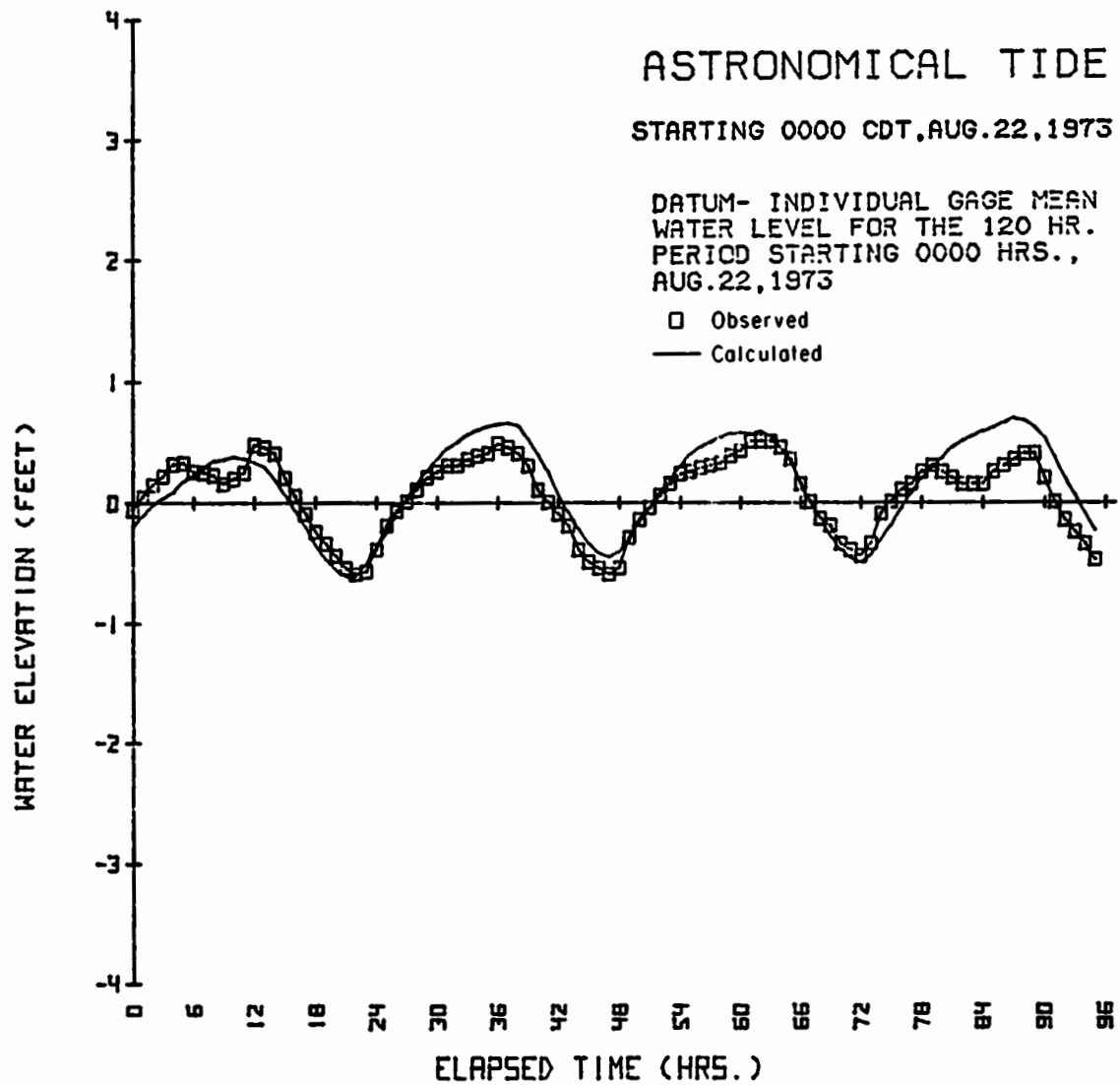


Figure 24. Astronomical tide for Hackberry, Calcasieu River and Pass.

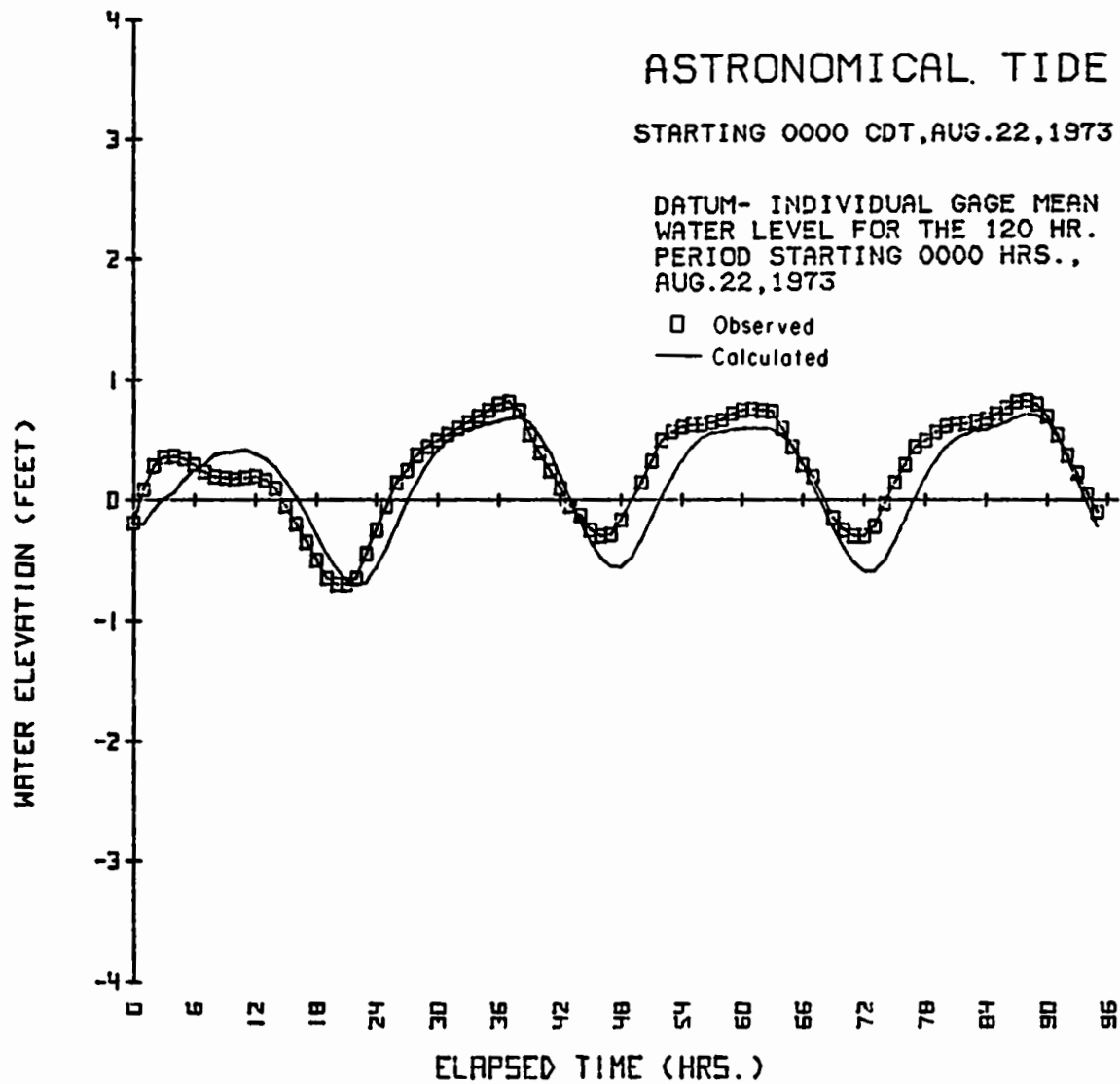


Figure 25. Astronomical tide for Intracoastal Waterway at Calcasieu Lock, west.

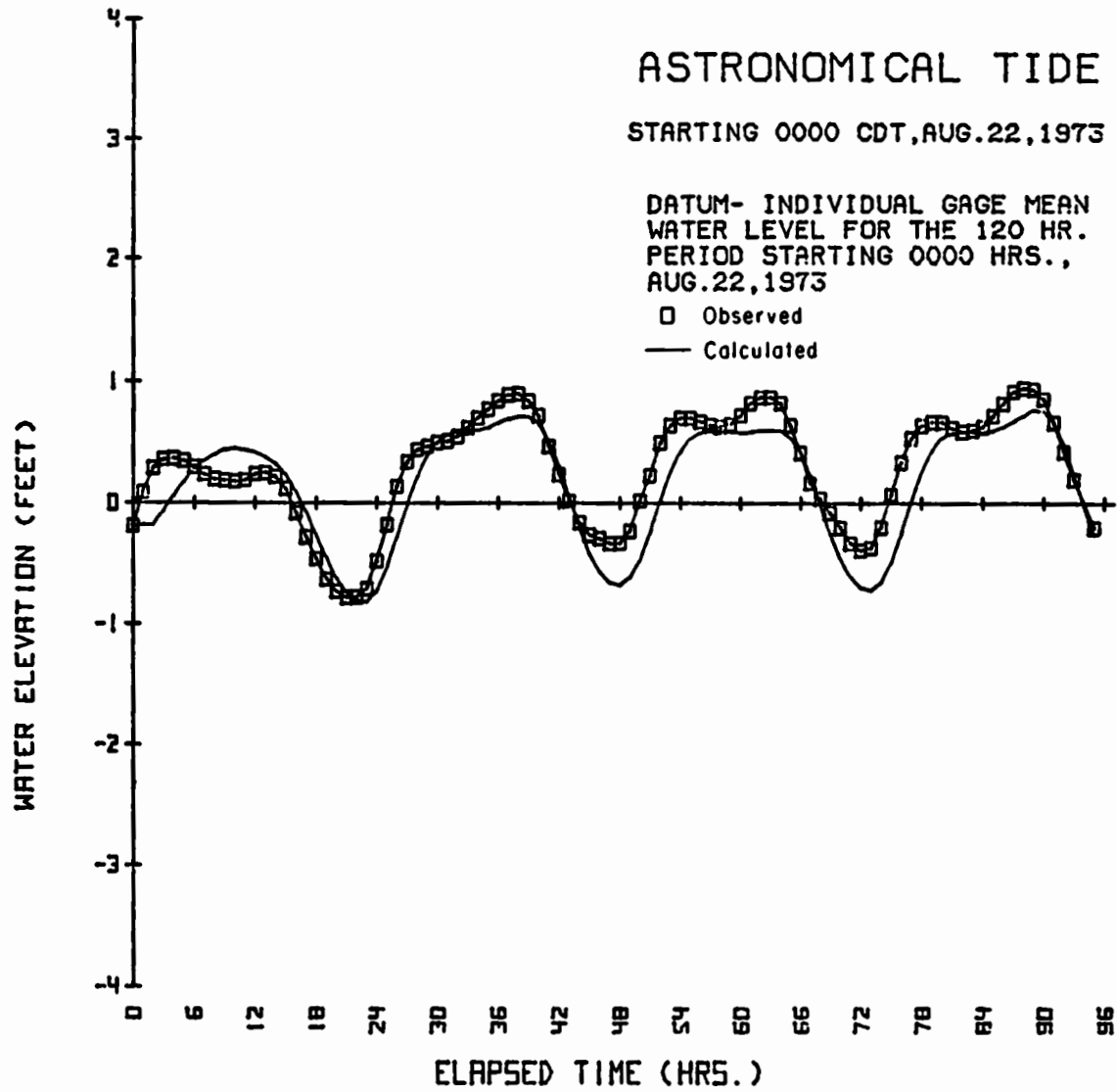


Figure 26. Astronomical tide for Lake Charles, Calcasieu River.



Reproductions of channel output at three different times (30, 60, and 90 hours from start) are shown in Appendix F. The output shows flows (in cubic feet per second), direction of flow, and water level along the various channel reaches at the specified times.

## VI. HURRICANE CARLA VERIFICATION

### 1. Forcing Function Input.

a. Wind-Stress Fields. The x and y components of the wind stress for each 3 hours in a 72-hour period for an 8 by 6 coarse grid for Hurricane Carla are given in the input listings in Appendix D. For convenience in spotting possible errors in input, the wind-stress vectors were plotted, based on the above input, by a special subprogram. Samples of these plots for each 12 hours are shown in Figures 27 to 32. The plots showed suspect entries, which were subsequently corrected before any runs were attempted, and have I increasing upward and J increasing to the left; i.e., the seaward boundary is on the right.

b. River Discharge Input. The river discharges for the Calcasieu River, Neches River, and Sabine River for each 3 hours are listed as block (IDENT) 12 in Appendix D.

c. Gulf Hydrograph Input. The final input for HG, the water level input along the seaward boundary, was taken as interpolated values between Sabine Pass and Calcasieu Pass with input sequences at those passes adjusted to match the observed values at the Sabine Pass U.S. Coast Guard Station and Cameron after some modification due to flow through these passes. The input is given sequentially at 3-hour intervals along with the wind-field input in Appendix D.

### 2. Further Adjustments and Results.

a. Adjustments. In the series of runs for the Hurricane Carla simulation, it was necessary to make some adjustments in the block topography, particularly in the upper reaches of the Neches River, in order to provide more ponding area at the levels of flooding encountered. These changes, which are reflected in the final topography (App. D), do not change the results of the astronomical tide calibration because the changes were at levels well above those encountered with the astro-tide runs.

A further modification was the reduction of the wind-stress values to 80 percent of those shown in the listings and in the vector plots for the upper left-hand region of the grid. Specifically for I.LE.3 and J.GE.4, the wind-stress components were so reduced in the final runs for Hurricane Carla. This reduction was also used in the later application for Standard Project Hurricane (SPH) simulations. The rationale for this adjustment is based on the greater sheltering in this region due to both topography and vegetation. The initial H for all locations in the bay was taken as 3.2 feet.

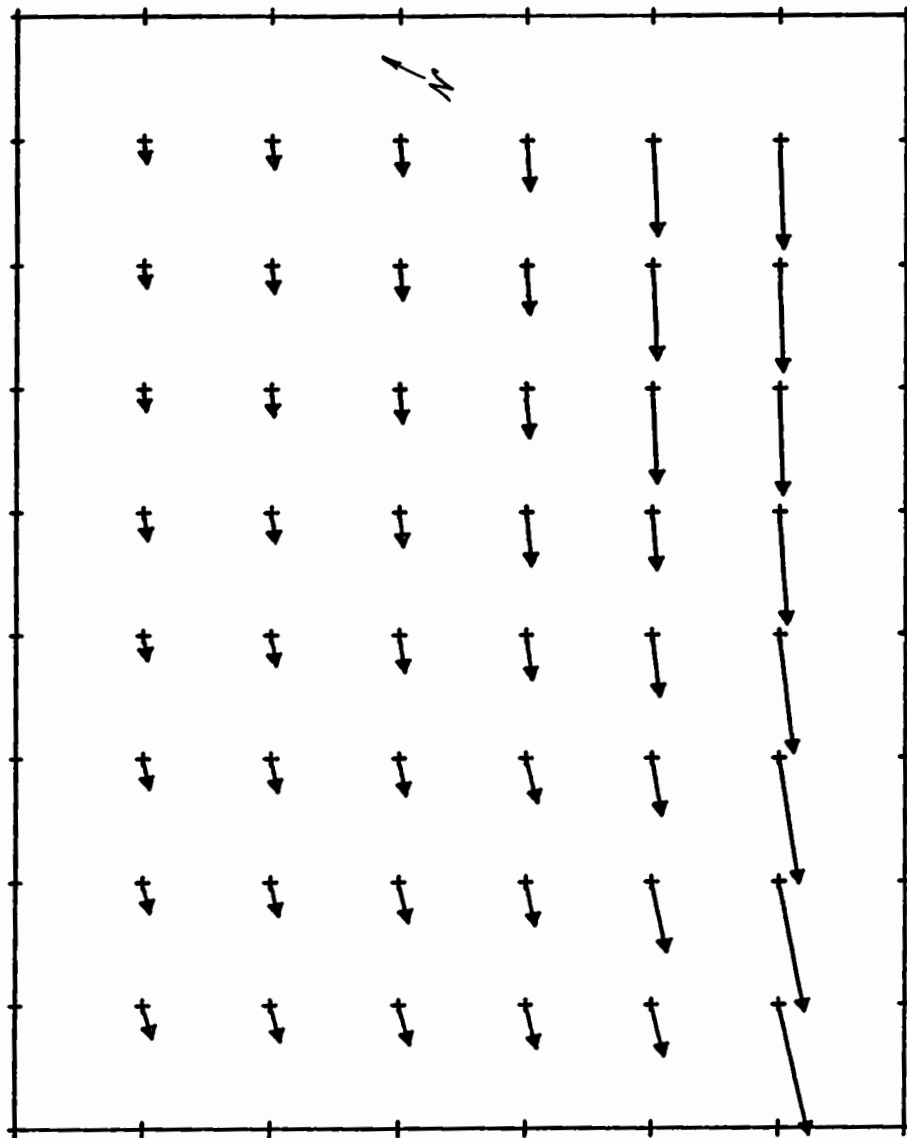


Figure 27. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 12 hours.

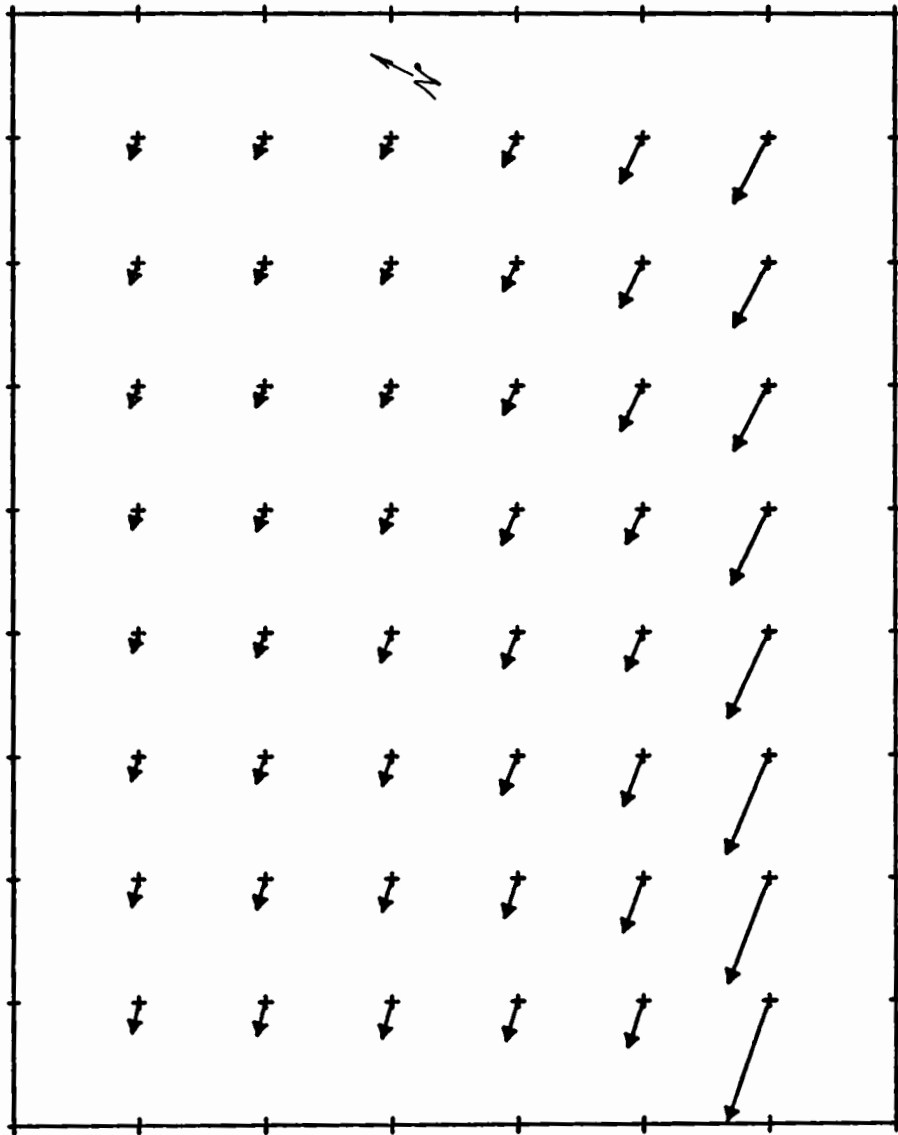


Figure 28. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 24 hours.

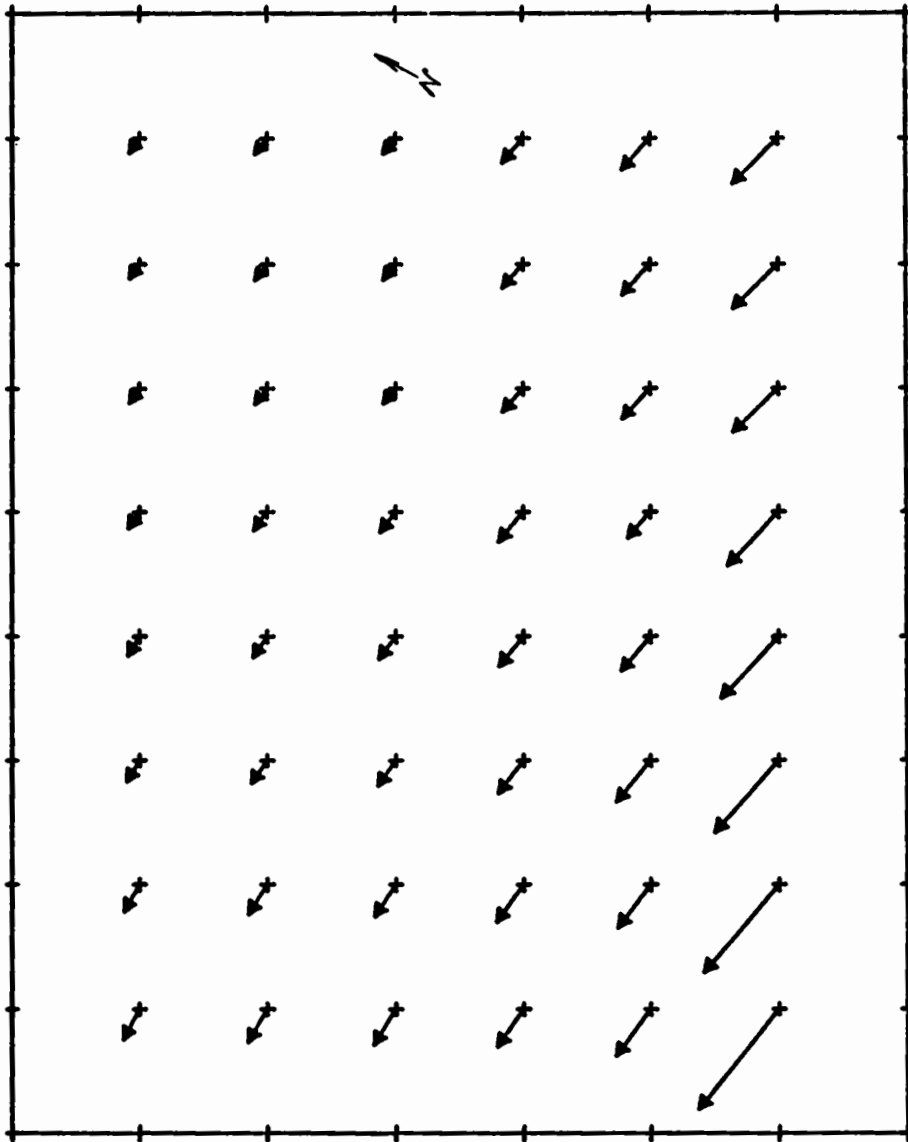


Figure 29. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 36 hours.

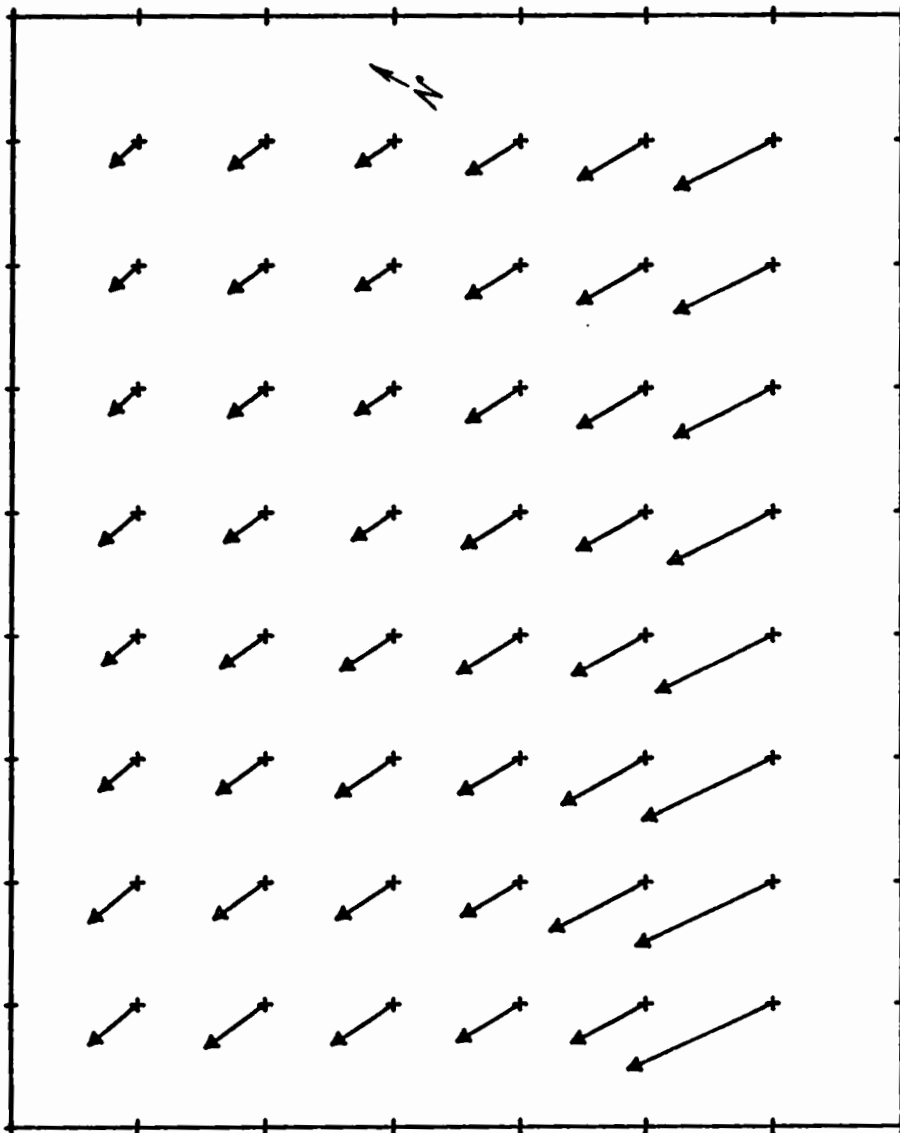


Figure 30. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 48 hours.

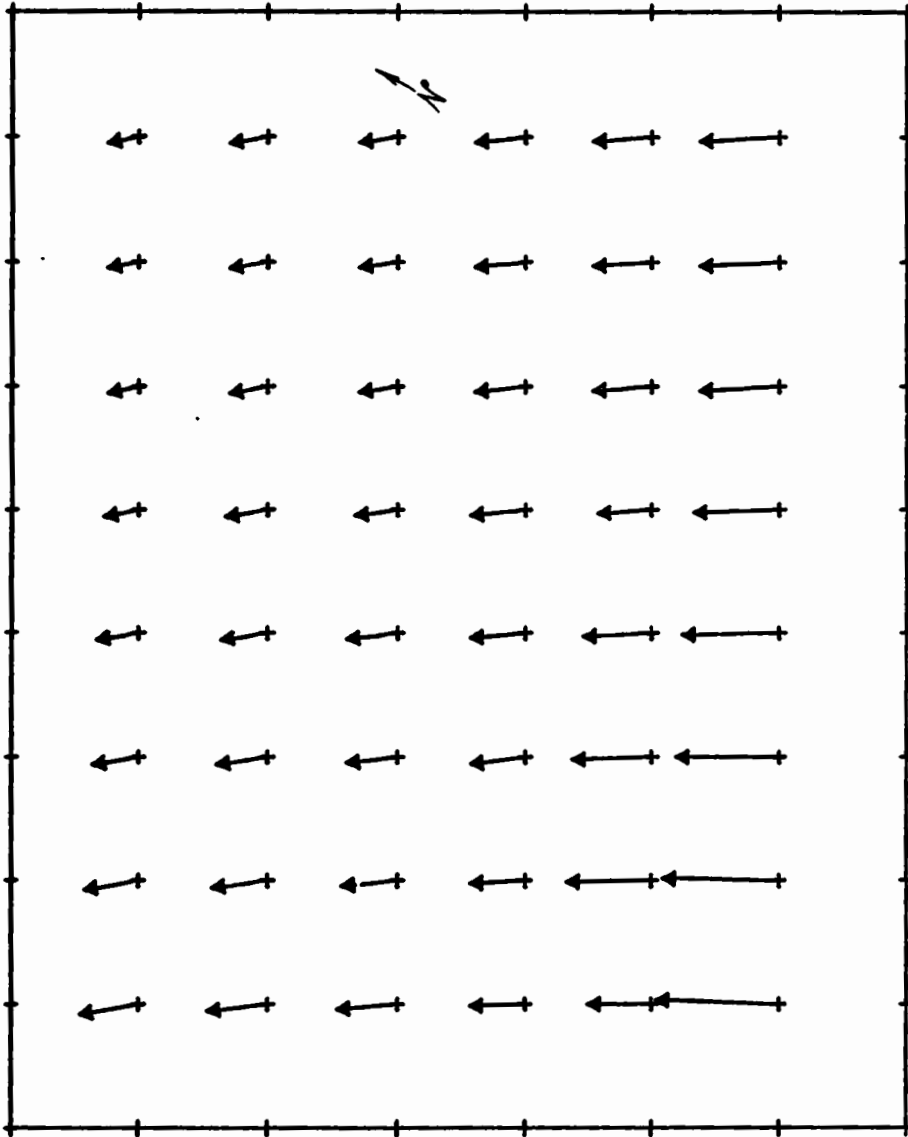


Figure 31. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 54 hours.

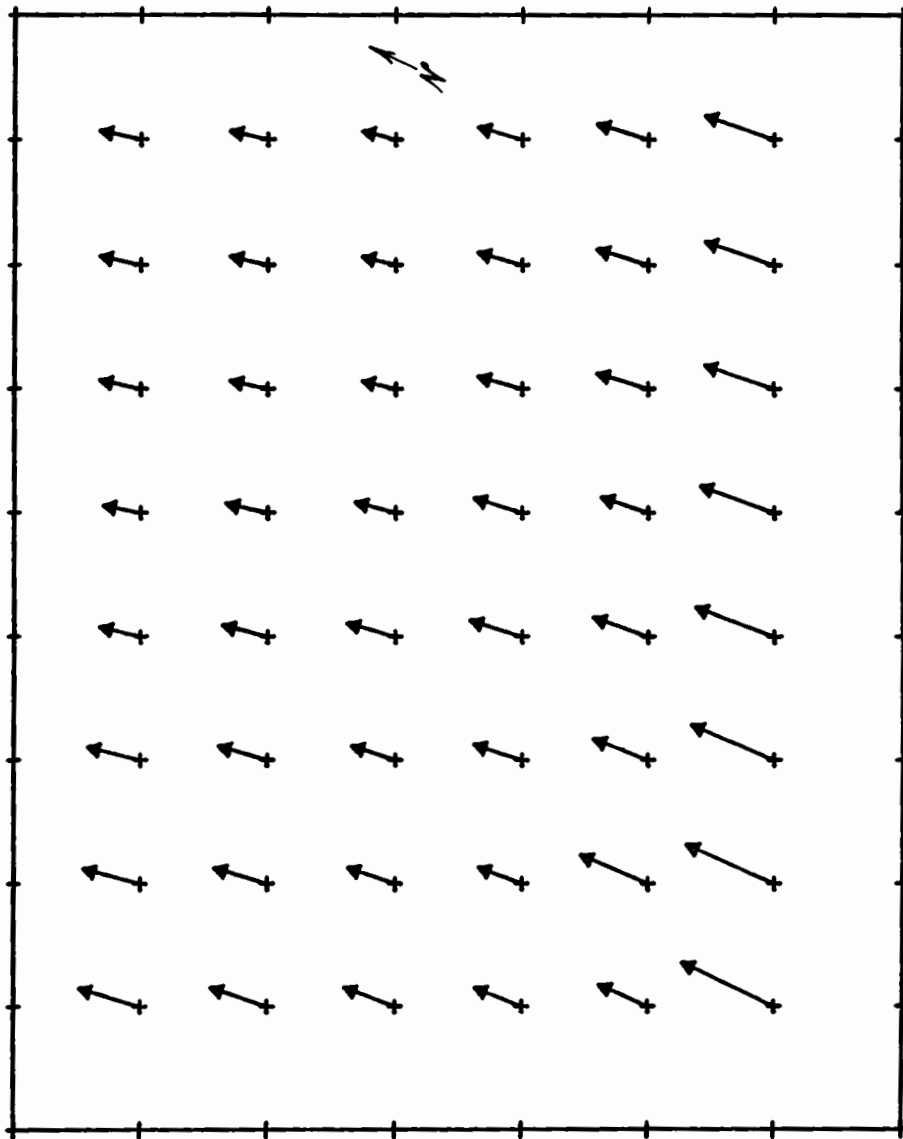


Figure 32. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 60 hours.

b. Results. The results of the Hurricane Carla simulation are given in Figures 33 to 41, and Appendix G. The input (observed) hydrograph for Sabine Pass is shown in Figure 33 for a 72-hour period starting at 0000 hours c.s.t., 10 September 1961. These results are based on a block friction factor of 0.0010.

The computed and observed values (where available) at gages 2 to 9 are shown in Figures 34 to 41. The principal discrepancy occurs at Beaumont where the computed peak surge exceeds the peak observed value by about 0.8 foot. It was found later that by increasing the block friction to 0.0025, this difference was reduced to 0.4 foot without materially changing the results at other key locations in the system.

The auxiliary sample output for the simulated Hurricane Carla run (App. G) gives, in addition to the serial listings of the above hydrographs and flow at the two main passes, sample listings of channel output at elapsed times of 30 and 60 hours.

## VII. STANDARD PROJECT HURRICANE (SPH)

### 1. LR-ST Storm Data.

The large radius, slow translation (LR-ST) storm was utilized as an atmospheric forcing function for the verified model of the Sabine-Calcasieu system. The storm parameters were extracted from the pertinent gulf coast section of the National Hurricane Research Project Report No. 33 (Graham and Nunn, 1959). Table 4 lists these values which were also used in conjunction with the analytic storm representation given by Jelesnianski (1965).

Table 4. Atmospheric parameters for the large radius, slow translation (ST) and medium translation (MT) storms.

Parameters	ST storm	MT storm
Radius to maximum winds	27 nmi	27 nmi
Maximum windspeed	100 mi/h	100 mi/h
Central pressure	27.55 in	27.55 in
Translation speed	4 kn	11 kn

Wind-stress vector plots have been prepared beginning at  $t = 30$  hours and at 10-hour increments to  $t = 80$  hours (Figs. 42 to 47). The storm track, which is taken normal to the general shoreline, has the Sabine-Calcasieu system on the right-hand side of the storm approaching the coastline. Landfall of the storm center is close to grid block 1,1. The orientation of these plots relative to the topography is similar to the wind fields shown for the Hurricane Carla verification. The gulf hydrographic input, provided by the Galveston District, was developed by an application of a one-dimensional bathystrophic model (Marinos and Woodward, 1968; Bodine, 1971). A tidal component has been added to this



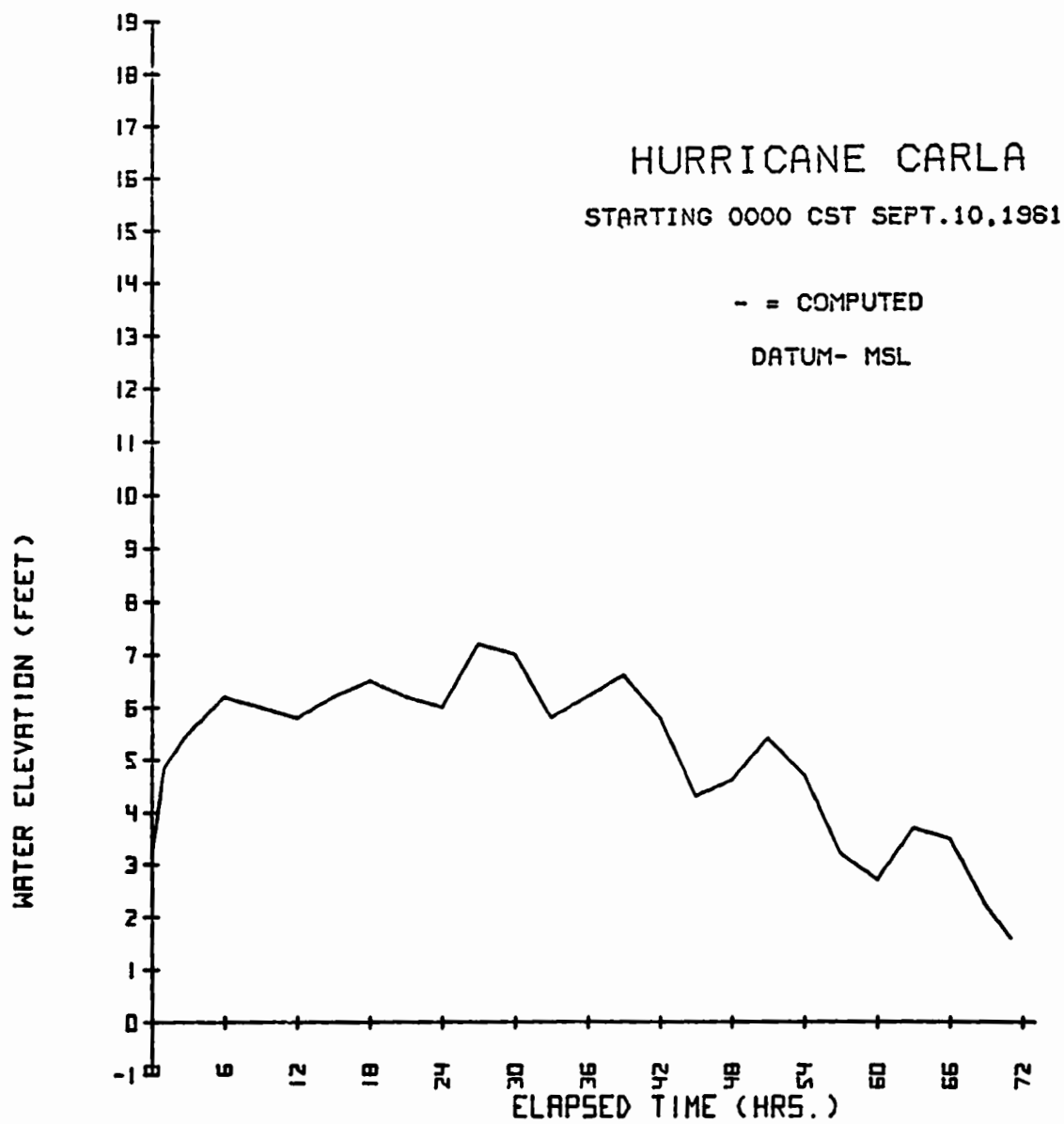


Figure 33. Hydrograph at Sabine Pass, southwest jetty for Hurricane Carla.

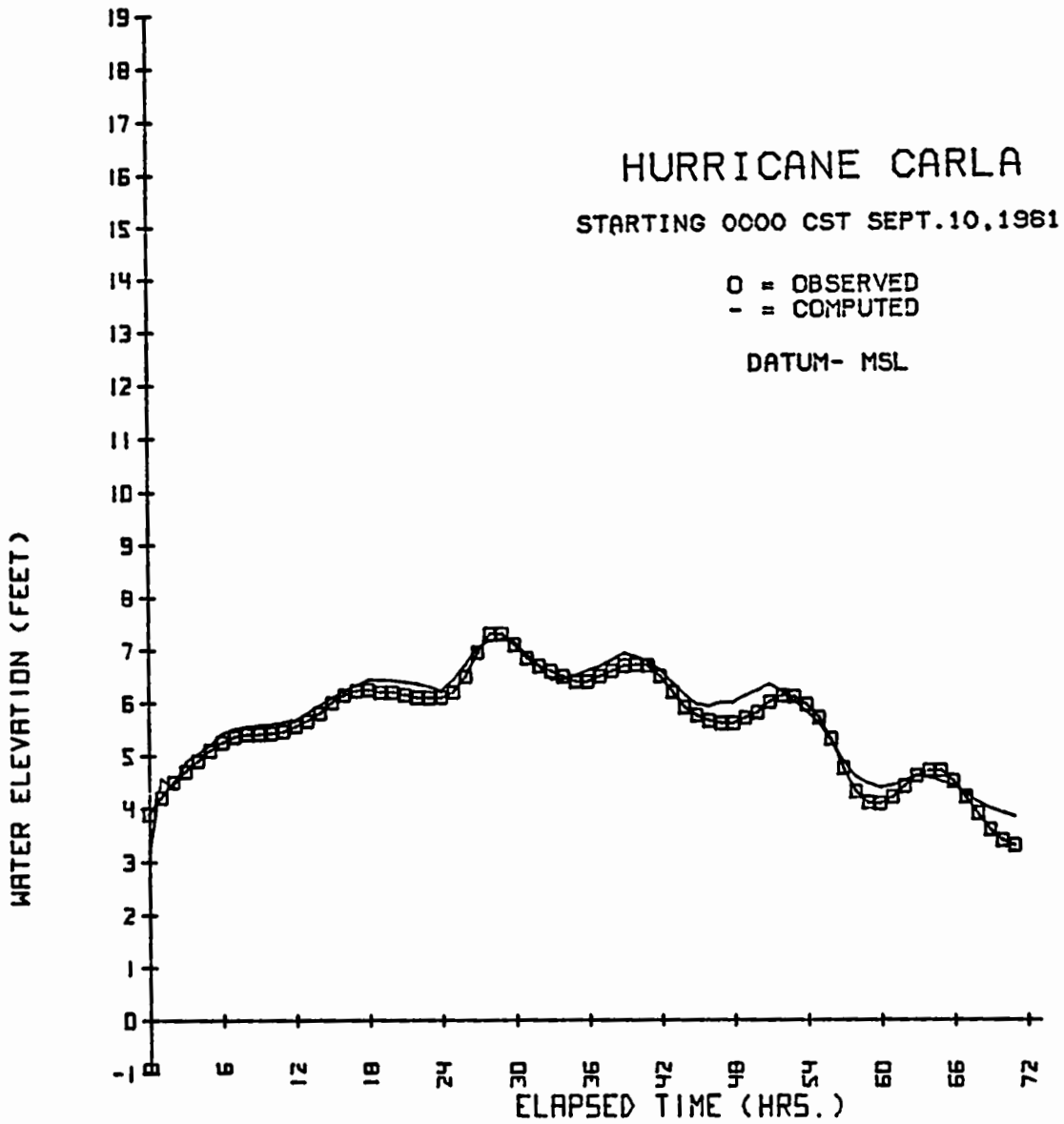


Figure 34. Hydrographs at Sabine Pass, U.S. Coast Guard Station for Hurricane Carla (FK = 0.0010).

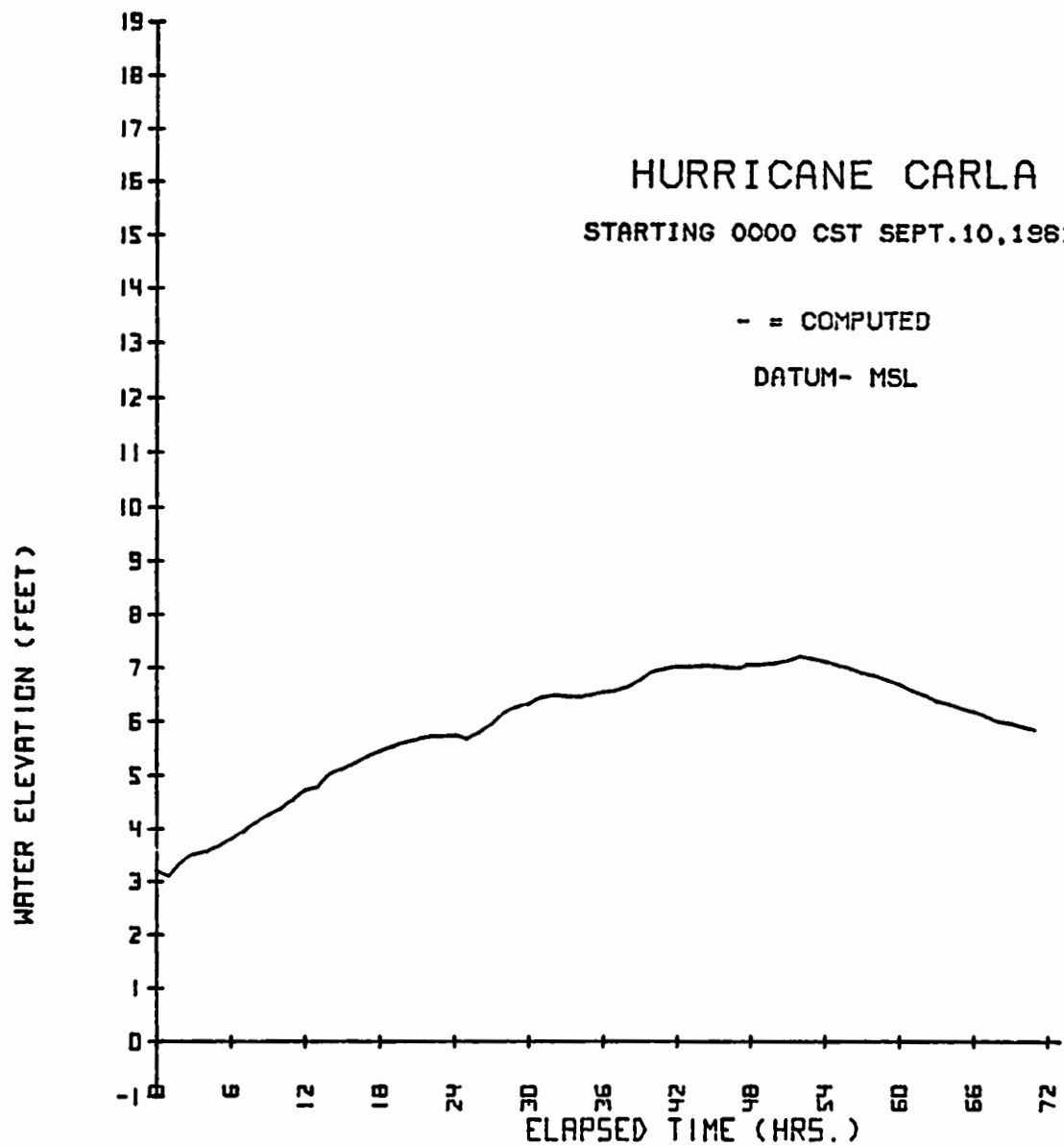


Figure 35. Hydrograph at Port Arthur for Hurricane Carla (FK = 0.0010).

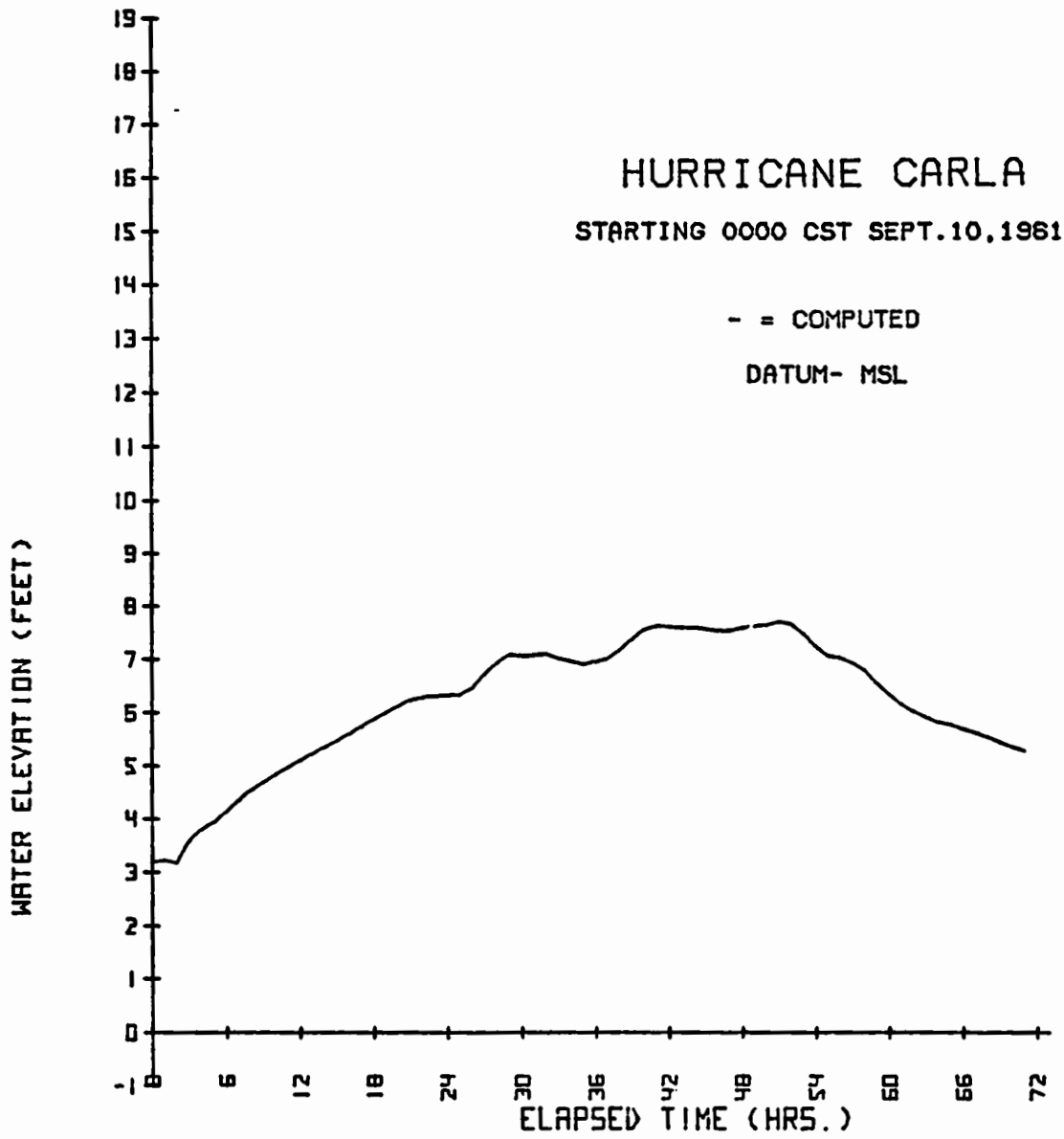


Figure 36. Hydrograph at north Sabine Lake for Hurricane Carla (FK = 0.0010).

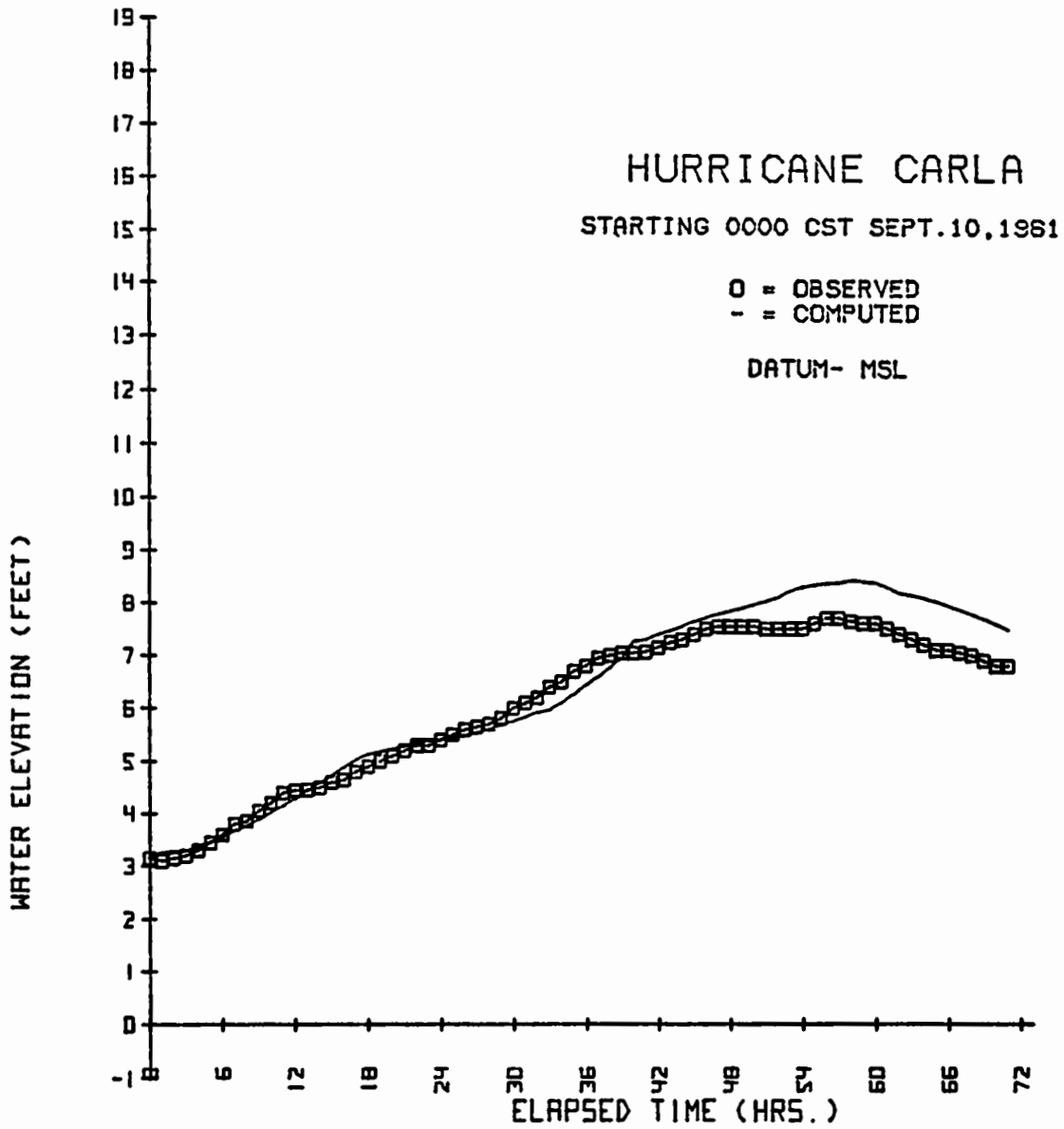


Figure 37. Hydrographs at Beaumont, Neches River, and Brakes Bayou for Hurricane Carla (FK = 0.0010).

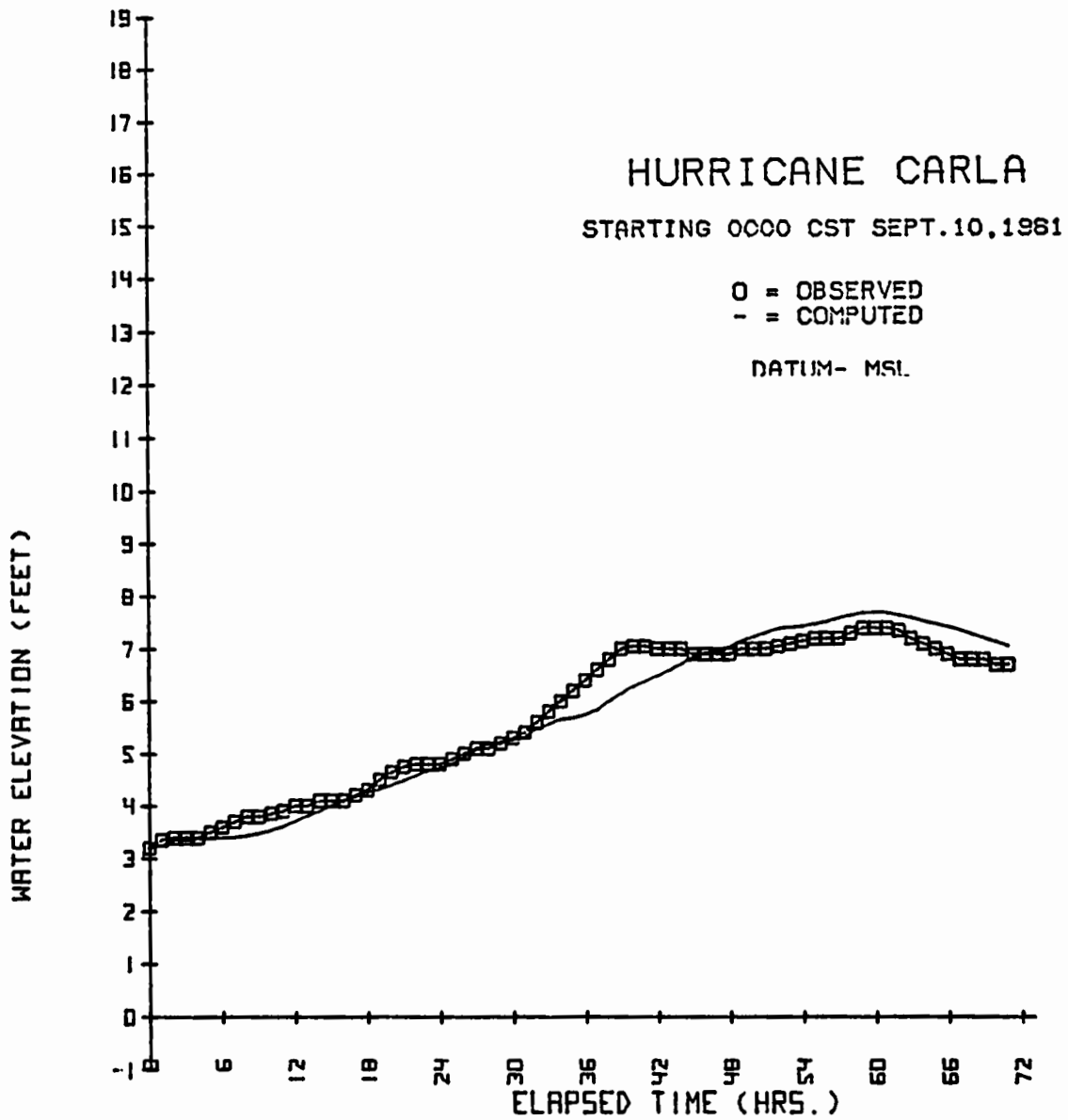


Figure 38. Hydrographs at Orange Naval Station, Sabine River for Hurricane Carla (FK = 0.0010).

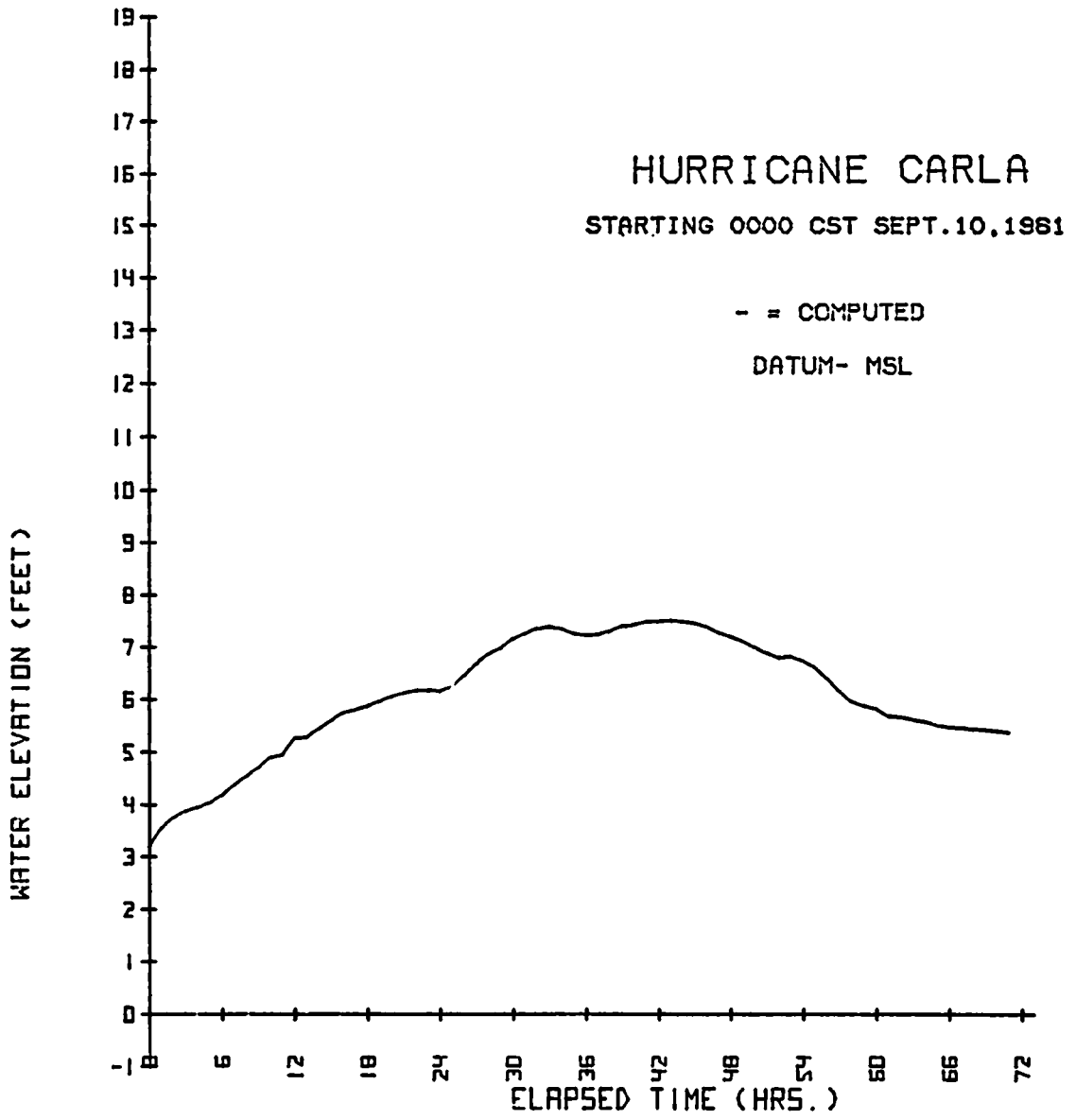


Figure 39. Hydrograph at west end of Intracoastal Waterway for Hurricane Carla (FK = 0.0010).

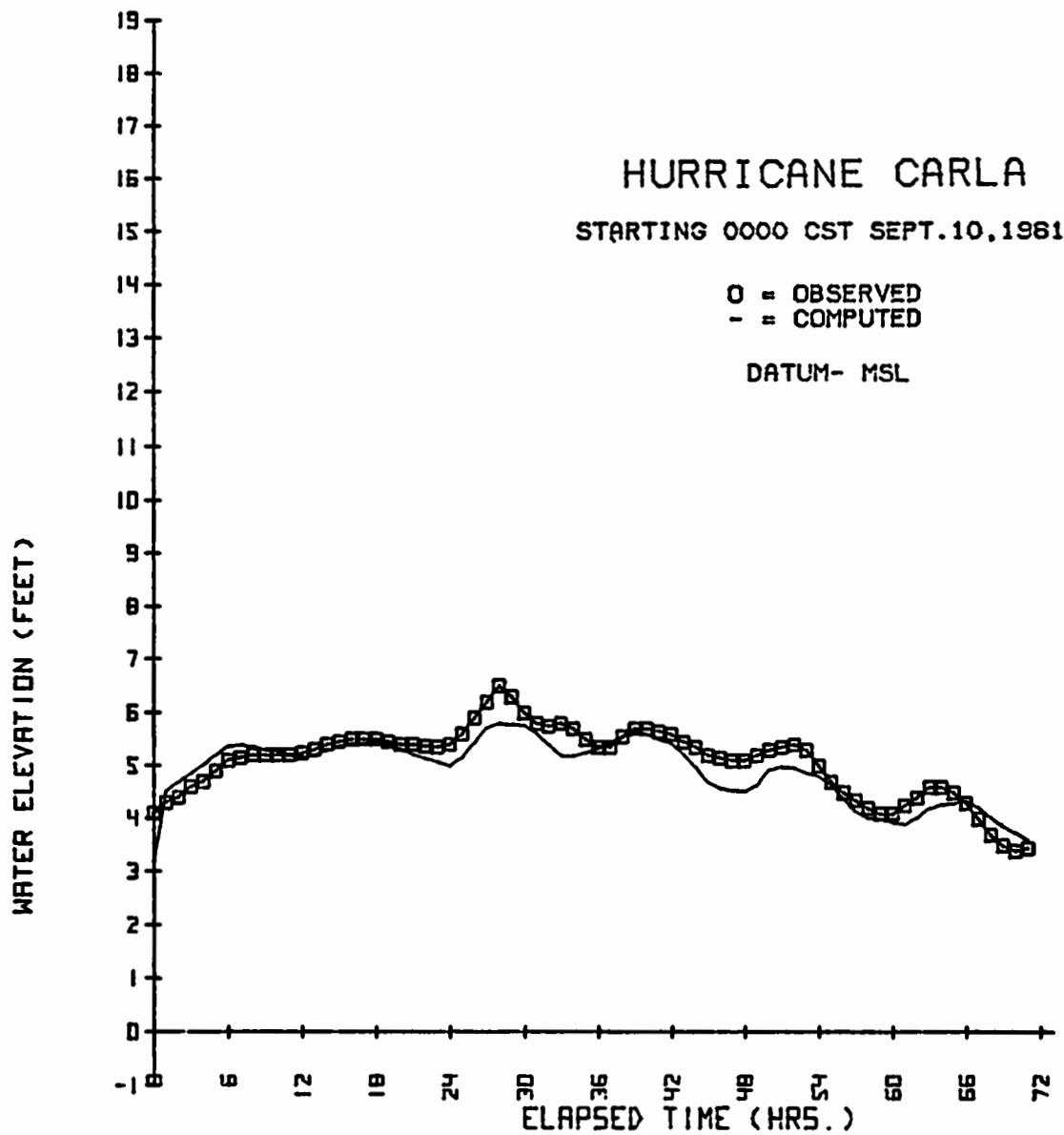


Figure 40. Hydrographs at Cameron, Calcasieu Pass for Hurricane Carla (FK = 0.0010).



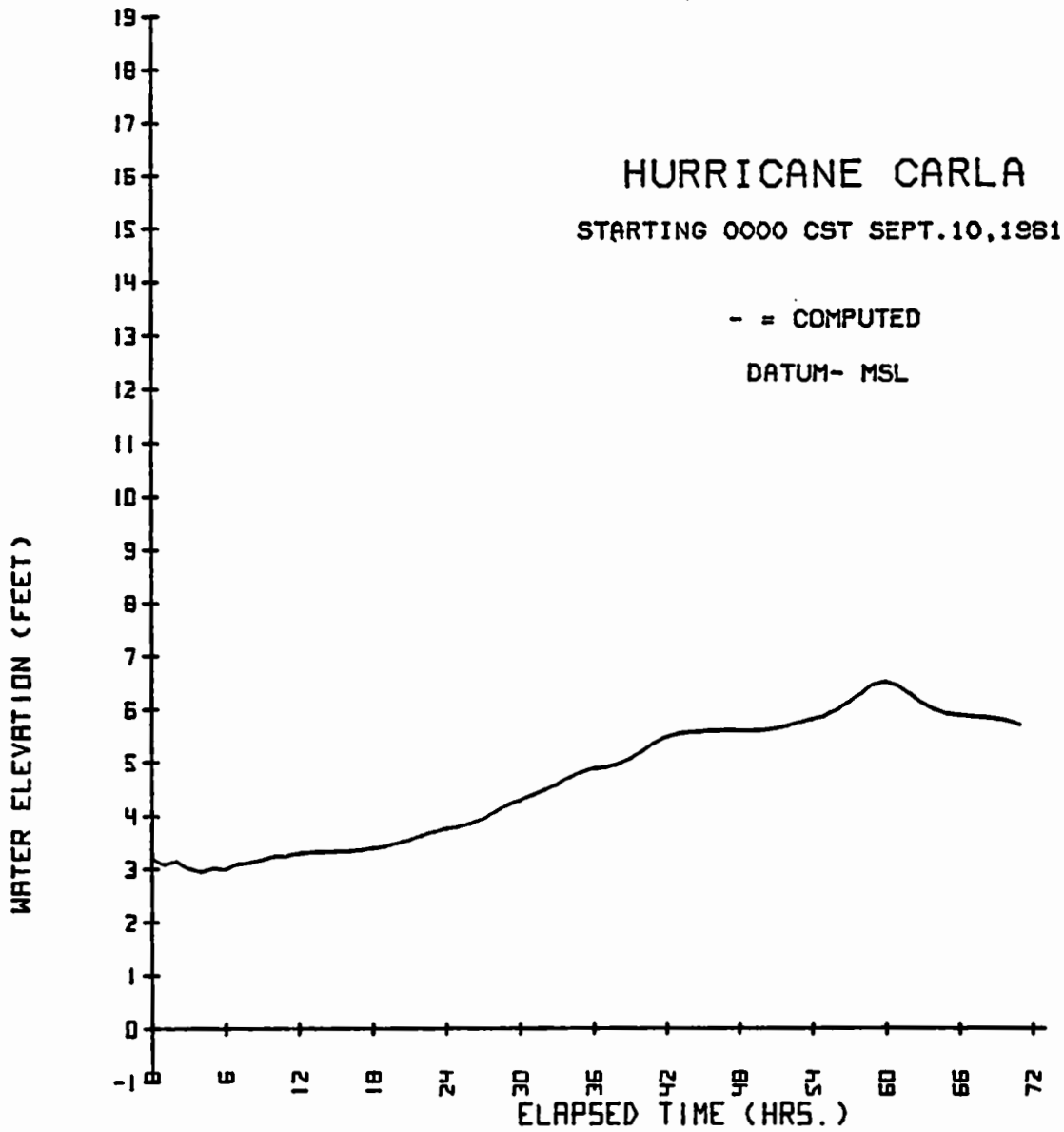


Figure 41. Hydrograph at Lake Charles for Hurricane Carla (FK = 0.0010).

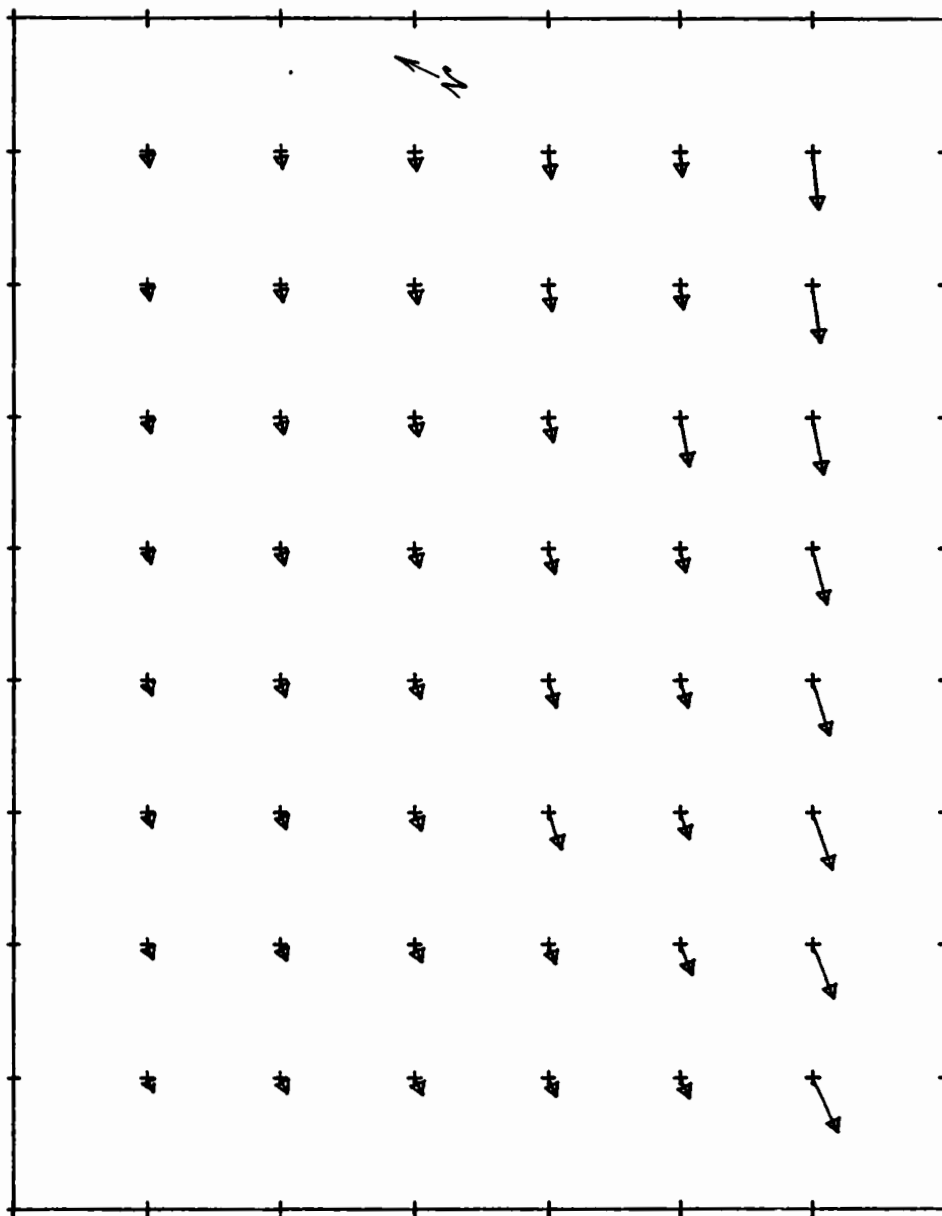


Figure 42. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 30 hours.

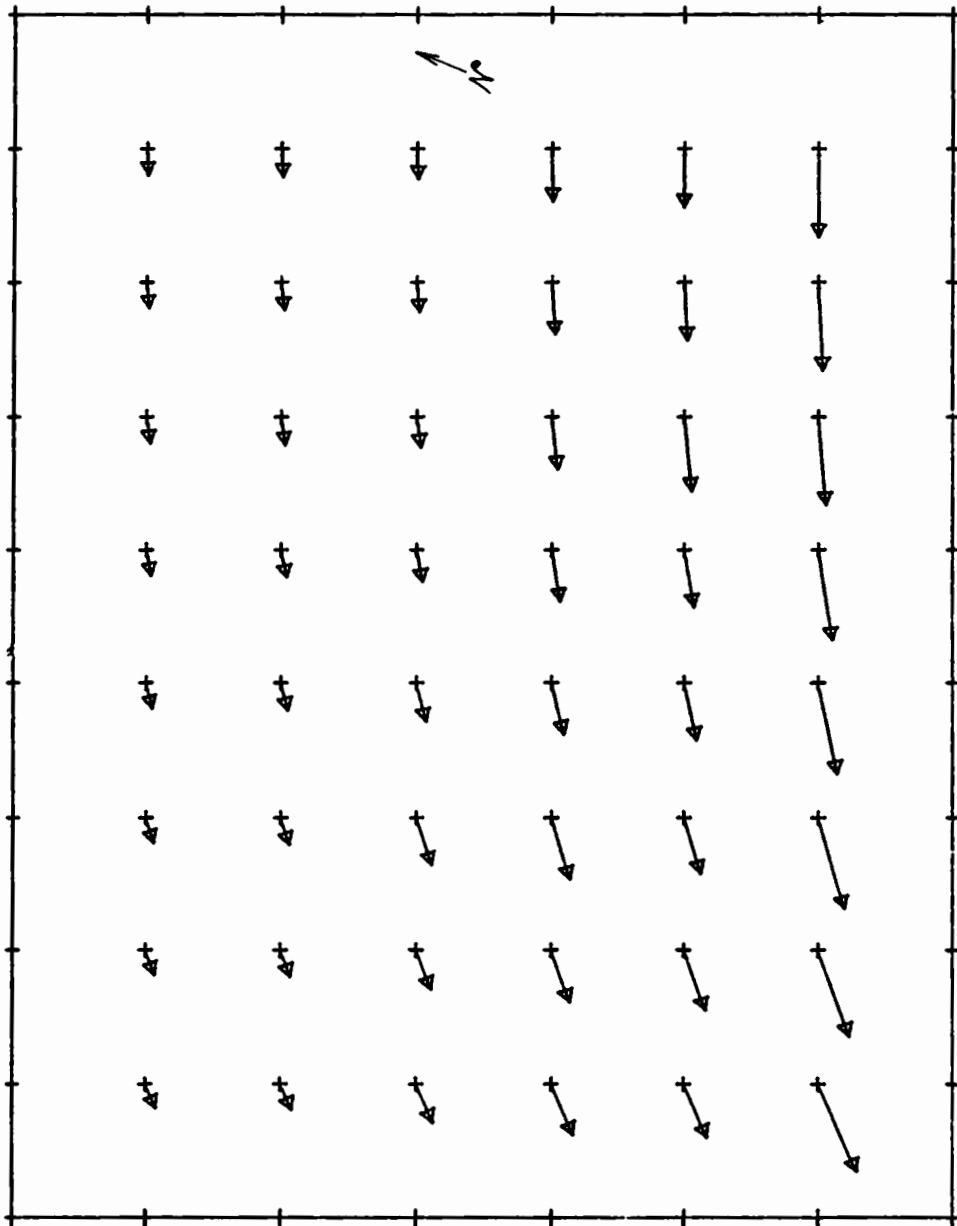


Figure 43. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 40 hours.

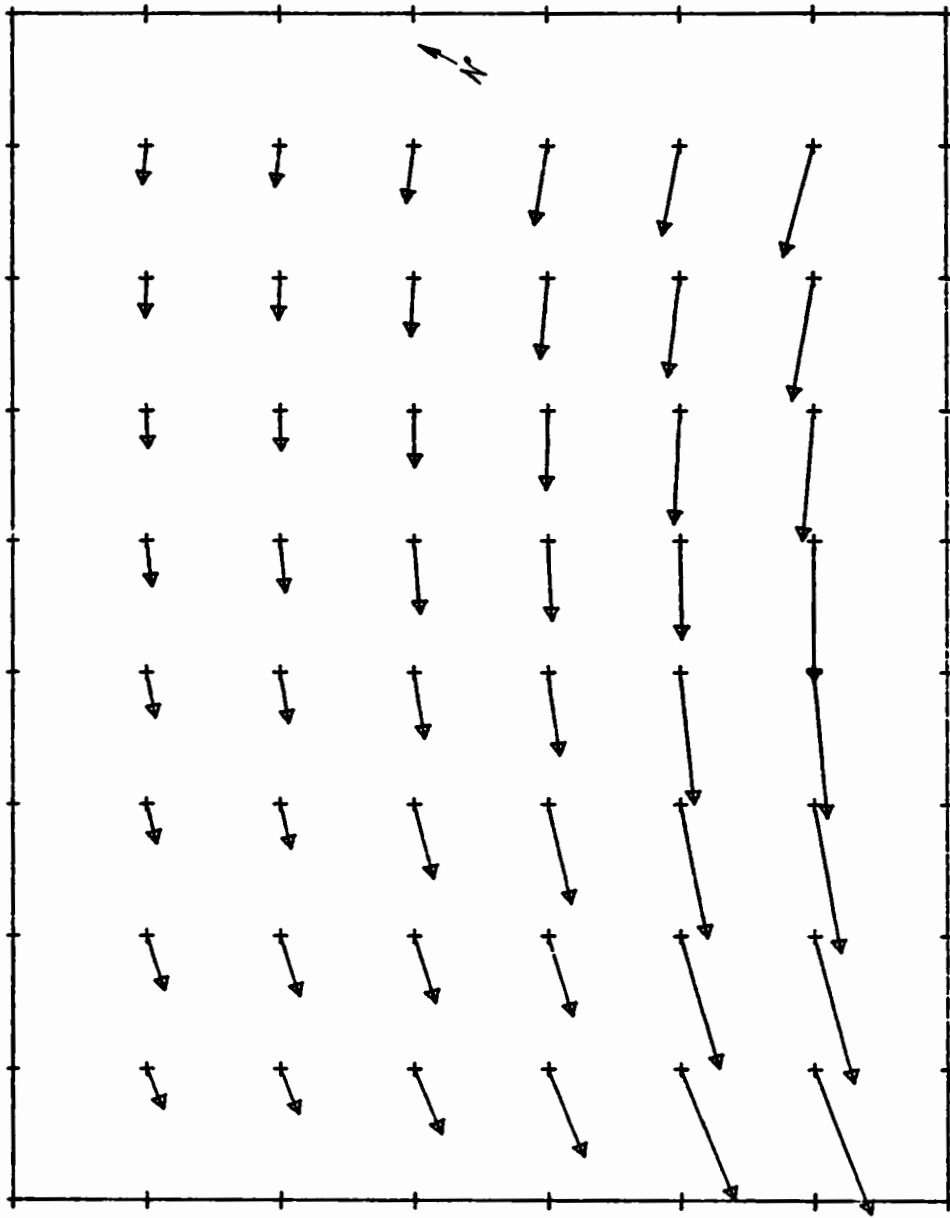


Figure 44. Wind-stress vectors for SPII large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 50 hours.

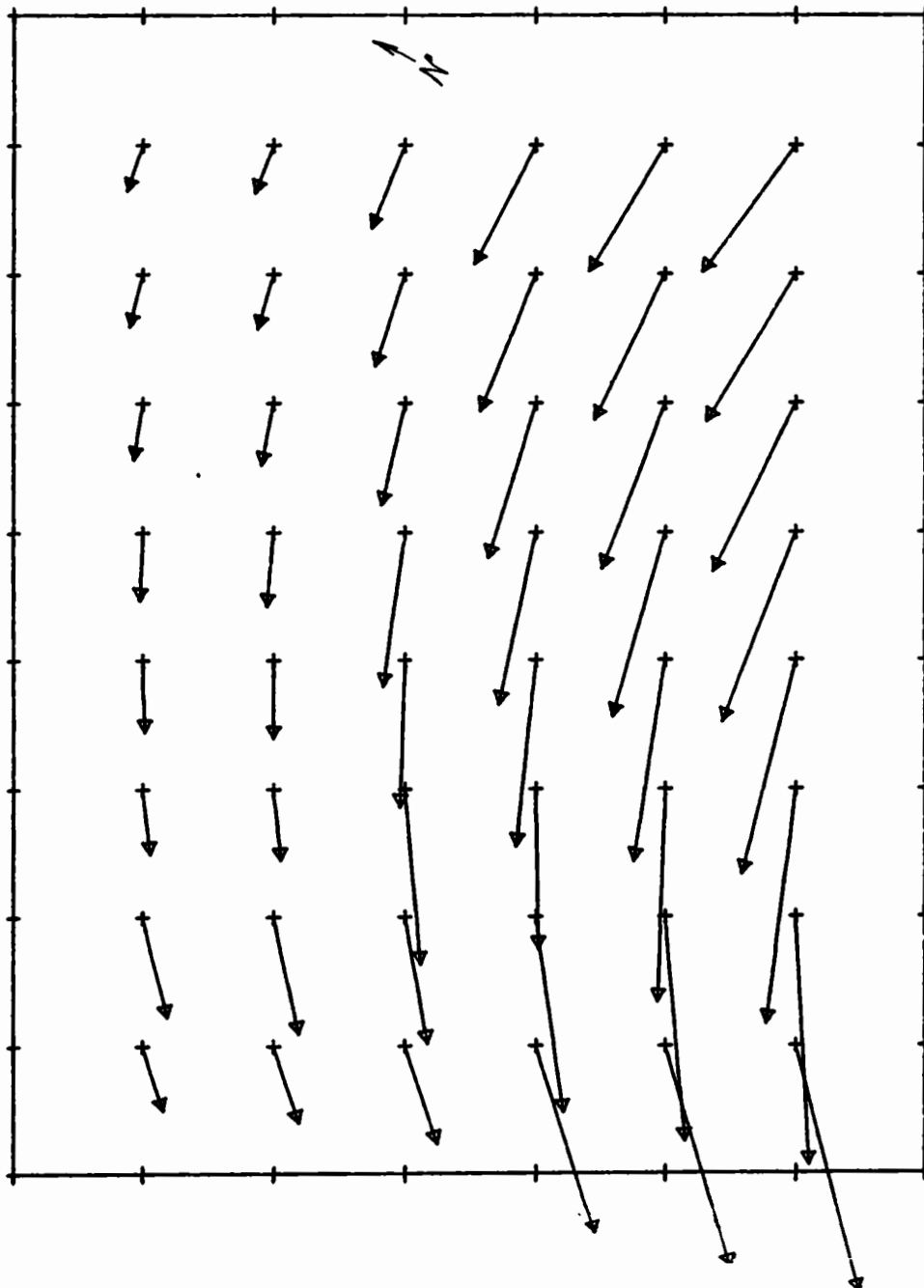


Figure 45. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 60 hours.

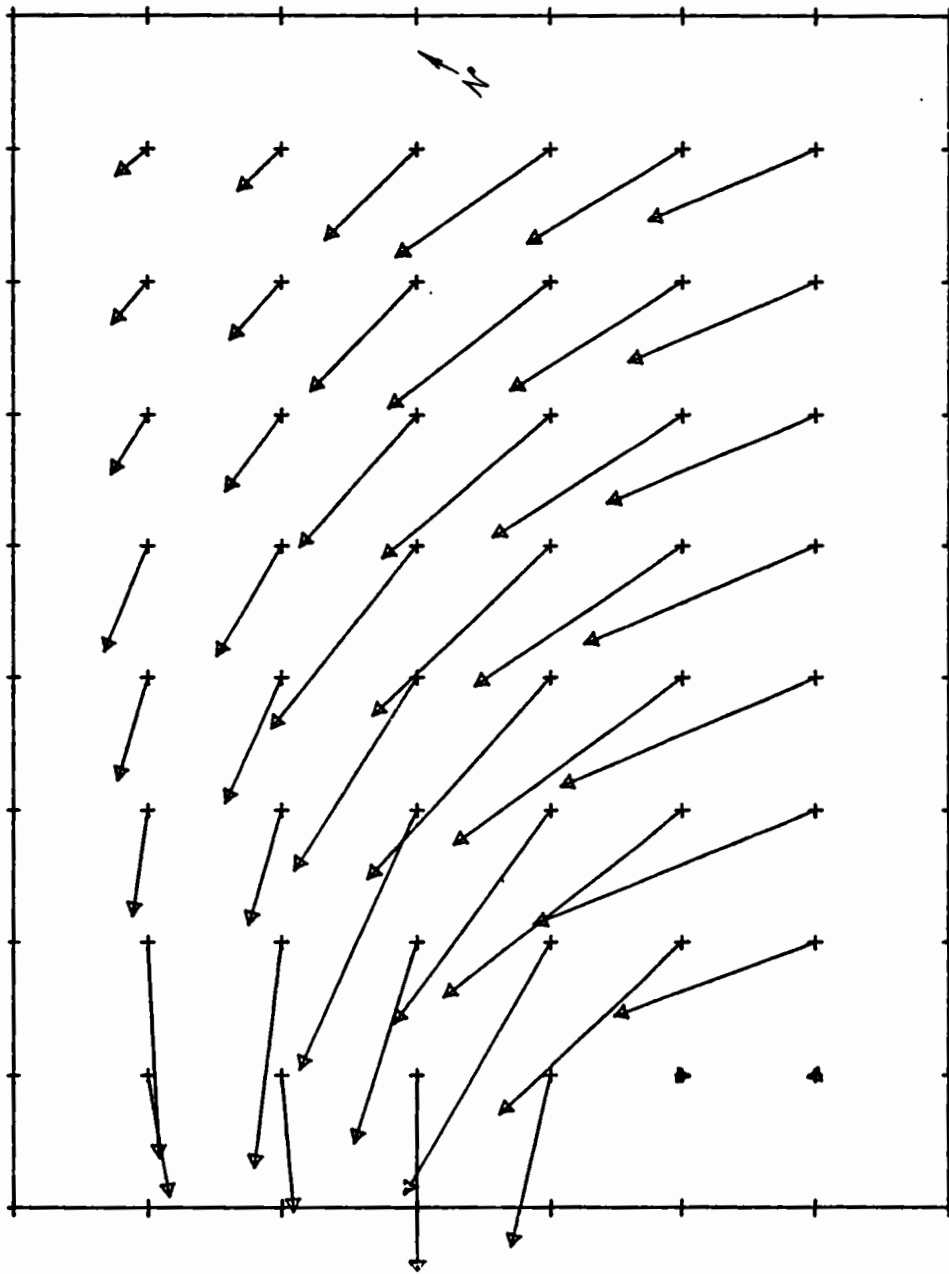


Figure 46. Wind-stress vectors for SPII large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 70 hours.

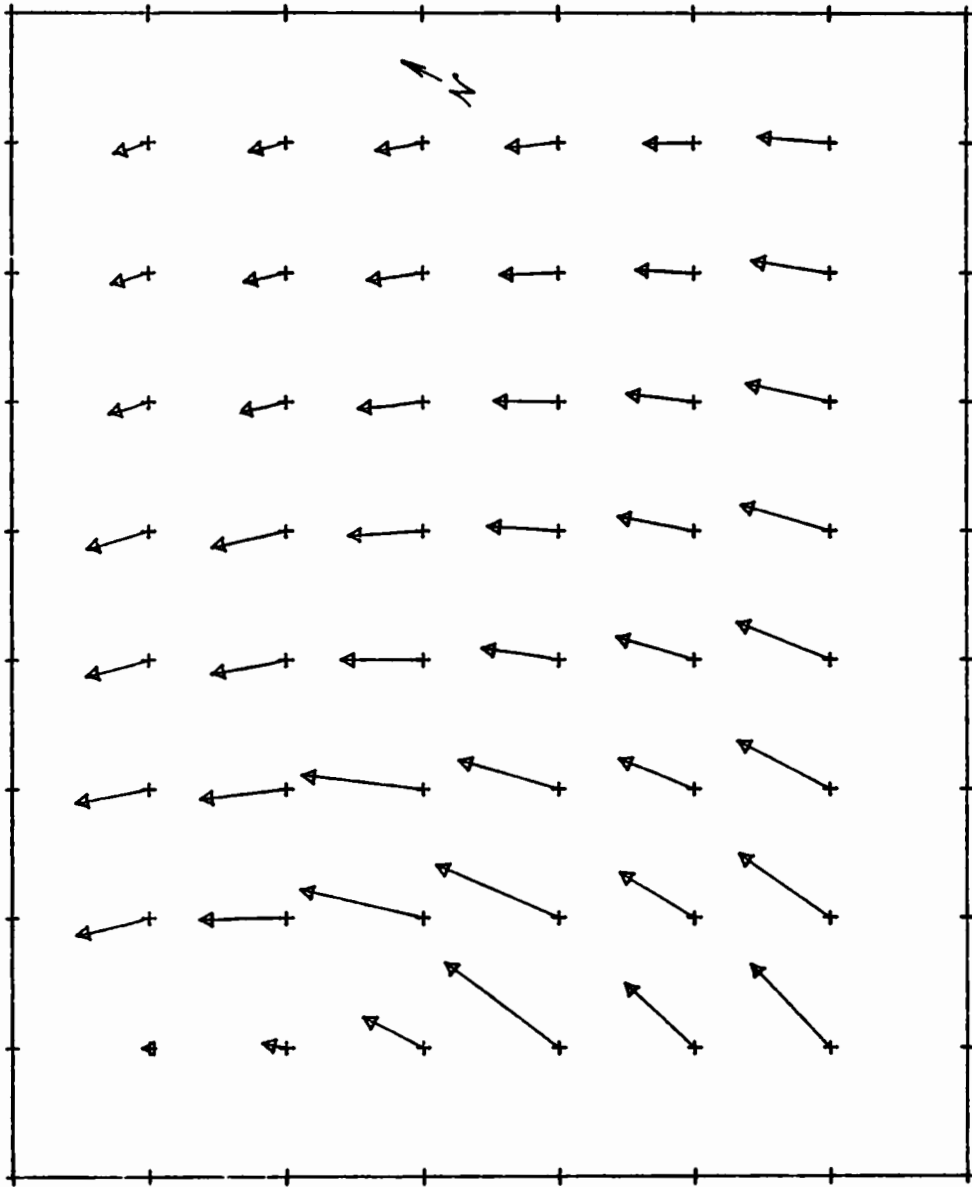


Figure 47. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 80 hours.

open-coast surge. A total rainfall of 16 inches in 24 hours is included as input. The results are based on a block friction factor of 0.0010 and are therefore tentative.

## 2. LR-ST Storm Results.

Nine simulated hydrographs are presented in Figures 48 to 56. The hydrograph locations are selected to coincide with the locations of the Hurricane Carla graphs and are shown on the base map in Figure 16. The maximum water level excursion in these graphs is nearly 19 feet and occurs at the Beaumont gage (Fig. 52) at approximately  $t = 80$  hours. The highest elevation at Sabine Pass, southwest jetty (Fig. 48) is 13 feet and occurs at  $t = 77$  hours. The Port Arthur surge crests (Fig. 52) at slightly greater than 14 feet shortly after high water is reached on the open coast. The gage at north Sabine Lake (Fig. 51) reaches a maximum of 15.3 feet which coincides with Port Arthur. The surge at Beaumont develops continuously from the 7-foot level at  $t = 66$  hours to a maximum at 80 hours in direct correlation with the surge character at the coastline. However, the 7-foot level is reached at the coast approximately 6 hours before Beaumont. At the Orange Naval Station (Fig. 53) the surge development appears more monotonic with a steady climb from  $t = 54$  hours to the peak surge of nearly 16 feet shortly before the Beaumont peak. The recession stage at Sabine Pass occurs quickly with passage of the storm, and the open-coast water level has returned to normal level by  $t = 90$  hours.

Drainage inland slowly reduces water levels and, at  $t = 90$  hours, Port Arthur and north Sabine Lake have water elevations of approximately 10 feet. Farther inland, the water elevation at Beaumont and Orange stands at 13 feet. The system continues to drain over the next 10 hours of prototype computation but slows considerably since the runoff reaches peak rate at the end of the run.

Recalculations at the Galveston District, using a block friction of 0.0025 which improved the Hurricane Carla simulation, indicated significant reductions in the peak surges for the LR-ST storm (Table 5). The greatest reduction is for Beaumont, as in the case of Hurricane Carla; however, the amount of reduction is disproportionately greater than that seen for Hurricane Carla.

Table 5. Comparison of peak surges for the LR-ST storm, using two different block friction factors.

Location	Surges (ft above MSL)	
	FK = 0.0010	FK = 0.0025
Port Arthur	14.3	14.1
Beaumont	18.7	17.1
Orange Naval Station	15.9	15.2
Lake Charles	14.1	13.9



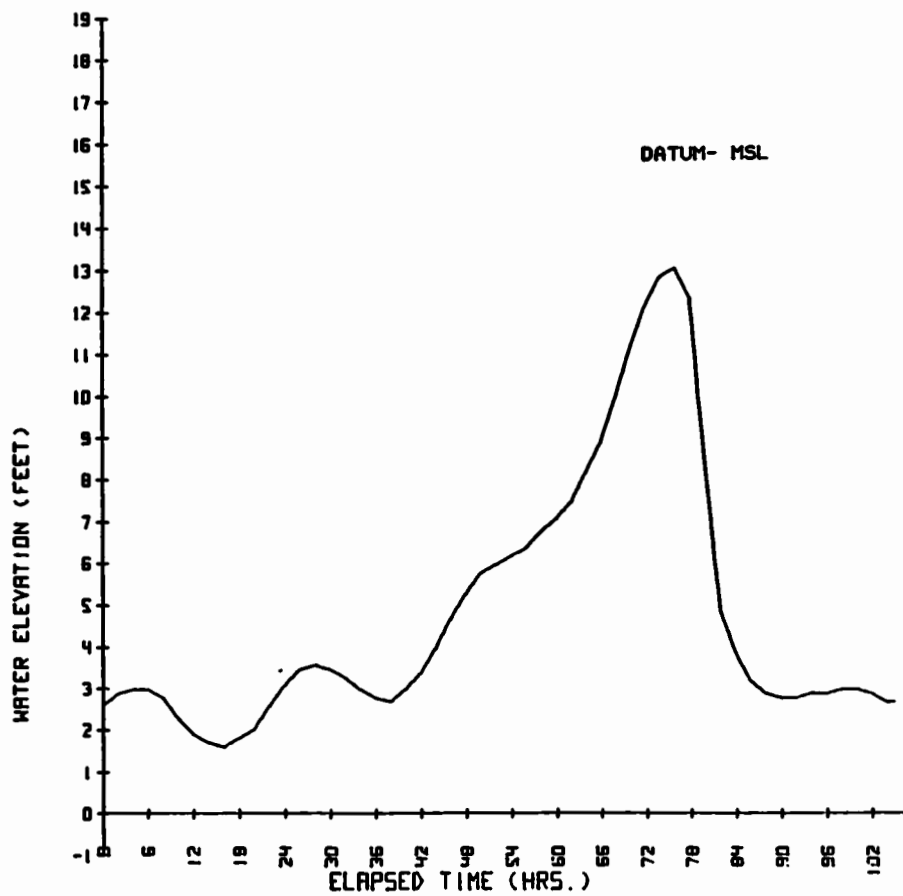


Figure 48. Hydrograph for SPH, LR-ST at Sabine Pass, southwest jetty (FK = 0.0010).

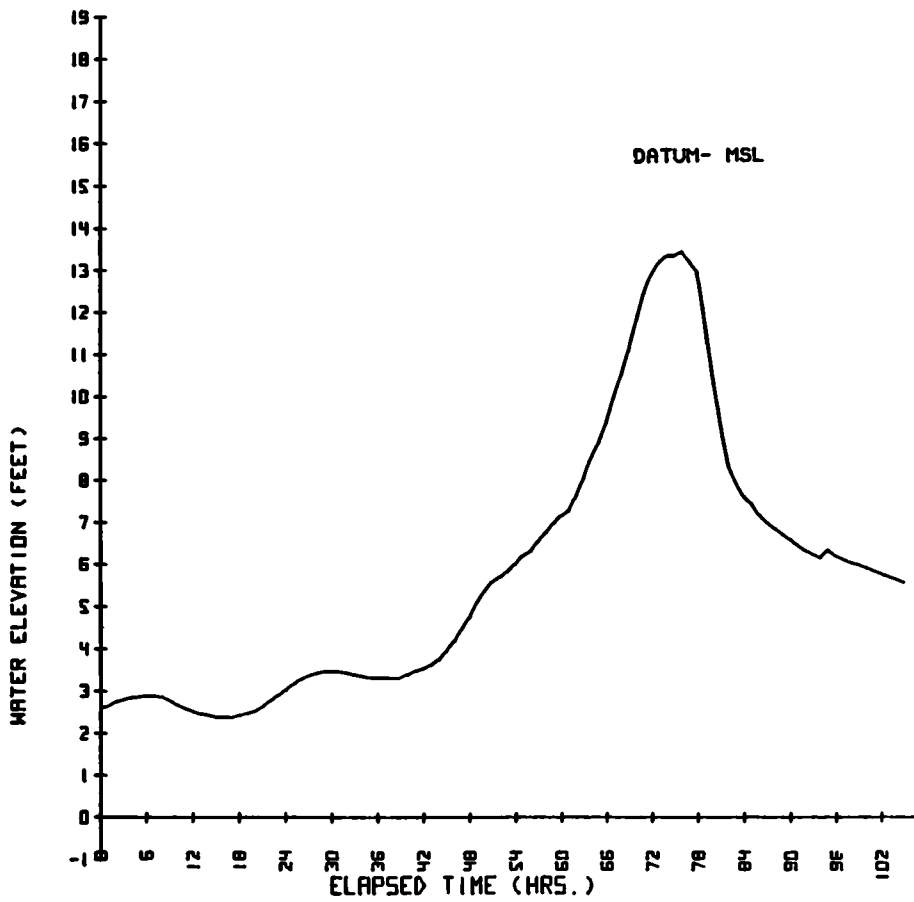


Figure 49. Hydrograph for SPH, LR-ST at Sabine Pass, U.S. Coast Guard Station (FK = 0.0010).

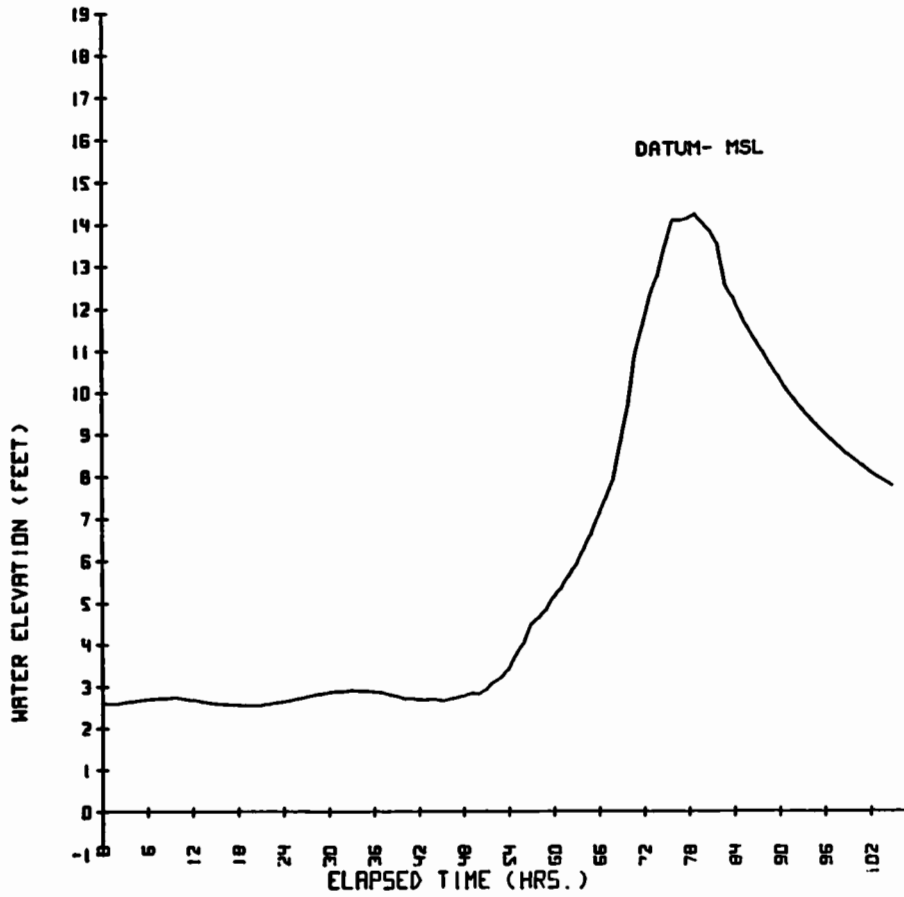


Figure 50. Hydrograph for SPH, LR-ST at Port Arthur (FK = 0.0010).

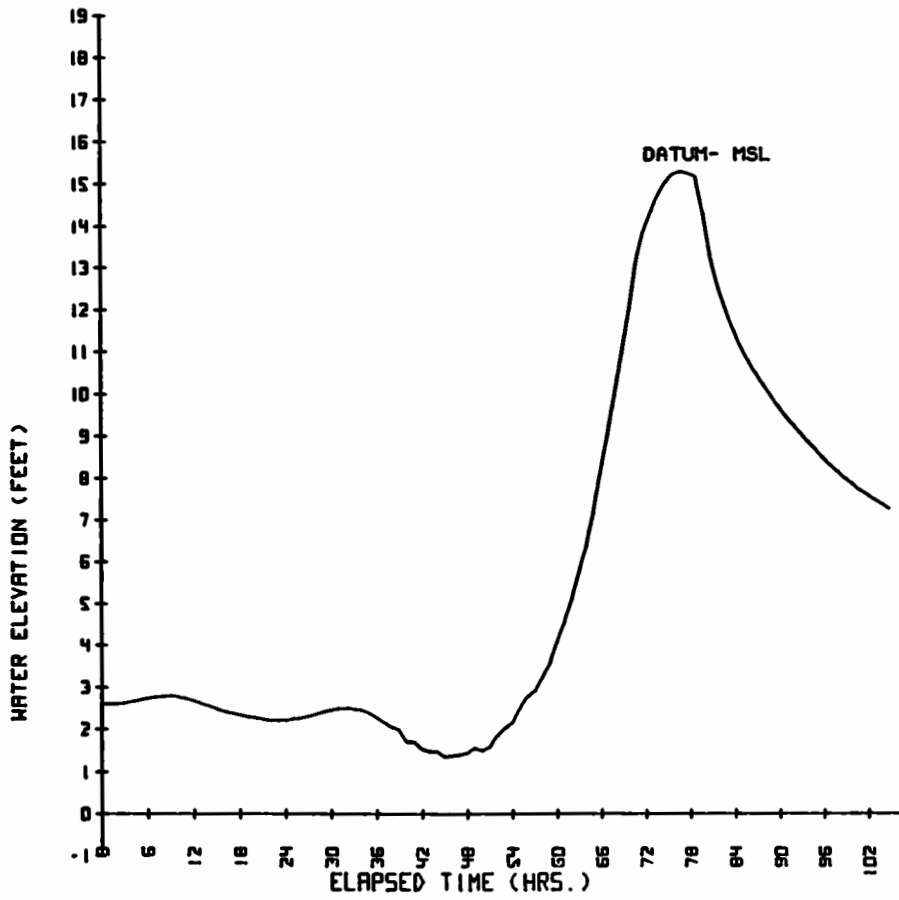


Figure 51. Hydrograph for SPH, LR-ST at north Sabine Lake (FK = 0.0010).

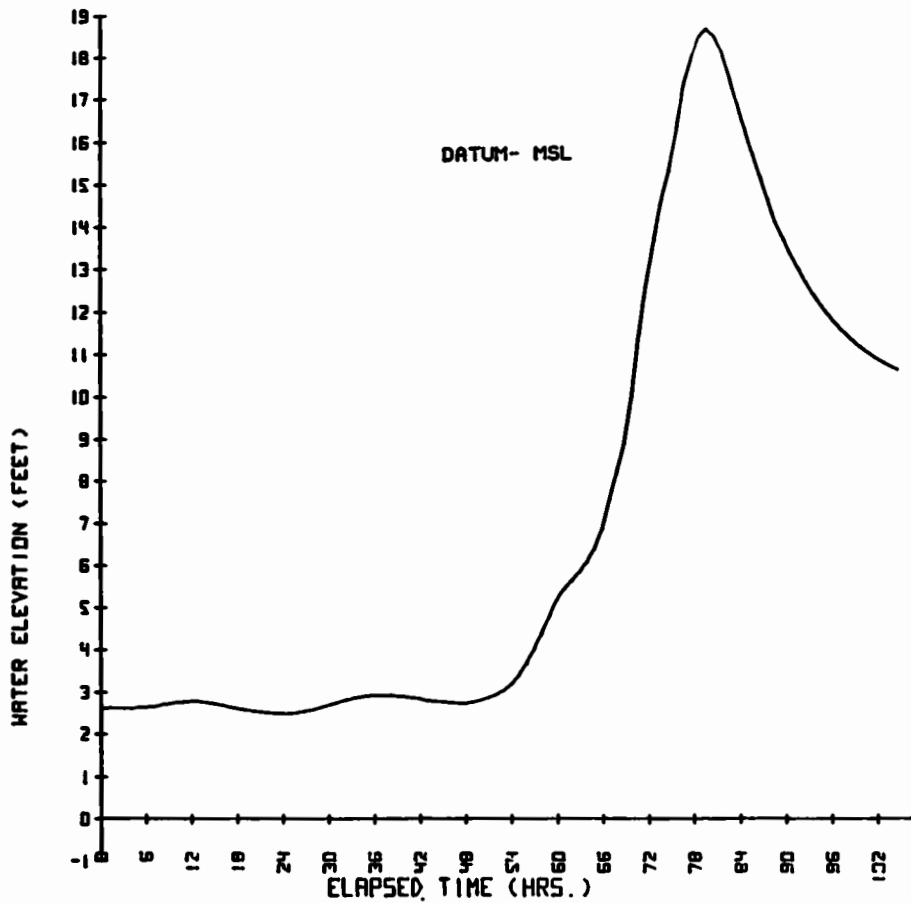


Figure 52. Hydrograph for SPH, LR-ST at Beaumont, Neches River, and Brakes Bayou.

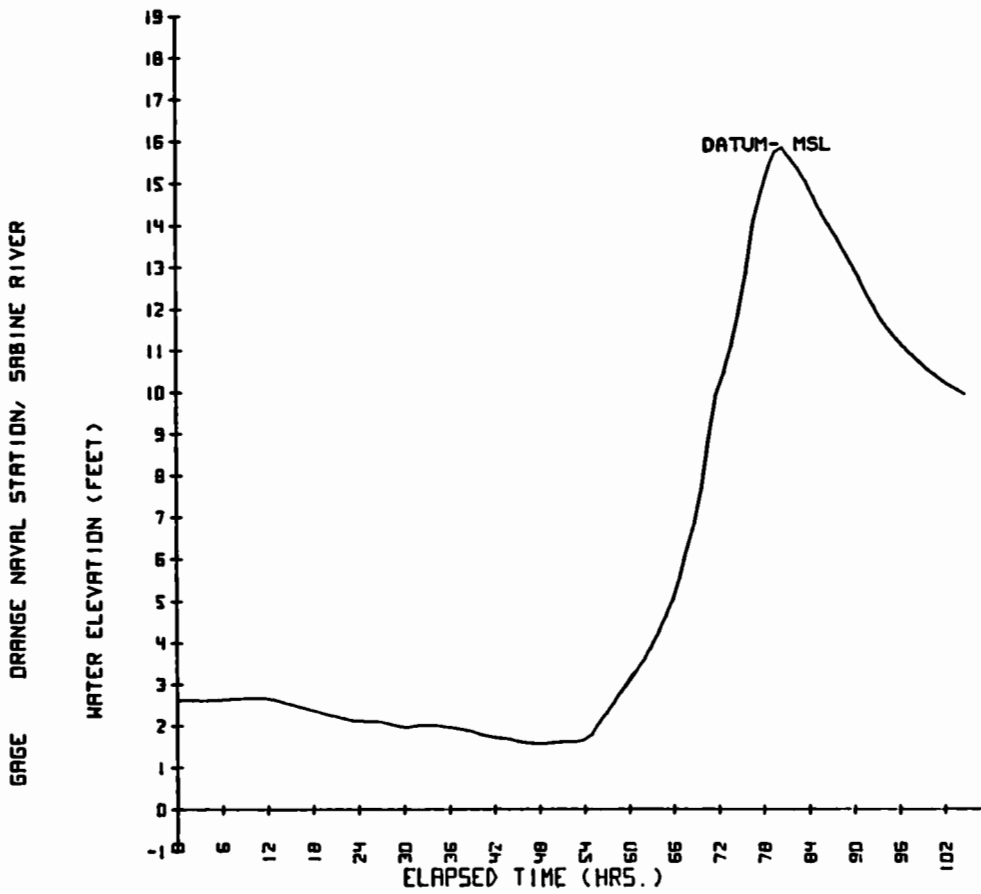


Figure 53. Hydrograph for SPH, LR-ST at Orange Naval Station, Sabine River (FK = 0.0010).

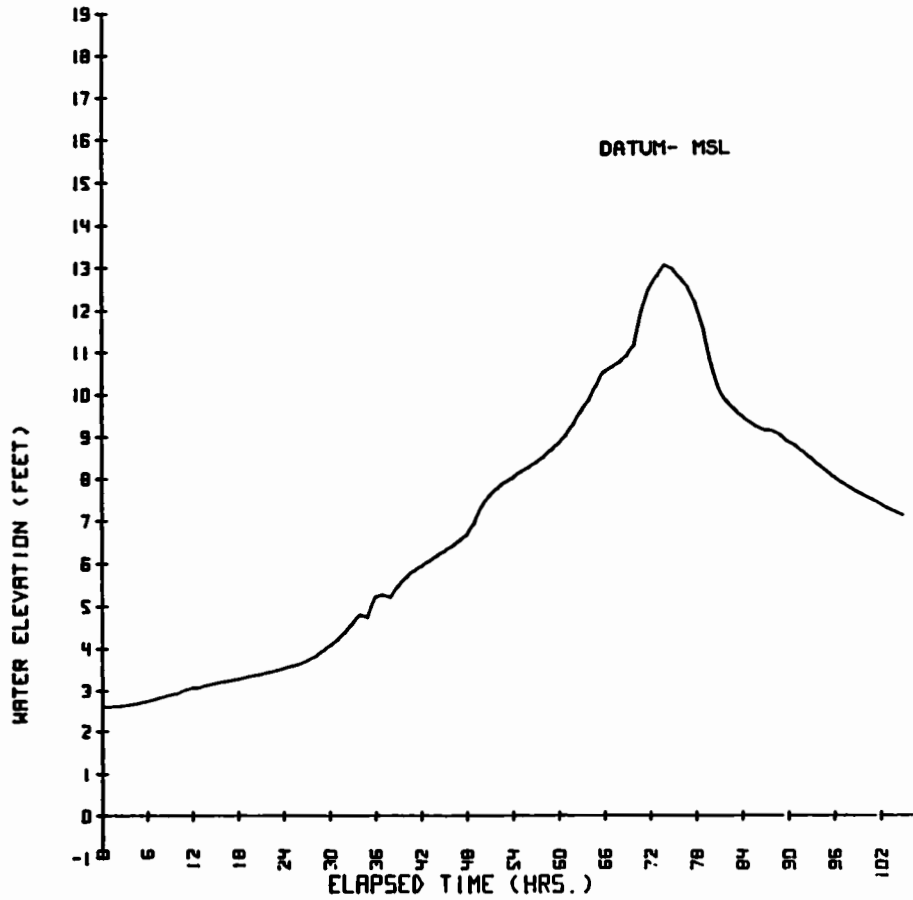


Figure 54. Hydrograph for SPH, LR-ST at west end of Intra-coastal Waterway (FK = 0.0010).

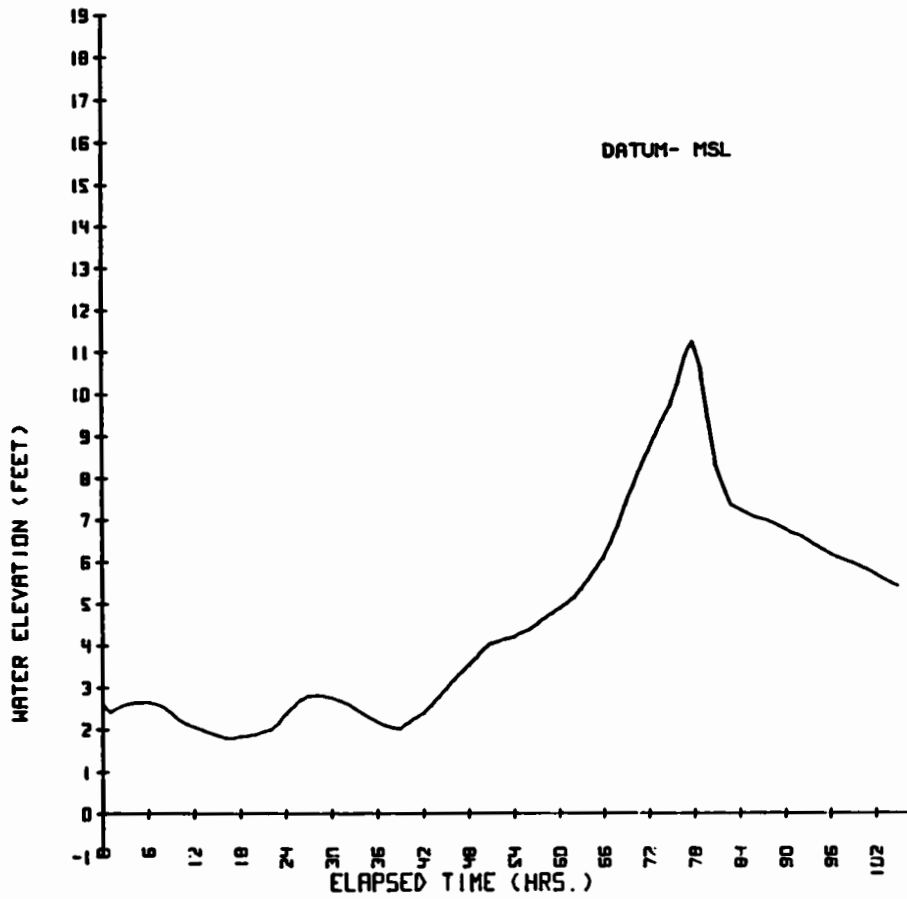


Figure 55. Hydrograph for SPH, LR-ST at Cameron, Calcasieu Pass (FK = 0.0010).



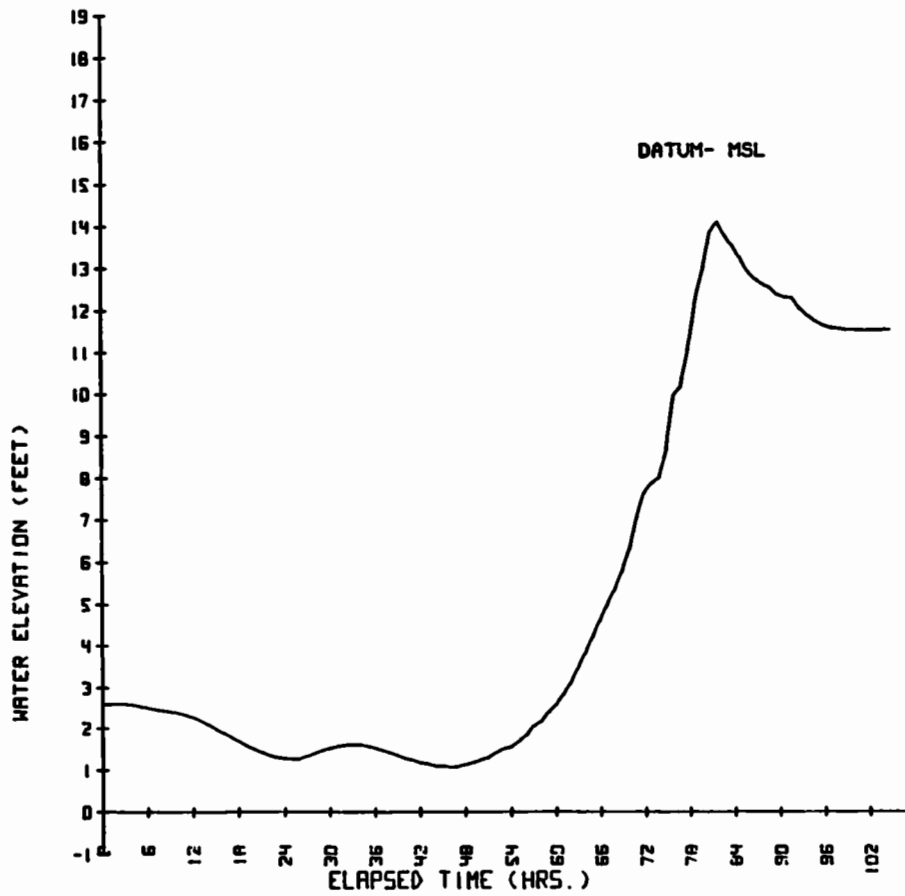


Figure 56. Hydrograph for SPH, LR-ST at Lake Charles, Calcasieu River (FK = 0.0010).

### 3. LR-MT Storm Data.

The large radius, medium translation (LR-MT) storm has identical characteristics to the LR-ST storm with the exception of a higher translation speed of 11 knots. Wind vector plots from  $t = 15$  hours to  $t = 40$  hours are shown at 5-hour increments in Figures 57 to 62. The storm track is identical to that of the LR-ST storm. The gulf hydrographic input was derived by one-dimensional, bathystrophic analysis and provided by the Galveston District. Runs were made both with and without rainfall. Again, the results given graphically below are the tentative results based on  $FK = 0.0010$ .

### 4. LR-MT Storm Results.

The more rapid movement of the storm center across the Sabine-Calcasieu system yielded generally smaller water level excursions inside the bay system in comparison with the LR-ST storm. Hydrographs at the established prototype locations are shown in Figures 63 to 71 for the computer run with rainfall (16 inches) and without rainfall. Note that direct comparison between the LR-ST results and LR-MT results should be made on the basis of Figures 48 to 56 and 63 to 71, respectively. All of the SPH runs use an initial water level of about 2.5 feet in the bay system.

A summary of the peak values and relative times of water level at seven locations for the three different SPH runs is given in Table 6. Although the absolute values of the water levels depend on the value of  $FK$  (as discussed in previous sections), all results in Table 6 are based on the same  $FK$  and hence the difference between values is not too sensitive to  $FK$ .

Table 6. Comparison of peak surge and time of peak surge, showing effects of translational speed of storm and rainfall ( $FK = 0.0010$  for all three cases).

Location	Slow speed		Medium speed			
	With rainfall		With rainfall		Without rainfall	
	(ft above MSL)	(time) <sup>1</sup>	(ft above MSL)	(time)	(ft above MSL)	(time)
Sabine Pass entrance	13.0	0	14.9	0	14.9	0
Port Arthur	14.3	2	13.2	2	12.5	2
North Sabine Lake	15.3	1	15.3	1	14.7	1
Beaumont	18.7	4	15.1	5	11.5	6
Orange Naval Station	15.9	4	14.5	5	11.7	6
Cameron	11.3	1	11.0	1	10.8	1
Lake Charles	14.1	6	14.2	6	13.2	6

<sup>1</sup>Nearest hour after that of Sabine Pass entrance.

Comparison of the first and second sets of peak levels in Table 6 indicates a reduced response at nearly all stations within the Sabine-Calcasieu system with an increase in the translational speed of the storm, in spite of the increased surge at the shoreline (Sabine Pass entrance). A reduction in volume response within the system is expected

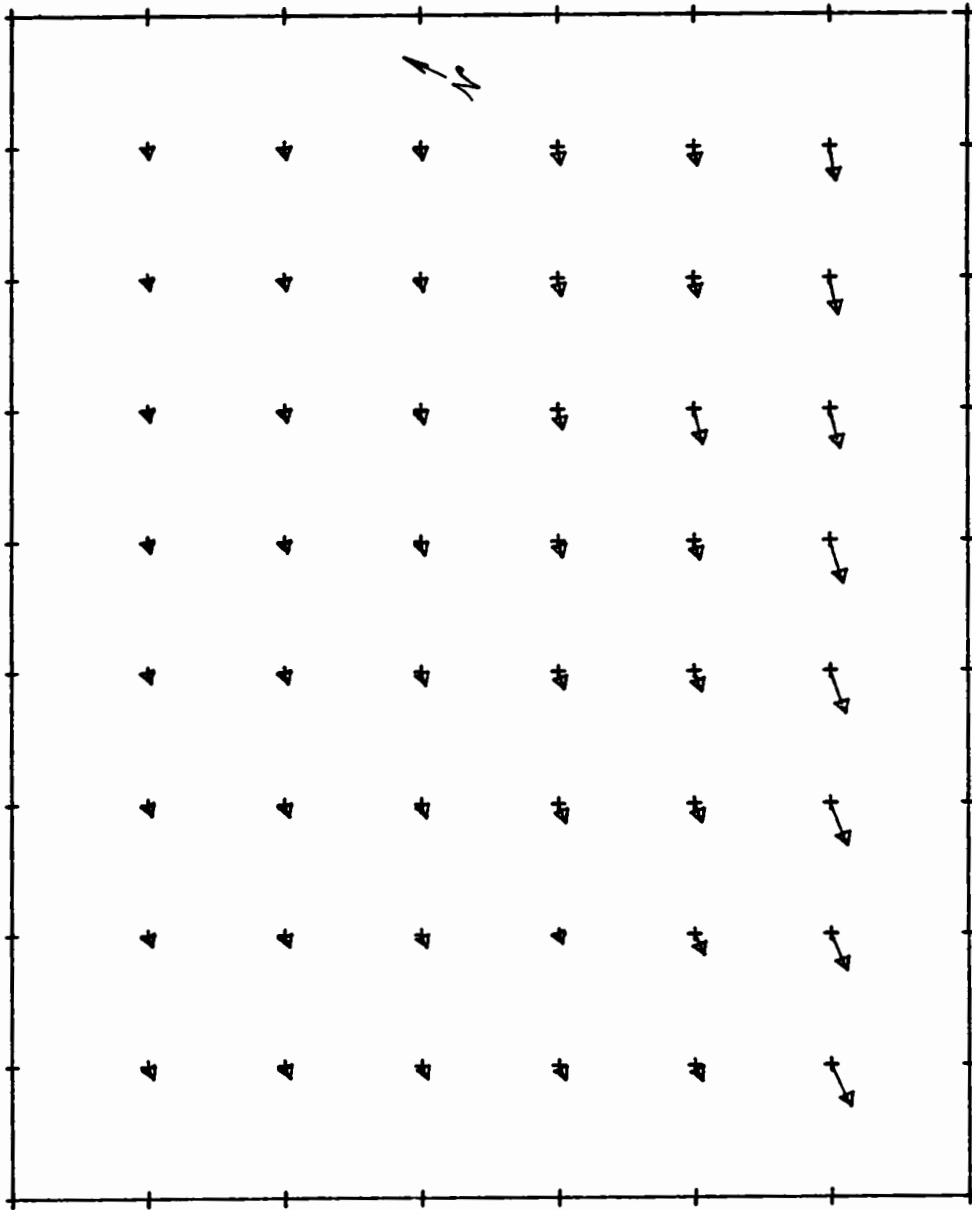


Figure 57. Wind-stress vectors for SPII large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 15 hours.

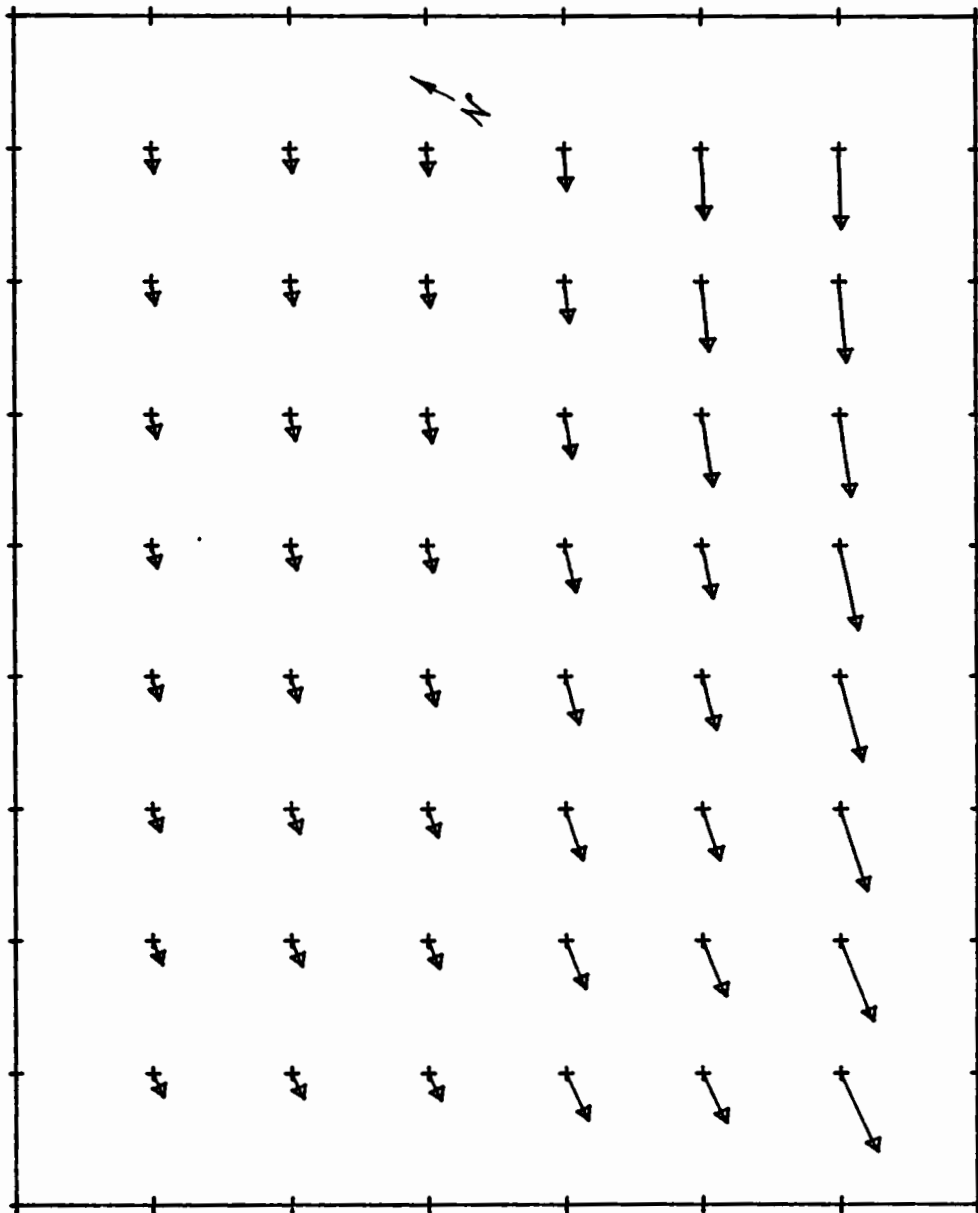


Figure 58. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 20 hours.

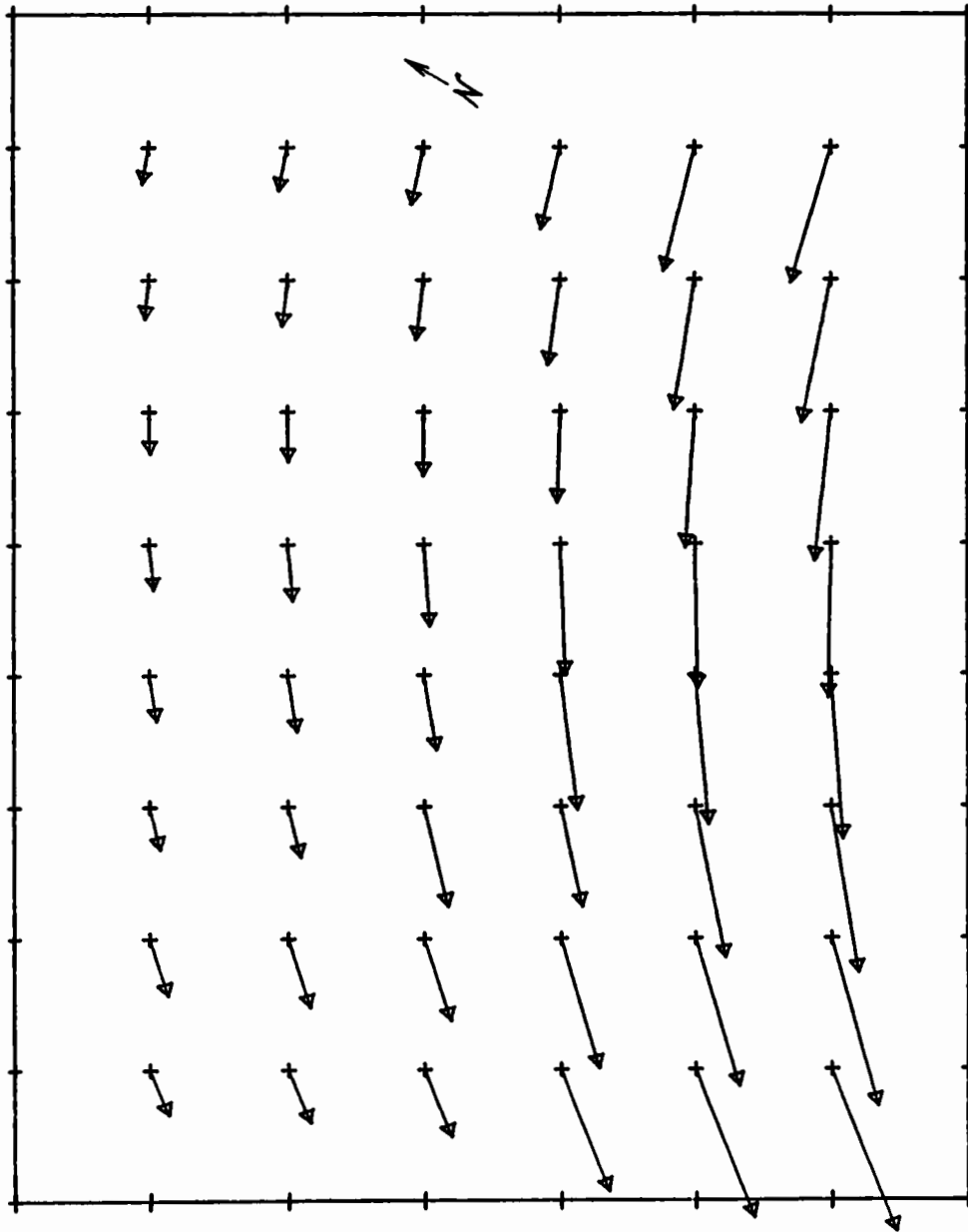


Figure 59. Wind-stress vectors for SPII large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 25 hours.

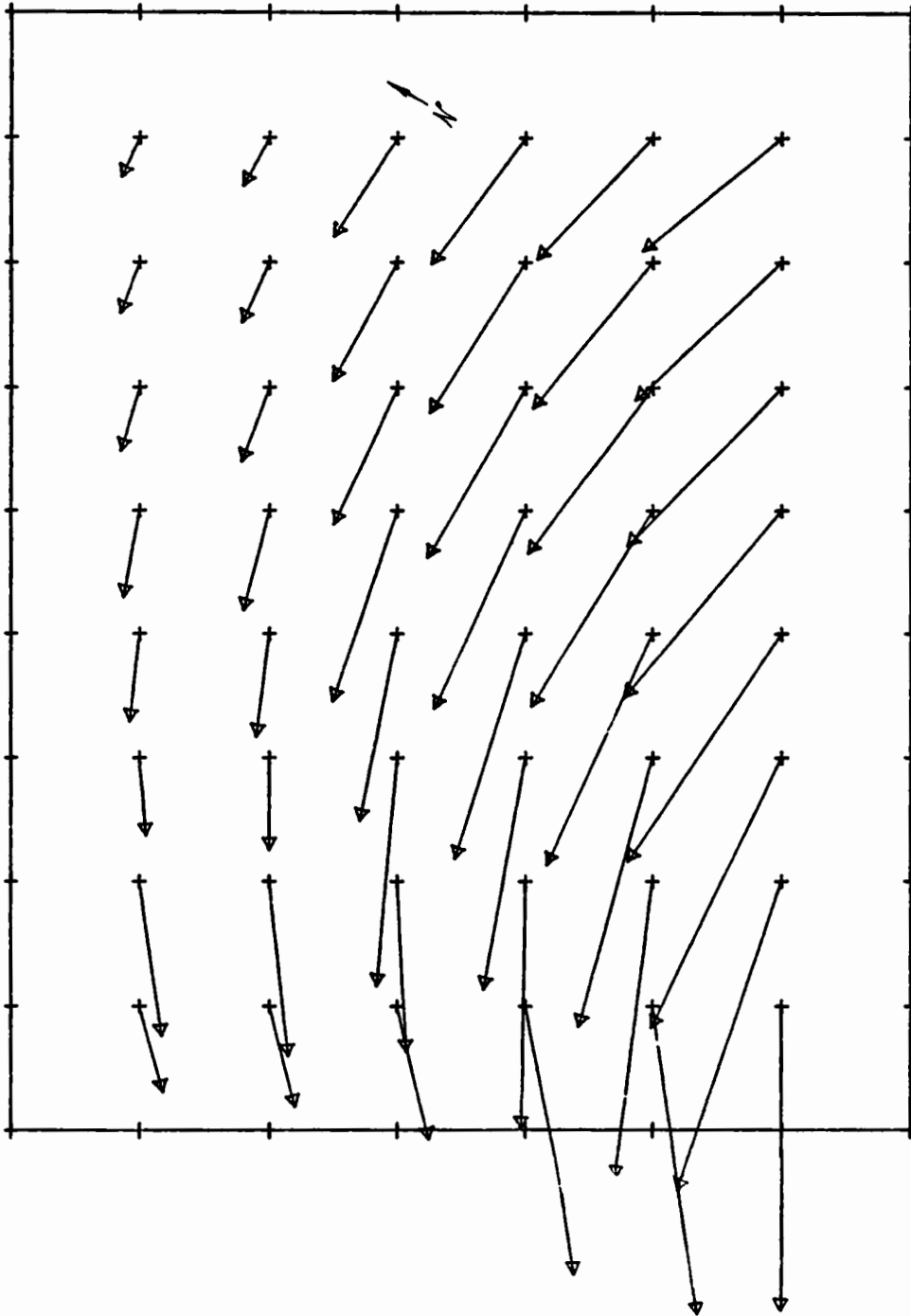


Figure 60. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 30 hours.

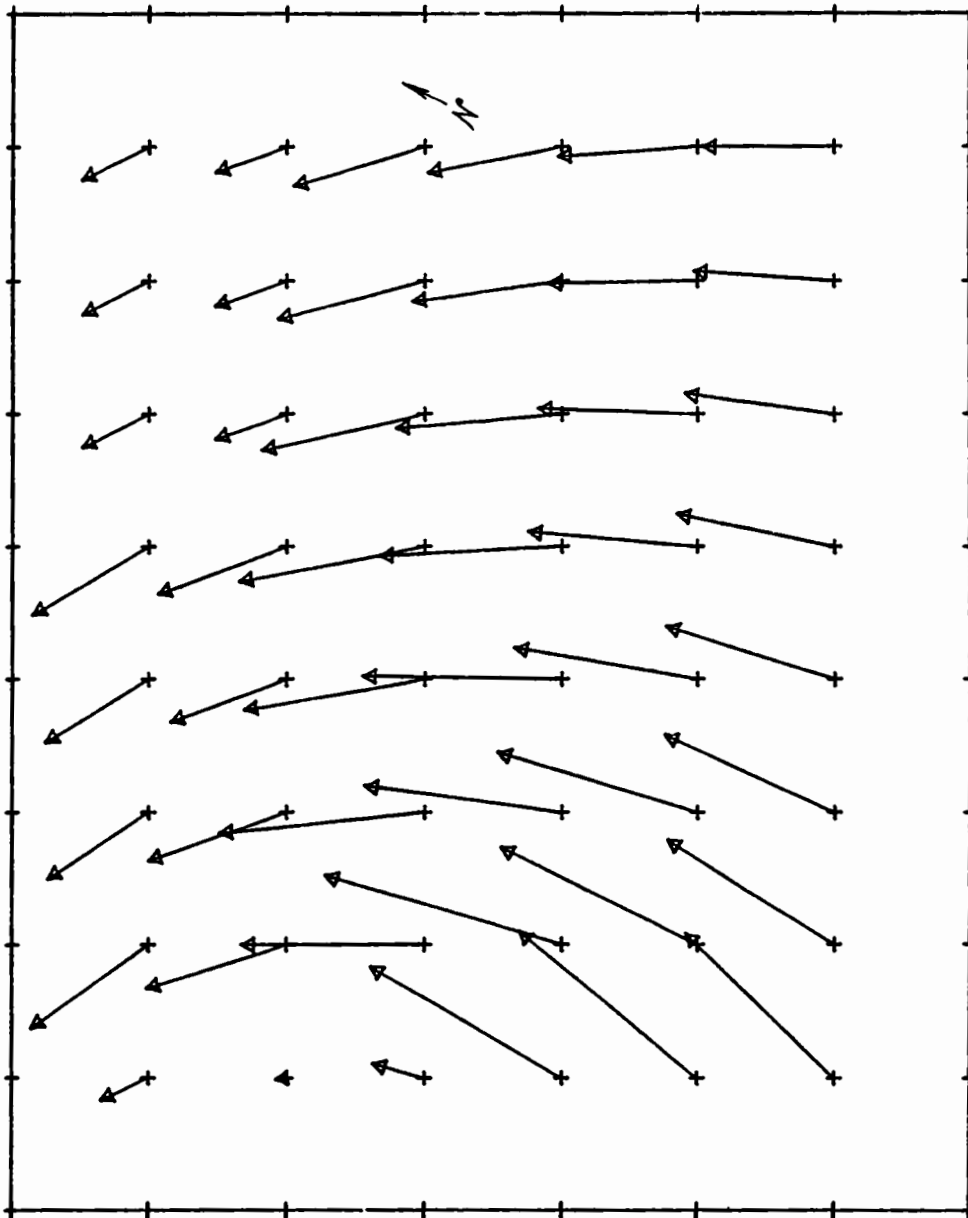


Figure 61. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 35 hours.

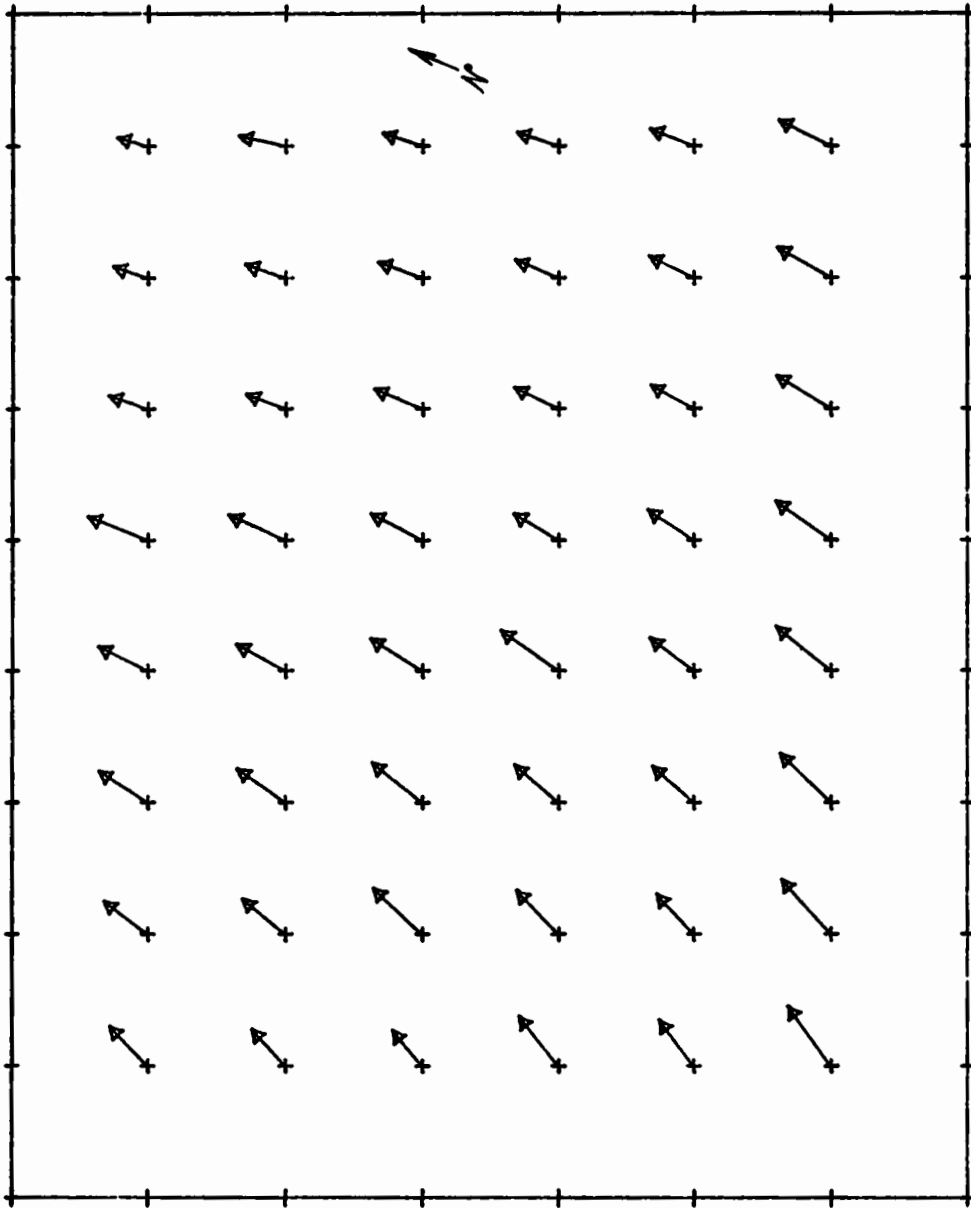


Figure 62. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 40 hours.



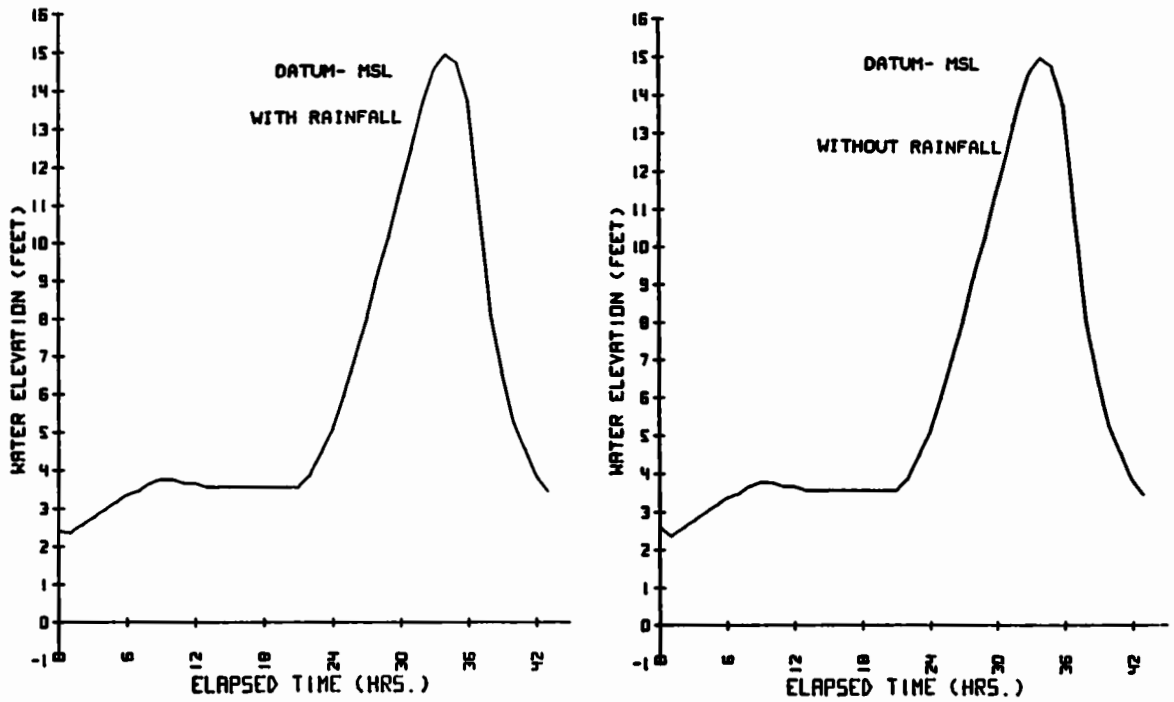


Figure 63. Hydrographs for SPH, LR-MT (with and without rainfall) at Sabine Pass, southwest jetty.

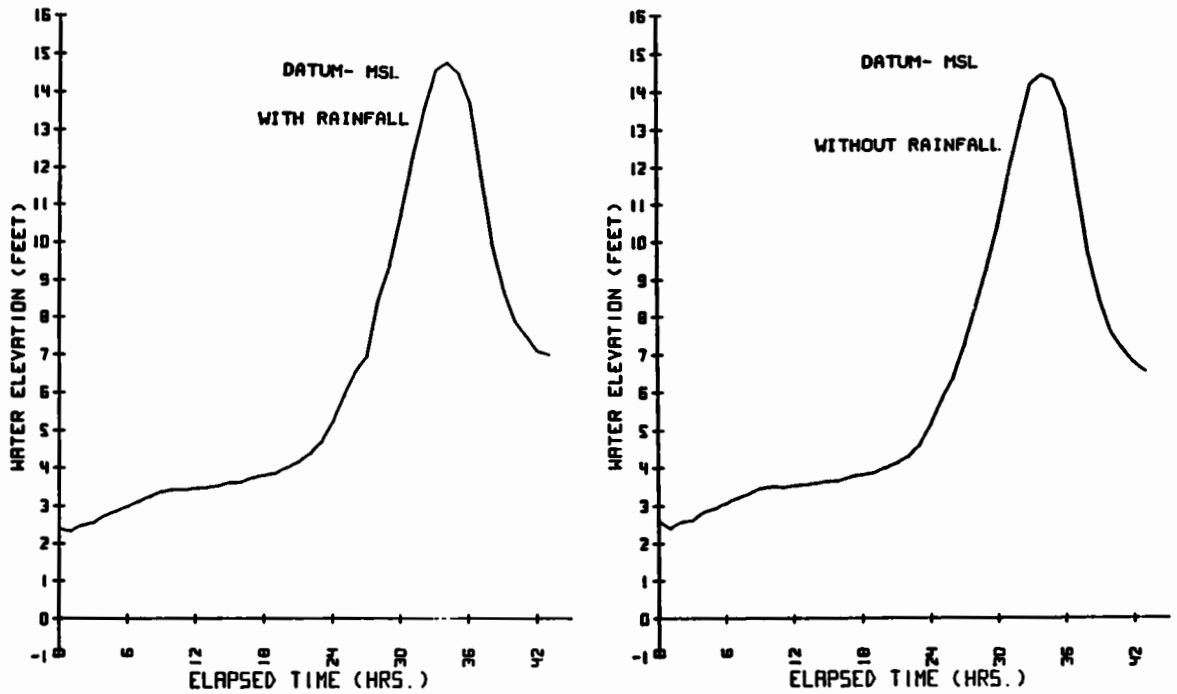


Figure 64. Hydrographs for SPH, LR-MT (with and without rainfall) at Sabine Pass, U.S. Coast Guard Station (FK = 0.0010).

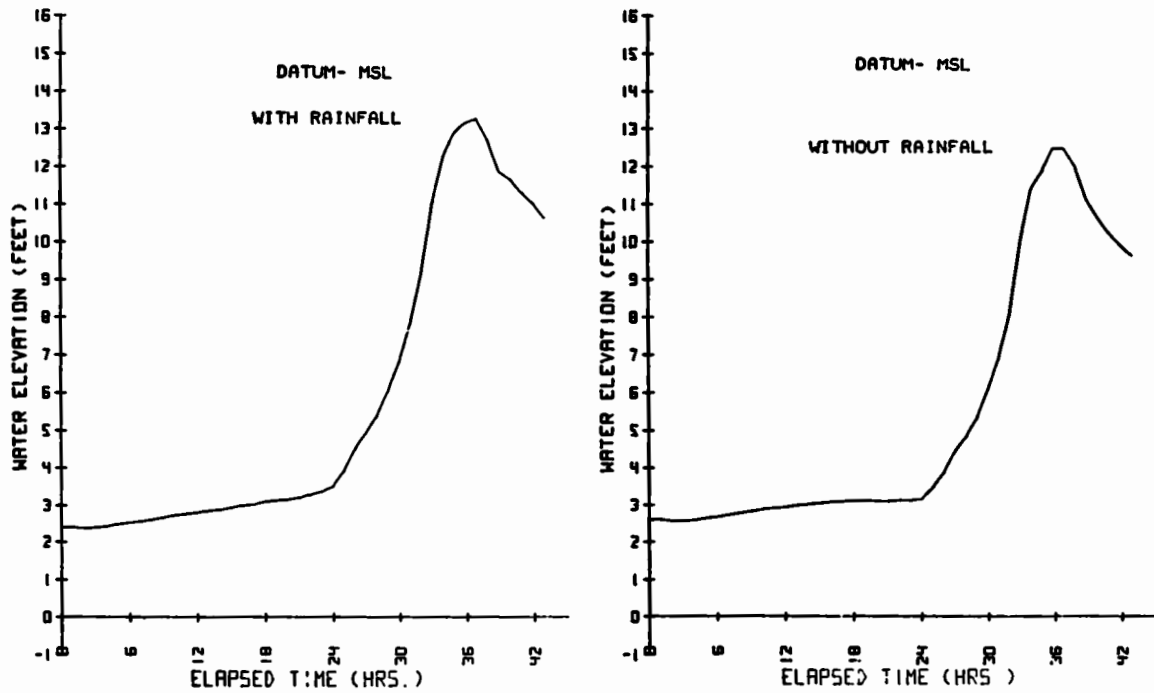


Figure 65. Hydrographs for SPH, LR-MT (with and without rainfall) at Port Arthur (FK = 0.0010).

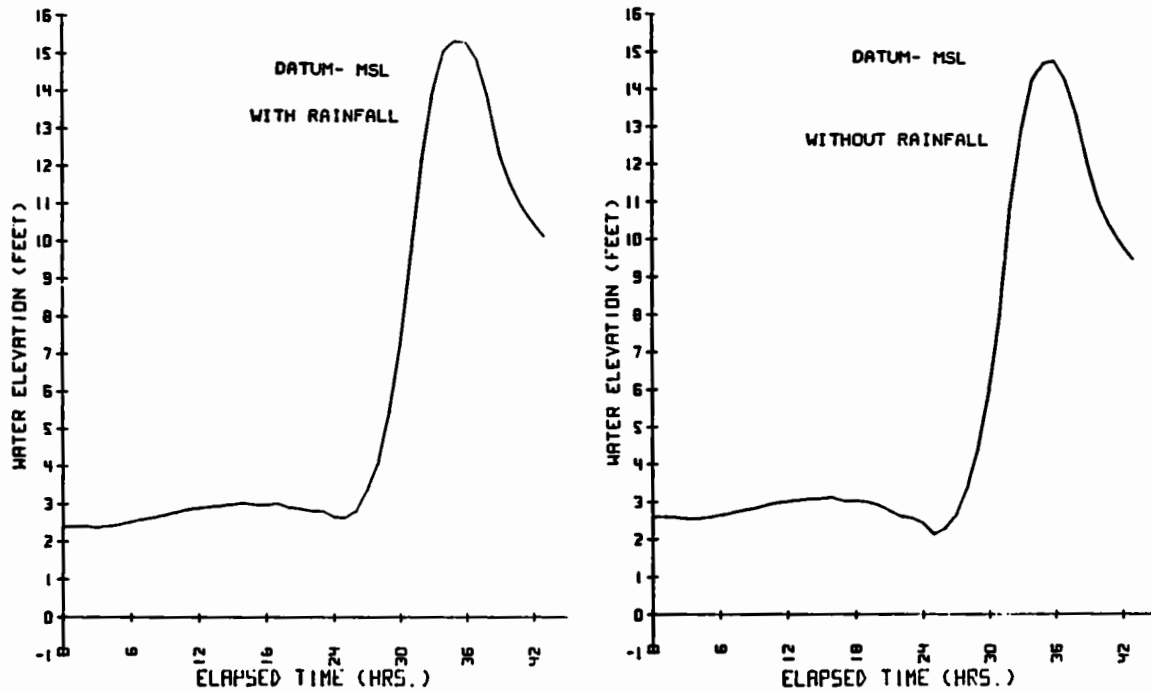


Figure 66. Hydrographs for SPH, LR-MT (with and without rainfall) at north Sabine Lake (FK = 0.0010).

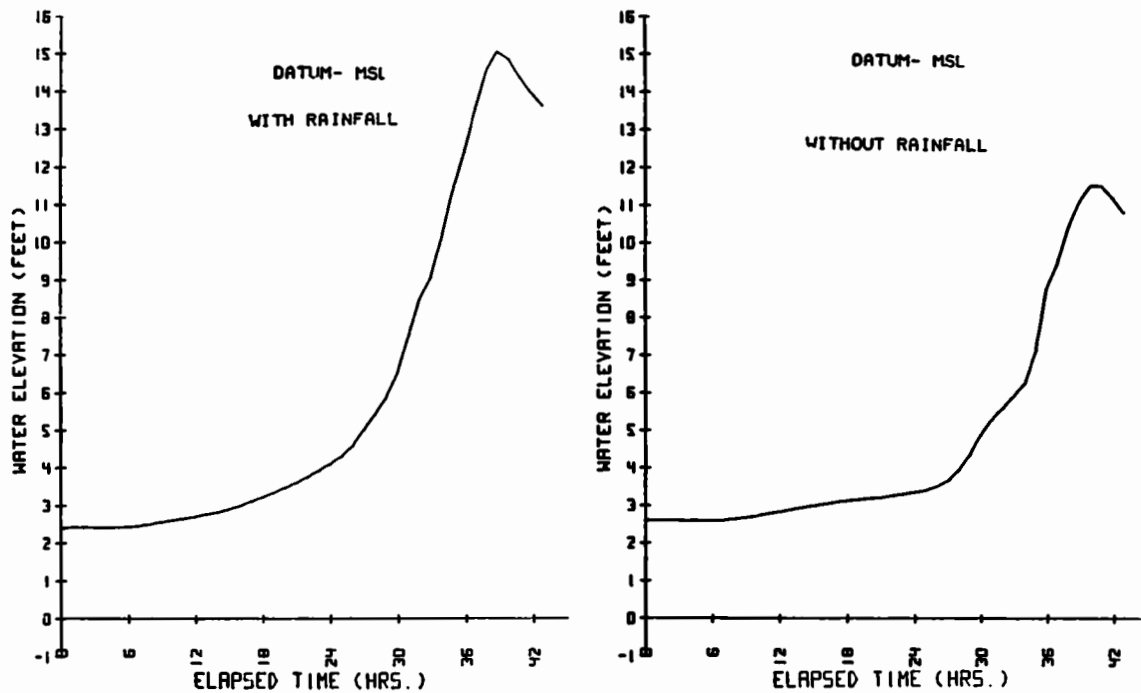


Figure 67. Hydrographs for SPH, LR-MT (with and without rainfall) at Beaumont, Neches River, and Brakes Bayou (FK = 0.0010).

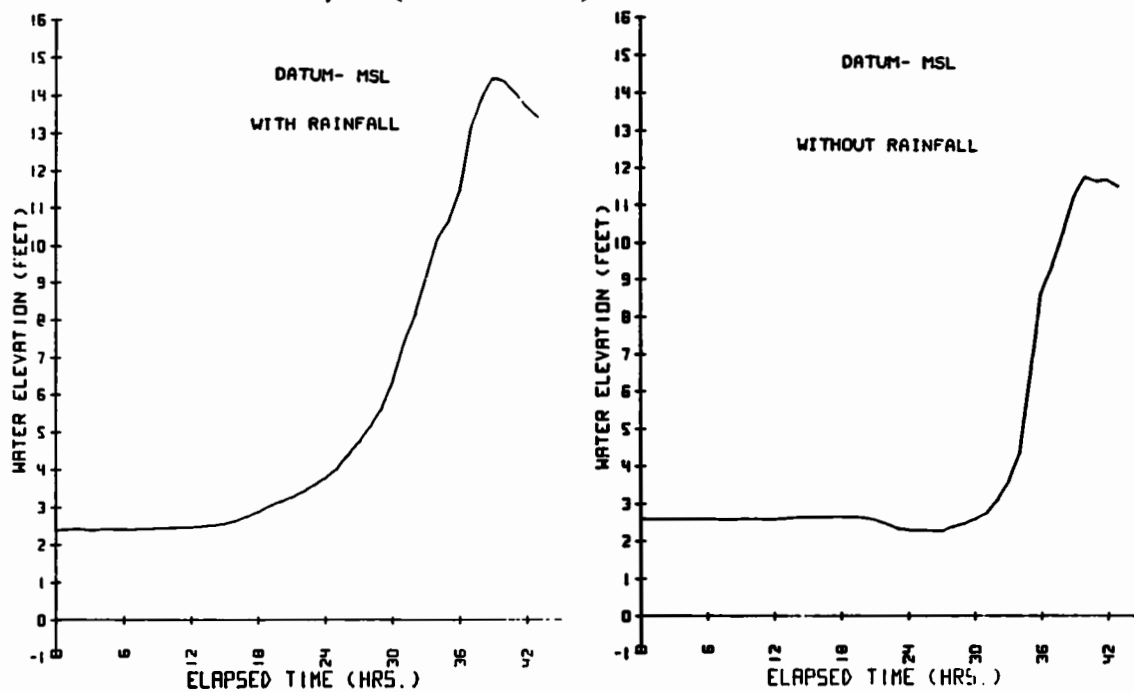


Figure 68. Hydrographs for SPH, LR-MT (with and without rainfall) at Orange Naval Station, Sabine River (FK = 0.0010).

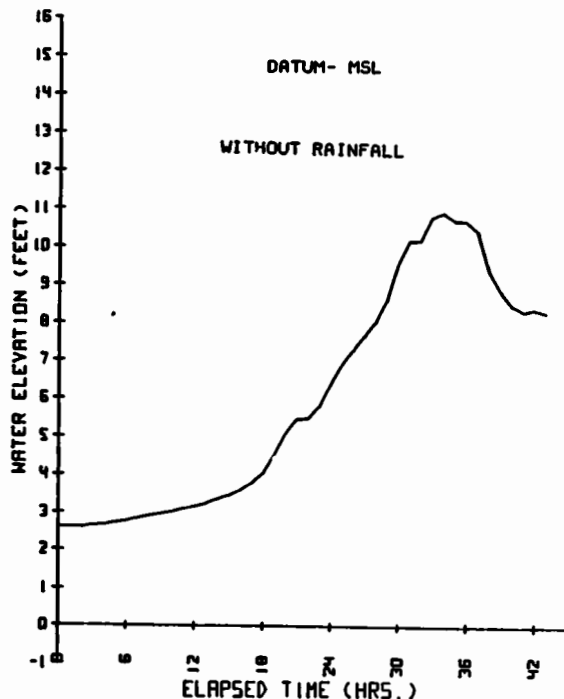
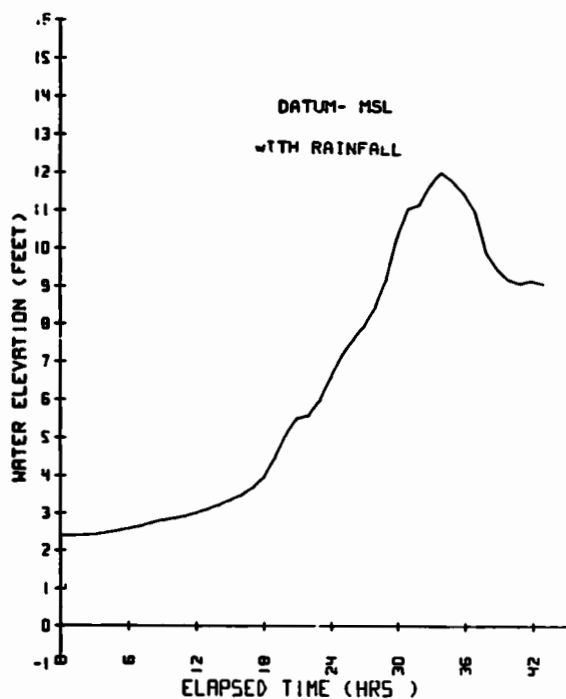


Figure 69. Hydrographs for SPH, LR-MT (with and without rainfall) at west end of Intracoastal Waterway (FK = 0.0010).

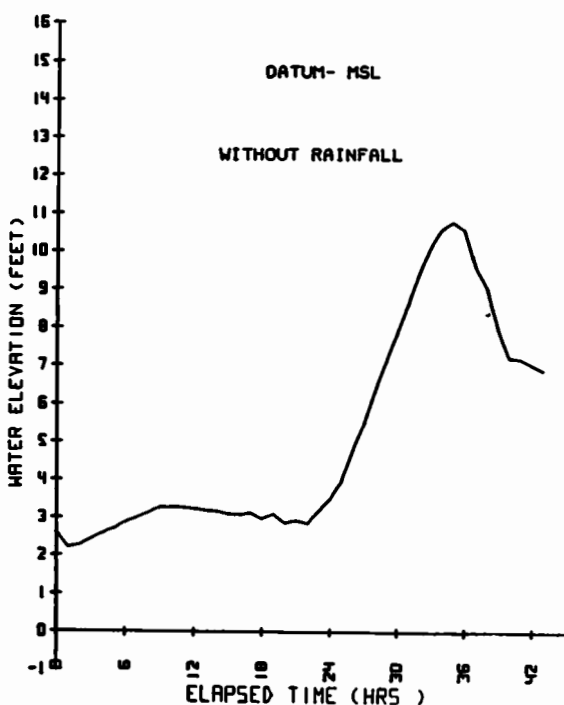
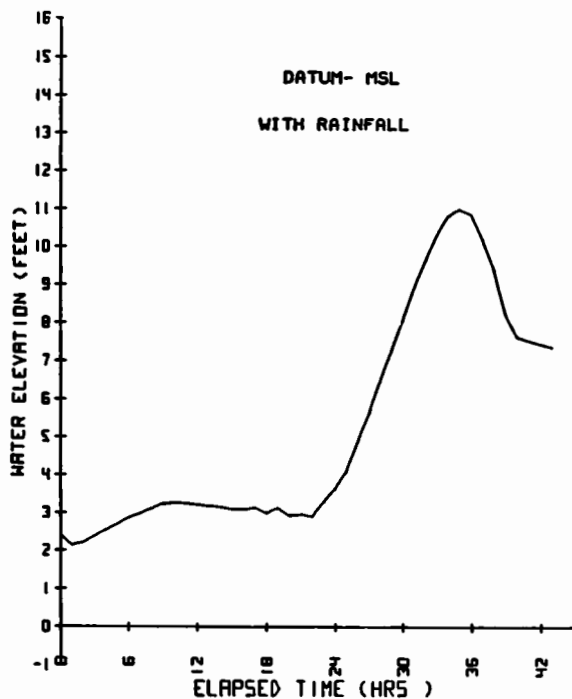


Figure 70. Hydrographs for SPH, LR-MT (with and without rainfall) at Cameron, Calcasieu Pass (FK = 0.0010).

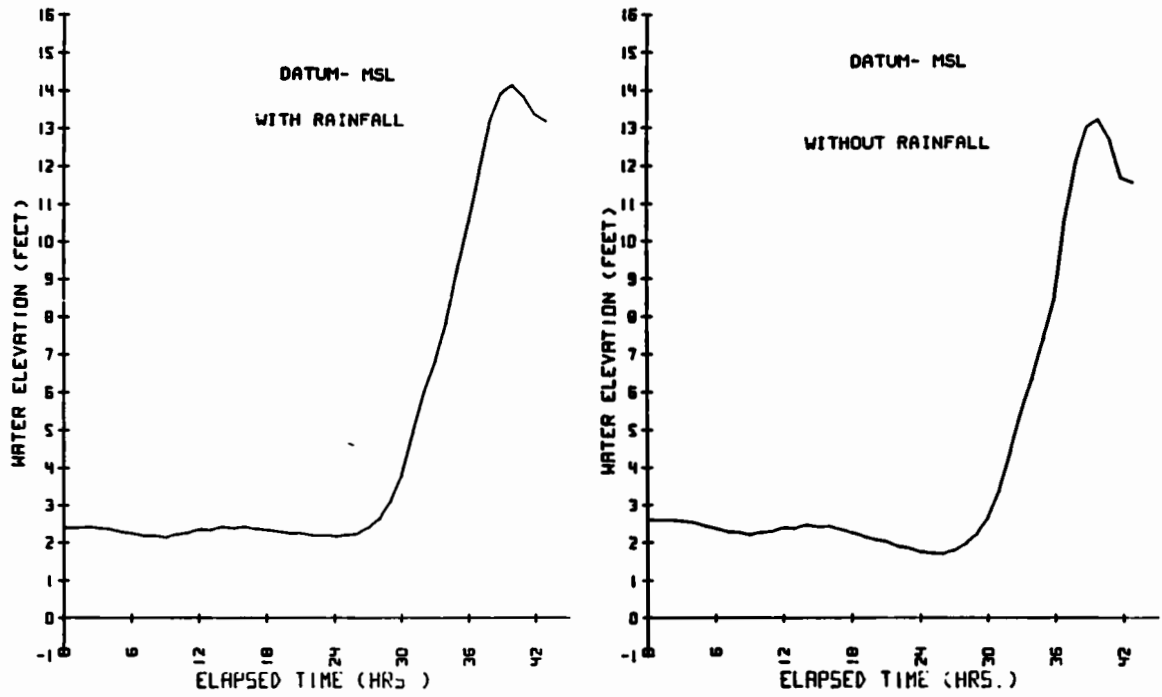


Figure 71. Hydrographs for SPII, LR-MT (with and without rainfall) at Lake Charles, Calcasieu River (FK = 0.0010).

for the greater speed (shorter duration) storm because of the constricted connection to the sea. Port Arthur shows a reduction of 1.1 feet for the MT storm relative to the ST storm; north Sabine Lake appears to show no change. An examination of the wind fields close to the time of the peak surges (Figs. 46 and 61) indicates that a greater wind-induced setup within the lake occurs between Port Arthur and the north Sabine Lake station for the medium speed storm, due to the favorable orientation of the winds near the time of peak surge at the lake entrance.

The response at Beaumont and Orange, both of which are well inland of the main lake area, shows a significant reduction (3.6 and 1.4 feet, respectively) as well as a greater timelag for the faster storm. Moreover, the peak elevations for both of these stations are somewhat less than that at north Sabine Lake for the MT storm in contrast to the situation for the ST storm. The limited access of water to these regions is apparently responsible for this sensitivity to storm duration.

The influence of rainfall and associated runoff from drainage areas well inland is shown very dramatically from a comparison of the second and third sets of peak levels in Table 6, particularly for Beaumont and Orange Naval Station, where runoff produces a differential flooding of 3.6 and 2.8 feet, respectively. A differential of about 0.6 foot due to runoff and rainfall occurs even within Lake Sabine. The effects within Lake Calcasieu and upstream to the northeast are less pronounced due to the smaller runoff.

#### VIII. CONCLUSION

The use of a modified program for inclusion of subgrid scale channels has been demonstrated to be essential for simulation of tides in the upper reaches of a system like the Sabine-Calcasieu region, where the primary connection to locations such as Beaumont, Orange, and Lake Charles is via river channels which would not otherwise be resolved by a grid scheme of the order of a 1-nautical mile scale. Even for conditions of extreme flooding, as occur during hurricanes, the incorporation of the subgrid scale channels provides a degree of freedom for return flow in the presence of water level gradient, which would otherwise not exist in models which exclude subgrid scale channels. The simulation of Hurricane Carla in particular is improved over that attainable with the SURGE I program which did not allow for the subgrid scale channel subroutine.

While programs such as SURGE I can, in principle, simulate the effects of channels, provided the grid scale is of the order of the channel width, the required computer time is usually prohibitive at least for explicit numerical models. Some advantage can be gained in respect to economy by the use of implicit numerical models such as that of Leendertse (1967); however, the accuracy of such schemes when used on a competitive basis, from the standpoint of economy (large time steps) can suffer relative to that which can be achieved with the subgrid scale channel routine. However, the best procedure for such numerical simulation remains to be determined.

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## APPENDIX A

### SURGE II PROGRAM

This appendix includes a complete listing of the SURGE II program. Except for SUBROUTINE CHANL, the program is much the same as that used in Reid and Bodine (1968). It should be emphasized that the coding of calculations of flow and water level for blocks does not include the effect of Coriolis force. Moreover, no attempt has been made to optimize the coding since the original version. The actual new part of the program is embodied in SUBROUTINE CHANL and the way in which the channel computations mesh with the block calculations. Thus, while many users may prefer their own version for calculations over the main grid, it should be possible to incorporate SUBROUTINE CHANL with their own program when applied to systems like the Sabine-Calcasieu region in which allowance for channels is essential.

```

1      PROGRAM SURGE(INPUT,OUTPUT,TAPES=INPUT,TAPES=OUTPUT)
      C
      C
      C
5     C.....C
      C
      C      SURGE II PROGRAM
      C      FOR SIMULATION OF TIDES AND AINO INDUCED SURGES IN BAYS WITH
      C      ALLOWANCE FOR SURROUNDSCALE CHANNELS AND BARRIERS
10    C      DEVELOPED FOR THE U. S. ARMY CORPS OF ENGINEERS, GALVESTON DISTRICT
      C      BY R. O. REYNOLDS, C. VASTANO AND T. J. REID OF
      C      COASTAL STUDIES, INC., P.O. BOX 9064, COLLEGE STATION, TX 77643
      C      DECEMBER, 1975
15    C.....C
      C
      C
      C
20    COMMON/ALX1/ IR(100),Jc(100),IZX(100),IZY(100),ICDXX(100)
1,ICDYY(100),ICDZX(100),ICDZY(100),LDCI(9),LDDJ(9),CIST(24)
2,CMBT(30),R0(A,30),MGP(8),XR(8,6),Y0(8,6),MR(8)
      C
      C      COMMON/ALX2/ IZ(28,20),U(28,20),V(28,20),M(28,20),TIME
25    COMMON/ALX3/ NH,MN,IN,MMAX,IFU,INFLO,IN,JO,AM,KMAX,LMAX,DELX,DELT
1,CDN,FK,MGI,INUT,KI,LIJ,KII,LIJJ,JBL,IRR,KMM,LM,RF,CONST,S
2,IMRO,JHRO,KR,IRTR,IND,NO,KIM,NORT,MTIME,INTIME,NO,IND,GRAB
3,KCMB,DFU,INTER
      C
30    COMMON/ALX4/ MRO(8),CRO(8),K0(24),X1(28,21),Y1(28,21),X2(28,21)
1,Y2(28,21),X(28),Y(28),MGI(28),MG2(28),M01(8),MR(9),VM(29)
2,MG(28),MR2(8)
      C
35    COMMON/ALX5/ ICG(130),JCG(130),I-CX(130),I-CY(130),IZCX(130)
1,IZCY(130),ICXP(130),ICXN(130),ICYP(130),ICYN(130),MC(130),MP(130)
2,KCM,KCY(130),KCY(130),KCM(130),UCY(130),JCF(130),KRI(130),IBN
3,KEM(2,130),VCT(130),VCF(130),ACGX(130),ACGY(130),KXP(130)
4,KCP(130),KLR(90),KLM,IFC(130),FC
40    COMMON/ALX6/ MGRY(6,25), MRRY(8,25), XRY(6,6,25), YRY(8,6,25)
      C
      C      COMMON/ALX7/ IEND,NF,IBL,NJ,ALPHA(40)
45    COMMON/ALX8/ MS(9,72), GS(6,72), TIME(72)
      C
      C      COMMON/ALX9/ KZ,LZ,NHRO,C1,L2,C3,IMH,JO,AM,KMAX,IFX1,IFXC,IFIRST
1,JAIND,ME1,XND,ME-3,XNJRT, C1,N4IN,AM,AL,LIJ,KMIK
      C
50    COMMON/ALX1/ AGAGE,NFLO,IGAGE(12),JGAGE(12),FLO(6),XMI,KMAX
      C
      C      CODE AND ICARD(0) CARD ONE OF INPUT, CONTROLS INITIAL
      C      COMPUTATIONS AND READ ACTIONS AS FOLLOWS
      C

```

```

55      C          ICARD=0 THIS IS FOR STARTING A PROBLEM
      C          CARD INPUT IS ICARD FOLLOWED BY THE FULL GALV
      C          DATA DECK LESS THE BLANK CARD AT THE END
      C          PLUS THE GALV LIST DECK
      C          PLUS THE CHANNEL DATA DECK (IF NCM.GT.ZERO)

60      C          ICARD=1 THIS IS FOR CONTINUING A PROBLEM
      C          CARD INPUT IS ICARD FOLLOWED BY THE CONTIN DECK
      C          THEN FOLLOWED BY THE FIRST FOUR DATA CARDS OF THE
      C          GALV DATA DECK. (THE CONTIN DECK IS OUTPUTTED FROM
      C          A PREVIOUS RUN)

65      C          CODE WORD (IBL) IS THE STARTING COLUMN IN THE LISTING OF M
      C          CODE WORD (NCM) IS THE NUMBER OF BLOCKS WITH CHANNELS
      C          CODE WORD (NWIN) IS NEGATIVE FOR NO WIND FIELD (OMITS CARD INPUT
      C          OF X,Y FIELDS AND USES FORMAT 910 FOR MG INPUT)
70      C          INTER IS THE INTERVAL USED IN THE SAVE M OPERATION
      C          NGAGE IS THE NUMBER OF HYDROGRAPHS TO BE SAVED
      C          NFLOW IS THE NUMBER OF FLOW GRAPHS TO BE SAVED
      C          IMIN AND IMAX ARE THE DESIRED LOWER AND UPPER LIMITS OF M FOR
      C
75      C          GRAPHICAL OUTPUT
      C          PRINT 10
      C          10 FORMAT (1-1)
      C          READ 10, MCARD
      C          1 FORMAT(11.11,10I4)
80      C          IF(ICARD.EQ.0) GO TO 2
      C          HEADS PREVIOUS RESULTS FOR CONTINUATION OF PROBLEM
      C          CALL CONTIN(1)
      C          INTIME=TIME
      C          2 CONTINUE
85      C          READ 10, ISENT,IBL,NCM,NOWIND,INTER,NGAGE,NFLOW,IMIN,IMAX
      C          XMAX = IMAX
      C          XMIN = IMIN
      C
90      C          50 FORMAT(3X, ICARD=,I2, IBL=,I3, NCM=,I4, (NOWIND=,I3
      C          1, IINTER=,I3, NGAGE=,I3, NFLOW=,I3, IMIN=,I3, IIMAX=,I3,/)
95      C          51 FORMAT(3X, ICONT=,I2, IINTIME=,I3, I X=,I4, I XMIN=,I4
      C          1, I XMAX=,I4, I NFUS=,I4, I IDUT=,I4, I INFLO=,I4)
      C          52 FORMAT(3X, ICONT=,I2, I IML=,I3, I J=,I3, I XMAX=,I3
      C          1, I XMIN=,I3)
      C          53 FORMAT(3X, ICONT=,I2, I DELTA=,F4.1, I Y MI DELT=,F4.0,
      C          1 I SEC CORR=,F6.3, I FAX=,F7.4, I FC=,F7.4, I HGT=,F6.3, I FT(1)
      C          54 FORMAT(3X, ICONT=,I2, I KIS=,I3, I LJS=,I3, I KITS=,
      C          1, I3, I LJS=,I3, I JAL=,I3, I JAS=,I3,/)
      C          55 FORMAT(3X, IML=,I3, I IML=,I3, I JML=,I3, I ZML=,I5
      C          1, I COSX=,I4, I COSY=,I4, I COSZ=,I4, I COSX=,I4, I COSY=,I4)
100      C          57 FORMAT(1 /,3X, ICONT=,I2, I IML=,I3, I JML=,I3, I X=,I3
      C          1, I IML=,I3, I IML=,I3, I NOW=,I3, I IML=,I3, I IML=,I3)
      C          59 FORMAT(3X, ICONT=,I2, I RF=,F6.3, I CONST=,F7.4, I S=(,F6.5)
105      C          75 FORMAT(3X, ICONT=,I2, I 5, I 3X, (OBSERVED DATA= 2 VALUES IN TERMS OF FEET,
      C          1 CC VALUES ARE TIMES 1000))
      C          76 FORMAT(1 /,3X, ICONT=,I2, I 3X, I BLOCK TOPOGRAPHY= 2 VALUES IN FEET())

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```

79 FORMAT(1 /,3X,(IDNT= 9(,3X,13,( PATHS OF I,J FOR RUNOFF LOCATIONS)MAIN0054
11) MAIN0055
80 FORMAT(1 /,3X,(IDNT= 10(,3X,(PERCENT RAINFALL EACH MPTIME() MAIN0056
110 81 FORMAT(1 /,3X,(IDNT= 11(,3X,(CHANNEL STRESS VALUES AT MPTIME() MAIN0057
82 FORMAT(1 /,3X,(IDNT= 12(,3X,13,( SETS OF RUNOFF VALUES IN CFS FOR MAIN0058
11,13,( MPTIMES() MAIN0059
83 FORMAT(1 /,3X,IMGR FOLLOWED BY HOR ARRAY (IN FEET) AT MTIME=(,14)MAIN0060
115 84 FORMAT(1 /,3X,(X VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME() MAIN0061
1E(,14) MAIN0062

C
  PRINT 50, ICARD,IBL,KCM,NOWIND,INTER,NGAGE,NFLC,IMIN,IMAX MAIN0063
  READ 100, IDNT1,TIME,AM,MMIN,MMAX,NFU,IGUT,INFLD MAIN0064
  PRINT 51, IDNT1,TIME,AM,MMIN,MMAX,NFU,IGUT,INFLD MAIN0065
120 100 FORMAT (11,2X,15,9(3X,15)) MAIN0066
  101 FORMAT(1X,12,15,9(3X,15)) MAIN0067
  IF(IDNT1=1)1000,150,1000 MAIN0068
  150 READ,100, IDNT1,IM,JM,KM,KMAX,LMAX MAIN0069
  PRINT 52, IDNT1,IM,JM,KM,KMAX,LMAX MAIN0070
  KM=KMAX=1 MAIN0071
  LML=LMAX=1 MAIN0072
  IF(IDNT1=2)1000,200,1000 MAIN0073
  200 HEAD 250, IDNT1,DELX,DELTC,DO,FK,FC,NGI MAIN0074
  PRINT 53, IDNT1,DELX,DELTC,DO,FK,FC,NGI MAIN0075
  DELX=DELX=0.80, MAIN0076
  250 FORMAT (11,27,0,9FB,0) MAIN0077
  260 FORMAT (1X,11,27,3,9FB,0) MAIN0078
  IF(IDNT1=3)1000,295,1000 MAIN0079
  295 IF(ICARD,FG,C) GO TO 3 MAIN0080
  135 YTIME=TIME
  PRINT 10 MAIN0081
  GO TO 745 MAIN0082

C
  3 CONTINUE MAIN0085
  INTIME=TIME MAIN0086
  300 READ 100, IDNT1,MI,LJ,MI,LJJ,JBL,JBP MAIN0087
  PRINT 54, IDNT1,MI,LJ,MI,LJJ,JBL,JBP MAIN0088
  IF(IDNT1=4)1000,350,1000 MAIN0089
  145 350 IF(NFU=MMAX=MM)365,360,365 MAIN0090
  360 IF(NFU=MMIN=MM)365,360,365 MAIN0091
  365 PRINT 366 MAIN0092
  366 FORMAT (2,MMIN OR MMAX IN ERROR ) MAIN0093
  368 CONTINUE MAIN0094
  IF ((MI=KMM)=IM) 440,380,370 MAIN0095
  150 370 IF ((MI=(KMM=1))=IM) 390,400,440 MAIN0096
  380 IF ((LJ=LMM)=JM) 440,400,390 MAIN0097
  390 IF ((LJ=(LMM=1))=JM)400,440,440 MAIN0098
  400 IF (KMM = MI) 410,410,400 MAIN0099
  410 IF (LMM = LJ) 450,450,460 MAIN0100
  155 440 PRINT 450 MAIN0101
  450 FORMAT (1M,42M K,L RANGE NOT CONSISTENT WITH I,J RANGE////) MAIN0102
  GO TO 400 MAIN0103

C
  460 PRINT 470 MAIN0104

```

```

160 470 FORMAT (1X,40X 'RANGE OF K OR L TOO LARGE FOR PROGRAM'////)      MAIN0105
480 STOP                                                                    MAIN0106
490 CONTINUE                                                                  MAIN0107
    IF(KM.EQ.0) GO TO 501                                                    MAIN0108
    PRINT 74                                                                    MAIN0109
165 DO 500 K=1,KM                                                            MAIN0110
    READ 100, IONT1, I3(K), J3(K), IZ(K), TZ(K), IC00X(K), IC00Y(K), IC0SX( MAIN0111
2K), IC0SY(K)                                                                MAIN0112
    PRINT 85, K, I3(K), J3(K), IZ(K), TZ(K), IC00X(K), IC00Y(K), IC0SX( MAIN0113
14), IC0SY(K)                                                                MAIN0114
170 IF(IONT1=9)1000,500,1000                                                MAIN0115
500 CONTINUE                                                                  MAIN0116
501 CONTINUE                                                                  MAIN0117
    PRINT 74                                                                    MAIN0118
    DO 550 I=1,IM                                                            MAIN0119
175 READ 100, IONT1, (I2(I,J), J=1,10)                                       MAIN0120
    PRINT 101, I, (I2(I,J), J=1,10)                                         MAIN0121
    IF(IONT1=6)1000,500,1000                                                MAIN0122
540 READ 100, IONT1, (I2(I,J), J=11,JM)                                       MAIN0123
    PRINT 101, I, (I2(I,J), J=11, JM)                                       MAIN0124
180 IF(IONT1=8)1000,550,1000                                                MAIN0125
550 CONTINUE                                                                  MAIN0126
    READ 100, IONT1, I*NO, J*RO, K*ISTR, I*NO, C*ACTM, N*ORT                MAIN0127
    PRINT 97, IONT1, I*NO, J*RO, K*ISTR, I*NO, C*ACTM, N*ORT                MAIN0128
    IF (IONT1=7)1000,500,1000                                                MAIN0129
185 560 READ 250, IONT1, F, C, S*ST, S                                       MAIN0130
    PRINT 98, IONT1, F, C, S*ST, S                                       MAIN0131
    IF(IONT1=9)1000,570,1000                                                MAIN0132
570 PRINT 99, I*NO                                                         MAIN0133
    READ 100, IONT1, (L*OI(JJJ), L*OJ(JJJ), JJJ=1, K)                       MAIN0134
190 PRINT 101, IONT1, (L*OI(JJJ), L*OJ(JJJ), JJJ=1, K)                       MAIN0135
    IF(I*NO.LT.8) GO TO 575                                                  MAIN0136
    READ 100, IONT1, (L*OI(JJJ), L*OJ(JJJ), JJJ=8, I*NO)                   MAIN0137
    PRINT 101, IONT1, (L*OI(JJJ), L*OJ(JJJ), JJJ=8, I*NO)                   MAIN0138
195 575 CONTINUE                                                                MAIN0139
    IF(IONT1=9)1000,580,1000                                                MAIN0140
580 IF(CURT=0)590,590,000                                                  MAIN0141
590 IF(AIM=0)1000,200,000                                                  MAIN0142
600 PRINT 010                                                                MAIN0143
610 FORMAT (4X, K1= OR NOT EXCEEDS NO-, OR ACTM EXCEED NO- )             MAIN0144
200 C
620 IF(NC-INDICE, N) GO TO 025                                              MAIN0145
    J*NO= 1                                                                    MAIN0146
    GO TO 090                                                                  MAIN0147
625 PRINT 99                                                                    MAIN0148
205 DO 030 K1=1,2                                                            MAIN0149
    K2=1000+K1-1                                                            MAIN0150
    K2=100+K1                                                                MAIN0151
    READ 250, IONT1, (DIST(K), K=22, K23)                                    MAIN0152
    PRINT 240, IONT1, (DIST(K), K=22, K23)                                    MAIN0153
210 IF (I2(I,1)=)999,030,999                                              MAIN0154
630 CONTINUE                                                                  MAIN0155
    READ 250, IONT1, (DIST(K), K=21, 24)                                    MAIN0156

```

```

        PRINT 260,INT1,(DIST(K),4021.24)
        IF (INT1=1)990,640,999
215      640 K33=2/14
           IF (K4,EQ,A) GO TO 690
           IF (K33) 671,670,671
           K32 = 0
220      670 GO TO 672
           671 PRINT 81
           DO 680 K=1,K33
           K31=100*(K+1)+1
           K32=K+K
225      HEAD 250, INT1,(CHST(K4),K42=K31,K32)
           PRINT 240,INT1,(CHST(K4),K42=K31,K32)
           IF (INT1=1)990,640,999
           640 CONTINUE
           672 K31=K32+1
           HEAD 250, INT1,(CHST(K4),K42=K31,K32)
230      PRINT 240,INT1,(CHST(K4),K42=K31,K32)
           IF (INT1=1)990,690,999
           C
           690 PRINT 82, I=0, J=0
           DO 700 J=1,J=0
235      READ 250, INT1,(R0(I,J),I=1,I=0)
           PRINT 241,INT1,(R0(I,J),I=1,I=0)
           241 FORMAT(1X,I1,F7.0,9F2.0)
           IF (INT1=2)990,700,999
           700 CONTINUE
           WRITE
           I=0
240      CONTINUE
           705 CONTINUE
           IF (I=INT(LT,0)) GO TO 900
           C
245      710 HEAD 100, INT1,XTIME
           720 IF (INT1=1)900,730,999
           730 IF (XTIME=0)740,700,740
           740 PRINT 750,XTIME
           750 FORMAT(1X,XTIME,ER=00 AT .15)
250      760 PRINT 83, XTIME
           HEAD 250, INT1,(MGR(K),K=1,K=MAX)
           PRINT 260,INT1,(MGR(K),K=1,K=MAX)
           IF (INT1=6)990,770,999
           770 HEAD 250, INT1,(MGR(J),J=2,9)
           PRINT 240,INT1,(MGR(J),J=2,9)
           IF (INT1=6)990,780,999
           780 PRINT 84, XTIME
           DO 790 K=1,K=MAX
           HEAD 250, INT1,(K*(K+L),L=1,L=MAX)
250      PRINT 241,INT1,(K*(K+L),L=1,L=MAX)
           IF (INT1=6)990,790,999
           790 CONTINUE
           DO 800 K=1,K=MAX
           HEAD 250, INT1,(Y*(K+L),L=1,L=MAX)
260      PRINT 260,INT1,(Y*(K+L),L=1,L=MAX)

```

\*A1N0157  
 \*A1N0158  
 \*A1N0159  
 \*A1N0160  
 \*A1N0161  
 0000001N0162  
 0000001N0163  
 \*A1N0164  
 \*A1N0165  
 \*A1N0166  
 \*A1N0167  
 \*A1N0168  
 \*A1N0169  
 \*A1N0171  
 \*A1N0172  
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 \*A1N0196  
 \*A1N0197  
 \*A1N0198  
 \*A1N0199  
 \*A1N0200  
 \*A1N0201  
 \*A1N0202  
 \*A1N0203  
 \*A1N0204

```

      IF (INTJ=71000.000+999)
      GO TO 1015
C.....ADJUST AINC FIELD.....
      DO 802 K=1,3
      DO 802 L=1,4
      XR(K,L)=0.0000(K,L)
      YR(K,L)=0.0000(K,L)
      802 CONTINUE
C.....
      801 IT=1
      DO 810 I=1,M*MAX
      MGRM(I,IT)=MGO(I)
      DO 820 J=1,L*MAX
      XRM(I,J,IT)=XO(I,J)
      820 YRM(I,J,IT)=YO(I,J)
      810 CONTINUE
      DO 830 J=2,A
      830 MRM(J,IT)=MOR(J)
      IF (MTRF=M*MAX) 710=1015+1015
C
      900 M=1
      DO 901 K=1,IMRO
      901 MRU(K)=R(M,K)
      PRINT 910
      910 FORMAT(1)
      PRINT 915
      905 READ 910, IGA, MTIME*(M(1,J)+J=1,12)
      910 FORMAT(12,14,12F0.2)
      915 FORMAT(1 THE FOLLOWING ARE HOURLY TIDE LEVELS OUTSIDE MAIN(/
      1 ( PASS FOR TIDE CALIBRATION WITH NO AINC(/))
      PRINT 910, IGA, MTIME*(M(1,J)+J=1,12)
      1001 IG
      IF (IGA .NE.1) GO TO 999
      MLE=TIME+1
      MUM=TIME+12
      DO 920 J=1,12
      M=ML+J=1
      DO 920 I=1,M*MAX
      920 MGRM(I,M)=M(1,J)
      DO 930 I=2,A
      930 MRM(I,M)=M(1,J)
      940 CONTINUE
      IF (MUM,LT,M*MAX) GO TO 905
C
      DO 950 K=1,M*MAX
      DO 950 L=1,L*MAX
      XR(K,L)=0.0
      YR(K,L)=0.0
      950 CONTINUE
      GO TO 1015
C
      999 I=TIME+1010
      100 PRINT 1010,IG
      1010 FORMAT(1 ERROR IN DATA = MISSING (IF=XX+M(ADD)
      STOP
C
      1010 CALL PART 2
C
      1020 STOP
      END
  
```

```

1      C
      C
      SUBROUTINE PART 2                                PT2=C001
9      COMMON/ALX1/ IP(100),JP(100),IZX(100),IZY(100),ICGK(100)
      1,ICDXY(100),ICDSX(100),ICDSY(100),LGOI(A),LGOJ(A),DIST(24)
      2,CNST(30),QD(A,30),MGH(8),XR(A,6),YD(A,6),MOM(M)
      COMMON/ALX2/ IZ(2A,20),U(2A,20),V(2A,20),W(2A,20),TIME
      COMMON/ALX3/ IM,MIN,MAX,NFU,INFLO,IM,JM,KY,KYAX,LMAX,DELX,DELT
10     1,CCO,PK,MGI,IOPT,KI,LIJ,KII,LUJ,JHL,JP,KY,LM,RF,CONST,S
      2,IMR0,JMRC,KR,ISTR,IND,NOA,KIM,NORT,TIME,INT,IE,NC,IND,GRAV
      3,KCM,DFU,INTFR
      COMMON/ALX4/ HON(8),CRO(6),KB(24),X1(20,21),Y1(20,21),X2(20,21)
      1,Y2(2A,21),X(2A),Y(20),MGI(20),MG2(20),MJI(8),MRA(9),UN(20)
      2,MG(2A),MHP(2)
15     COMMON/ALX5/ ICG(130),JCG(130),ICX(130),ICV(130),IZCX(130)
      1,IZCY(130),QCV(130),QCV(130),QCV(130),QCV(130),MC(130),MP(130)
      2,KCM,KCX(130),KCY(130),KCM(130),UPT(130),UCF(130),KPI(130),IMC
      3,KCY(2,130),VCT(130),VCF(130),ADGX(130),ADGV(130),KCP(130)
20     4,KCVP(130),KLR(50),KLM,IFC(130),FC
      COMMON/ALX6/ MGR(8,25),MHW(4,25),X2(A,A,25),YR(8,6,25)
      COMMON/ALX7/ IEND,NE,INL,NJ,ALPHA(4)
      COMMON/ALX8/ MS(9,72),OS(6,72),TIME(72)
      COMMON/ALX9/ KZ,LZ,UMR0,C1,L2,C3,IM,JM,NT,NO,PEXT1,IT,KC,IFIRST
25     1,JP,IND,NE-1,XAC,NE-S,INCHT,CO,HAIN,LI,LIJ,KI,KI
      COMMON/ALX10/ NGAGE,NFLG,IGAGE(12),JGAGE(12),PFLD=(E),XMIN,XMAX
      C
      ARSF(X)ARSF(X)                                PT2=C026
      SQRTF(X)SQRTF(X)                              PT2=C027
30     C
      NUMRO=IMRC
      KZ=KI
      LZ=LIJ
      GRAV=32.1456355
      C1=FAOFLT
      C2=(GRAV*DELT)/(2.*DELT)
      C3=DELT/DELT
      IM=IM-1
      JM=JM-1
      AT=0
      NN=0
      NEXTI=1
      IT=2
      KC=1
      IFIRST=1
      J=I*J*NFU+1
      OO 130 I=1,IM
      VY(I)=0.
      UU 130 J=1,JM
      M(1+J)=I*(1+J)
      Z=IZ(I,J)
      IF(Z,LT,MGI) M(I+J)=MGI
      U(1+J)=0.
      PT2=C028
      PT2=C029
      PT2=C030
      PT2=C031
      PT2=C032
      PT2=C033
      PT2=C034
      PT2=C035
      PT2=C036
      PT2=C037
      PT2=C038
      PT2=C039
      PT2=C040
      PT2=C041
      PT2=C042
      PT2=C043
      PT2=C044
      PT2=C045
      PT2=C046
      PT2=C047
      PT2=C048
      PT2=C049
      PT2=C050

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55      130 V(I,J)=0.
        MTIME=1
        PT2=C051
        PT2=0052
      C
      C READ CHANNEL DATA AND ESTABLISH KEY ARRAYS
      IF(KCM.GT.0) CALL CHANL(1)
        PT2=0053
      C READ GAGE LOCATIONS FOR SAVING KEY M AND Q VALUES AS TIME SEQUENCE
      CALL SAVE(1)
        PT2=0054
      C HEAD LIST DATA AND PRINT PROBLEM IDENTIFICATION AND Z FIELD
      HEAD 15, TDEXT,IE'D,NF,I6F6I',NJ,NCARD
        PT2=0055
      15 FORMAT(I1,I4,4I5)
        PT2=0056
      140 FORMAT( /1M,15A2,15A2,10A2)
        PT2=0057
      220 FORMAT(1M1)
        PT2=0058
      230 FORMAT(15A2,15A2,10A2)
        PT2=0059
      PRINT 220
        PT2=0060
      DO 250 J=1,NCARD
      READ 230, (ALPHA(I),I=1,40)
        PT2=0061
      70      250 PRINT 140, (ALPHA(I),I=1,40)
        PT2=0062
        PRINT 230
        PT2=0063
      C
      C
      148 CONTINUE
        PT2=0065
      75      RF=(RF/12.)=CONST
        PT2=0066
        NE=1=NDW
        PT2=0067
        XN0=XN0C
        PT2=0068
        NE=3B=QRT
        PT2=0069
        XNORT=XNORT
        PT2=0070
      80      ISTR=ISTR+NFU
        PT2=C071
        I'D=I'D+NFU
        PT2=C072
        AJ=LZ
        PT2=0073
        AJ=LZ
        PT2=0074
      85      JH=JH+1
        PT2=0075
        LJK=LZ=1
        PT2=0076
        KIK=KZ=1
        PT2=0077
      C
      C ENTRY PART 29
        PT2=0078
      90      NU = (NUM*TIME)/INTER
        PT2=0079
      C PLOT CHANNELS AND BARRIERS
      CALL SAVE(2)
        PT2=0082
      C IF(KCM.GT.0) CALL CHANL(4)
        PT2=0083
      C START OF TIME INCREMENTING LOOP
      200 CONTINUE
        PT2=0084
      IF(NOWIND.LT.0) GO TO 430
        PT2=C085
      300 IF (NE=1=ND=) 330,310,310
        PT2=0086
      310 C=(CMST(NEXT1+1)-CMST(NEXT1))/XN0
        PT2=0087
        BIND=CMST(NEXT1)
        PT2=0088
      100      NE=1=1
        PT2=0089
        DO 320 NE+2=1,1=RO
        PT2=C090
      320 C=(NEX2)=(PO(NF+2,NEXT1+1)-N0(NC=2,NEXT1))/XN0
        PT2=0091
        NEXT1=NEXT1+1
        PT2=0092
        GO TO 340
        PT2=0093
      105      330 NE=1=1=1
        PT2=0094
        AN=1=1=1
        PT2=0095

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	CHST4=BT*V*(A1=1.)*C-	PT2=0096
	YC=CHST0	PT2=0097
	DO 350 K=1,IMRO	PT2=0098
110	NX=NEXT1	PT2=0099
	350 WRO(KA)=RO(KA, NX=1 )+(A1=1.)*CRO(KA)	PT2=0100
	IMDUM=NTIME/AFU	PT2=0101
	360 IF(NTIME=ISTR)430,350,370	PT2=0102
	370 IF(NTIME=ICN)380,410,430	PT2=0103
115	380 IF(NE=3=ICN)400,370,390	PT2=0104
	390 NE=4*(DIST(KC+1)-DIST(KC))/X*ORT	PT2=0105
	KC=KC+1	PT2=0106
	NE=381	PT2=0107
	GO TO 420	PT2=0108
120	400 NE=381E=3+1	PT2=0109
	GO TO 420	PT2=0110
	410 NE=400	PT2=0111
	KC=KC+1	PT2=0112
125	420 CONTINUE	PT2=0113
	A1=3E=3	PT2=0114
	A1=3E=3	PT2=0115
	R=(R*(DIST(KC+1)+(A1=3=1.)+A1=4 ))/X*ORT	PT2=0116
	RAI=80	PT2=0117
	GO TO 440	PT2=0118
130	430 RAI=80, A	PT2=0119
	440 CONTINUE	PT2=0120
	C	
	C	
	END OF PAIR AND RO VALUES	
	C	
	C	
135	START OF JND COMPUTATIONS	
	500 IF (J=IND=AFU)ROV=800,510	PT2=0121
	510 CONTINUE	PT2=0122
	560 IF (IFIRST)800,570,570	PT2=0123
	570 DO 580 J=1,IM	PT2=0124
140	MG1(I)=MG2(I)	PT2=0125
	DO 580 J=1,IM	PT2=0126
	X1(I,J)=X2(I,J)	PT2=0127
	580 Y1(I,J)=Y2(I,J)	PT2=0128
	DO 590 I=1,2, A	PT2=0129
	590 MBI(IR1)=M2(IR1)	PT2=0130
145	600 M1=EM-TI=1	PT2=0131
	IT=IT+1	PT2=0132
	DO 610 I=1,LMAX	PT2=0133
	MGR(I)=MGM(I,IT)	PT2=0134
	IF(IND=LT, A) GO TO 610	PT2=0135
150	DO 620 J=1,LMAX	PT2=0136
	X(I,J)=X(I,IT)	PT2=0137
	620 Y(I,J)=Y(I,IT)	PT2=0138
	610 CONTINUE	PT2=0139
	DO 630 J=2, A	PT2=0140
155	630 MRR(J)=MRR(J,IT)	PT2=0141
	640 J=IND+1	PT2=0142
	IS=1	PT2=0143
	DO 710 L=1,LMAX	PT2=0144
	JC=1+(L7*(L=1))	PT2=0145

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100      DC 680 K=1,KMM
          I1=1+(K2*(K-1))
          I2=I1+K*I1
          DXR=(XR(K+1,L)-XR(K,L))/AI
          DYH=(YR(K+1,L)-YR(K,L))/AJ
165      GO TO (450,650),IS
650      DMH=(MGR(K+1)-MGR(K))/AI
660      CN 580 IC=I1,I2
          DFU=IC-1
          Y2(IC,JC)=YR(K,L)+(DYR*(DFU+.5))
          X2(IC,JC)=XR(K,L)+(DXR*DFU)
170      GO TO (670,690),IS
670      MG2(IC)=MG(K)+DMR*(DFU+.5)
680      CONTINUE
          DC 690 IRT=2,A
          MB2(IRT)=MR(IRT)
175      GO TO (700,710),IS
700      IS=2
710      CONTINUE
          DO 740 I=1,IM
          DO 730 L=1,LMM
180          J1=1+(L2*(L-1))
          J2=J1+L*J1
          J1K=J1+K2
          J1L=J1+L2
          DXR=(X2(I,J1K)-X2(I,J1))/AI
          DYH=(Y2(I,J1L)-Y2(I,J1))/AJ
185      DO 720 J=J1,J2
          DFU=J-J1
          X2(I,J)=X2(I,J1)+DXR*(DFU+.5)
          Y2(I,J)=Y2(I,J1)+DYH*DFU
190      720 CONTINUE
          730 CONTINUE
          740 CONTINUE
          IF (IFIRST)750,800,800
195      750 IFIRST=1
          GO TO 570
          800 CONTINUE
          810 ANUP=PII
          -IND=J-1*ND
200      DFU=(I-1)*D=1./ANUP
          DFUM=DFU*(1./ANUP)
          DO 820 K=2,A
          MB(K)=M1(K)+DFU*(MB2(K)-M1(K))
          MG(I)=MG1(I)+DFUM*(MG2(I)-MG1(I))
205      C
          C SEEK WHOLE FIELD FOR FLOW FROM BLOCKS
          C
          GO 2010 J=1,JMM
210      C THIS BRANCH SKIPS THE INVESTIGATION OF POSSIBLE BARRIERS FOR THE
          C ROW J=1, FOR J=1 THE INDICATOR LG=3 IS SET, IF J IS GREATER THAN
          C 1 A SEARCH FOR BARRIERS IN THE ROW WILL TAKE PLACE.
          840 KJ = 0
          PT2=0146
          PT2=C147
          PT2=C148
          PT2=0149
          PT2=0150
          PT2=0151
          PT2=0152
          PT2=0153
          PT2=0154
          PT2=0155
          PT2=0156
          PT2=0157
          PT2=0158
          PT2=0159
          PT2=0160
          PT2=0161
          PT2=C162
          PT2=0163
          PT2=C164
          PT2=C165
          PT2=C166
          PT2=C167
          PT2=0168
          PT2=0169
          PT2=0170
          PT2=0171
          PT2=0172
          PT2=0173
          PT2=C174
          PT2=0175
          PT2=0176
          PT2=C177
          PT2=C178
          PT2=C179
          PT2=C180
          PT2=0181
          PT2=C182
          PT2=C183
          PT2=C184
          PT2=C185
          PT2=0186
          PT2=0187
          PT2=0188
          PT2=0189
          PT2=C190
          PT2=0191
          PT2=C192
    
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215 C      L=0
C      A NORMAL COMPUTATION SEQUENCE WILL OCCUR. THE FIRST X=DIR FLUX
C      TEMPORARY STORAGE IS SET AS THAT OF THE FIRST COLUMN.
C      THE NUMBER AND LOCATIONS OF THE BARRIERS PRESENT IN THE ROW ARE
C      FOUND AND PLACED IN TEMPORARY STORAGE. IF NO BARRIERS ARE PRESENT
C      THE INDICATOR KJ REMAINS ZERO.
C      IF (K=EQ,0) GO TO 870
220 C      LC 860 K=1,KM
C      IF (J=JR(K))860,850,860
250 850 KJ = KJ + 1
C      L=1
C      K5(L)=K
225 C
C      640 CONTINUE
C      BASED ON KJ, THE INDEX LJ IS SET TO INDICATE THE BARRIER SITUATION
C      IN THE NEXT COMPUTATION, LJ=1 FOR NO BARRIERS,
230 C      IF(KJ)870,870,860
C      870 LJ=1
C      GO TO 890
C      860 LJ=2
235 C
C      THIS IS THE PRIMARY LOOP FOR STEPPING THRU THE IM GRID COLUMNS,
C      890 DO 2000 I=1,IMM
C      BEGIN THE EXAMINATION OF THE BASIC TRIAD OF GRID SQUARES. THE
C      DUMMY VARIABLES M1,D1,M2,D2 AND Q ARE USED TO ALLOW ONE ROUTINE TO
C      BE EMPLOYED FOR BOTH SETS OF SQUARES, SQUARES ONE AND TWO ARE
C      TAKEN FIRST.
240 C      910 IF (J=1)910,910,920
C      910 M(I)=MG(I);I)=DFUN(MG2(I)=MG1(I))
C      920 X(I)=S(X1(I,J)+DFUN(X2(I,J)=Y1(I,J)))
C      Y(I)=S(Y1(I,J)+DFUN(Y2(I,J)=Y1(I,J)))
C      M1 = M(I,J)
245 C      Z = IZ(I,J)
C      D1 = M1-2
C      THIS BRANCH WILL SET UP A SEARCH FOR A BARRIER IN THE SQUARES
C      BEING CONSIDERED IF LJ=2, IF LJ=1 OR THE BARRIER EXISTS BETWEEN
C      THE OTHER PAIR OF SQUARES AN INDEX IS SET, LI=1, FOR A BARRIER,
250 C      LI=2.
C      1000 GO TO (1040,1010)+LJ
C      THIS X LOOP SEARCHES FOR A BARRIER IN THE PAIR OF SQUARES.
C      1010 DO 1030 K=1,KJ
C      K1=K9(K)
255 C      IF(I=IR(K))1030,1020,1030
C      1020 LI=2
C      GO TO 1050
C
C      1030 CONTINUE
260 C      1040 LI=1
C      THE DUMMY VARIABLES M2 AND D2 ARE SET FOR THE SQUARE ONE AND TWO
C      CALCULATION, THIS IS INDICATED BY L=1.
C      1050 CONTINUE
265 C      1053 M2 = M(I+1,J)
C      1054 Z = IZ(I+1,J)

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      D2 = M2 - 2
      LQ = 1
C
C THE INVESTIGATION OF THE RELATION BETWEEN DATUMS OF BOTH PAIRS OF
C SQUARES BEGINS HERE. THIS BRANCH TESTS LI FOR A BARRIER.
270 GO TO (1110,1070),LI
C A BARRIER EXISTS AND ON THE BASIS OF LQ THE DATUM IS ASSIGNED THE
C PROPER BARRIER HEIGHT.
      1070 GO TO (1020,1090),LQ
275 1080 ZB = I7X(KI)
      COGI = ICOSX(KI)
      COSI = ICOSX(KI)
      GO TO 1100
280 1090 ZB = I2Y(KI)
      COGI = ICOSY(KI)
      COSI = ICOSY(KI)
285 1100 ZB = ZB * 0.1
      COGI = COGI * .001
      COSI = COSI * .001
      GO TO 1140
C NO BARRIER EXISTS. THE RELATIVE DATUM HEIGHTS OF THE SQUARES ARE
C TESTED AND THE HIGHER DATUM SET EQUAL TO Zb.
290 1110 COUI = COI
      IF (M1 - D1 - M2 + D2) 1120, 1130, 1130
      1120 ZB = (M2 - M1)
      GO TO 1140
      1130 ZB = M1 - D1
C THE INVESTIGATION OF THE DEPTH SIGNATURES BEGINS AT THIS POINT.
C THE PROPER ASSIGNMENT IS MADE FOR THE FLUX CALCULATION.
295 1140 IF (D1) 1150, 1160, 1190
      1150 LM = 1
      GO TO 1170
C
300 1160 LM = 2
      1170 IF (D2) 1180, 1360, 1180
      1180 IF (M2 - ZB) 1190, 1360, 1260
      1190 IF (C2) 1200, 1210, 1230
      1200 LM = 1
      GO TO 1220
305 1210 LM = 2
      1220 IF (M1 - ZB) 1230, 1360, 1270
      1230 IF (M1 - ZB) 1150, 1160, 1240
      1240 IF (M2 - ZB) 1250, 1250, 1240
310 1250 LM = 2
      GO TO 1270
      1260 DM = ZB - M2
      DP = ABS(DM)
      TAD = 4. * C2
315 1270 DM = M1 - ZB
      DP = ABS(DM)

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      TAD = 4.0 / D1
      GO TO (1300,1350),L4
320 1280 GO TO (1460,1330),L1
      1290 M(I,J)=M1=01
      GO TO 1350
      1300 GO TO (1310,1320),L4
325 1310 M(I+1,J) = M2 = 02
      GO TO 1350
      1320 M(I,J+1) = M2 = 02
      GO TO 1350
      1330 IF (Z4=(M1=01)) 1460,1460,1340
330 1340 IF (Z5=(M2=02)) 1460,1460,1370
      1350 IF (CP.LT.C0000001) GO TO 1360
      DRE = (C001=004)*(C001=C04)
      GO TO 1360
      1360 G=0.
335 1370 GC TO 1570
      D=M=M1=M2
      TAD = D1+D2
      D4 = ((M1+M2)/2.) - Z6) * COSI
      DRE = D4=04
340 1380 GO TO (1390,1400),L4
      1390 G = U(I+1,J)
      PUSH = V(I) * DELT
      GO TO 1450
      1400 Q = V(I,J+1)
345 1440 PUSH = V(I) * DELT
      C
      C SPECIAL CALCULATION OF Q FOR BARRIERS
      1450 GDS=GRAV*RF
      RG=GUS/(C2*TAN)
350 FORCE=RG*(D+PUSH)+GDS*CM
      HRG=4G/2.
      G = SQRT(ABS(FORCE)+HRG**2) = HRG
      IF (FORCE.LT.0.) G = -G
      GO TO 1570
355 1460 GO TO (1470,1490),L4
      1470 G = U(I+1,J)
      1480 B1 = V(I,J) + V(I+1,J) + V(I+J+1) + V(I+1,J+1)
      PUSH = V(I) * DELT
360 1490 U = V(I,J+1)
      GO TO 1510
      1500 B1 = U(I,J) + U(I+1,J) + U(I+J+1) + U(I+1,J+1)
      1505 PUSH=V(I)*DELTA
      1510 A1 = G * G
365 1520 K = SQRT( ((A1 * A1) + (B1 * B1) ))
      1530 G = 1. + ((C1 * K) / ((D1+D2)*(D1+D2)))
      TAD = D1+D2
      D4 = M1=M2
      1540 IF (PUSH)1541,1542,1542
370 1541 IF (D2=0.0)1540,1540,1545
      1545 IF (D2=0.1)1540,1544,1540

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PT2=0265
PT2=0266
PT2=0267
PT2=0268
PT2=C269
PT2=0270
PT2=0271
PT2=0272
PT2=C273
PT2=0274
PT2=0275
PT2=0276
PT2=0277
PT2=C278
PT2=0279
PT2=C280
PT2=C281
PT2=0282
PT2=0283
PT2=C284
PT2=0285
PT2=0286
PT2=0287
PT2=0288
PT2=0289
PT2=0290
PT2=0291
PT2=0292
PT2=0293
PT2=0294
PT2=0295
PT2=0296
PT2=C297
PT2=0298
PT2=C299
PT2=0300
PT2=0301
PT2=0302
PT2=C303
PT2=0304
PT2=C305
PT2=C306
PT2=0307
PT2=0308
PT2=0309
PT2=0310
PT2=0311
PT2=0312
PT2=0313
PT2=0314

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1542 IF(O1=0.0)1560,1560,1543          PT2=0315
1543 IF(O1=0.1)1544,1544,1560          PT2=0316
1544 PUSH=0.0                            PT2=0317
375   GO(G=1.0)*.07+1.0                 PT2=0318
C
C   STANDARD CALCULATION OF G FOR BLOCKS
1560 G=(1.0/G)*(2+(C2*TAG*U)+PUSH)      PT2=0319
C
380   C   THE M AND D CALCULATIONS ARE MADE ON THE BASIS OF THE INDEX LG. IF
C   LG#1 THE CALCULATIONS ARE POSTPONED AND A RETURN TO THE POINT OF
C   INVESTIGATION OF THE DATUM RELATIONSHIPS IS MADE (STATEMENT 21)
C   AFTER THE DUMMY VARIABLES M2 AND D2 ARE SET UP FOR THE ONE-TREE
C   SQUARES.
385   1570 GO1=C1/C3                      PT2=0320
C   GO2=D2/C3                            PT2=0321
C   IF(ABS(O).LT.1.0E+10) GO=0.0        PT2=0322
C   GO TO (1571,1681)+LG                 PT2=0323
C
390   1571 IF(O)1572,1577,1573          PT2=0324
C   M(L)=1                                PT2=0325
C   GO TO 1580                            PT2=0326
1572 M(L)=0                              PT2=0327
395   IF(M2=ZP)1576,1575,1575          PT2=0328
C   GO=0.0                                PT2=0329
C   GO TO 1580                            PT2=0330
1575 IF(D2=O)1574,1574,1580           PT2=0331
1574 GO=0.0                              PT2=0332
C   GO TO 1580                            PT2=0333
400   1573 M(L)=1                        PT2=0334
C   IF(M1=ZP)1576,1580,1580             PT2=0335
1580 IF(I=1)1590,1590,1630             PT2=0336
1590 IF(J=JBL)1610,1610,1620          PT2=0337
C   LEFT HAND SEABEDS BOUNDARY CONDITION
405   1610 M(I,J)=G(I)                  PT2=0338
C   UNDO.                                  PT2=0339
C   GO TO 1670                            PT2=0340
C
1630 IF(J=JBL)1640,1640,1670          PT2=0341
410   1640 IF(I=IMM)1670,1650,1670     PT2=0342
C   RIGHT HAND SEABEDS BOUNDARY CONDITION
1650 M(IMM,J)=G(IMM)                  PT2=0343
C   GO TO 1680                            PT2=0344
1670 UNDO.                              PT2=0345
415   1680 M2=M(I,J+1)                  PT2=0346
C   Z=IZ(I,J+1)                          PT2=0347
C   D2=M2-Z                               PT2=0348
C   LG=L2                                  PT2=0349
420   2892 GO TO 1680                    PT2=0350
C
1681 IF(O)1671,1674,1673              PT2=0351
1674 IF(M(L))1690,1688,1689           PT2=0352
1671 IF(M2=ZP)1672,1682,1682         PT2=0353
1672 GO=0.0                            PT2=0354

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425      1682 IF(ML=)1684,1684,1685
         1685 IF(GD1=UN1)1686,1686,1684
         1686 UN1=GD1
         1687 IF(GD2=0)1687,1687,1690
         1687 Q=GD2
430      GO TO 1694
         1673 IF(M1=ZP)1672,1693,1683
         1683 IF(ML=)1683,1683,1689
         1688 IF(QD1=0)1692,1692,1690
         1692 Q=GD1
435      GO TO 1694
         1689 IF(Q+UM1=001)1690,1690,1691
         1691 ADDQ = Q+UM1 + 0.00001
         Q = (Q/(ADDQ))*QD1
         UN1 = (UM1/(ADDQ))*QD1
440      VN1 = Q
         U(I,J)=UM
         UN = UN1
         V(I,J) = VN(I)
         VN(I) = VN1
445      2000 CONTINUE
         U(I+J)=UN1
         2010 CONTINUE
         IF(KCM,GT,0) CALL CHANL(2)
         C
         C
450      S=SLEEP WHOLE FIELD FOR M ON BLOCKS
         C
         SUMMO.
         COUNT=0.
         DO 2020 J=1,JMM
455      DO 1790 I=1,IMM
         Z=IZ(I,J)
         DIMM(I,J)=Z
         IF(J=1)1700,1700,1710
         1700 M(I,1) = MG(I)
         1710 IF(D1)1740,1720,1720
         1720 IF(J=1)1700,1700,1721
         1721 IF(I=1)1722,1722,1723
         1722 IF(J=JML)1790,1790,1724
         1723 IF(I=IMM)1720,1724,1724
465      1724 IF(J=J90)1790,1790,1729
         1729 SETUP=C3*(U(I,J)-U(I+1,J))+V(I,J)-V(I,J+1))
         M(I,J)= M(I,J) + SETUP + RAIN
         SU=SU+ARSF(M(I,J))
         COUNT=COUNT+1.
470      IF ( D1 + SETUP + RAIN) 1740,1740,1750
         1740 M(I,J) = IZ(I,J)
         C
         1750 IF(KCM,GT,0) GO TO 1790
         C
475      C
         C
         ENTER BUNOFF VALUES ON ENTRY BLOCKS ONLY IF CHANNELS NOT PROVIDED
         DO 1770 J=1,JMM
         IF (LROJ(I,J)=J)1770,1760,1770

```

PT2=0355

PT2=0356

PT2=0357

PT2=0358

PT2=0359

PT2=0360

PT2=0361

PT2=0362

PT2=0363

PT2=0364

PT2=0365

PT2=0366

PT2=0367

PT2=0368

PT2=0369

PT2=0370

PT2=0371

PT2=0372

PT2=0373

PT2=0374

PT2=0375

PT2=0376

PT2=0377

PT2=0378

PT2=0379

PT2=0380

PT2=0381

PT2=0382

PT2=0383

PT2=0384

PT2=0385

PT2=0386

PT2=0387

PT2=0388

PT2=0389

PT2=0390

PT2=0391

PT2=0392

PT2=0393

PT2=0394

PT2=0395

PT2=0396

PT2=0397

PT2=0398

PT2=0399

PT2=0400

PT2=0401



```

1760 IF(LH0I(IJK)=1)1770,1780,1770
1770 CONTINUE
480 GO TO 1790
1780 M(I,J)=M(I,J)+MMO(IJK)*DELX/(DELX**2)
1790 CONTINUE
2020 CONTINUE
IF(NCM.GT.0) CALL CHANL(3)
485 C
C THE TIME INDICES ARE STEPPED TO THE NEW LEVEL.
NT = NT + 1
NTIME = NTIME + 1
J=INDOJ-INDO+1
490 IF(SUM/COUNT=100.) 2175,2140,2140
C TEST THE STABILITY OF THE COMPUTATIONS VIA AVE ABS(M).
C COMPUTATIONS ARE STABLE. CALCULATIONS CONTINUE.
C COMPUTATIONS ARE UNSTABLE. AN ON-LINE MESSAGE IS PRINTED
C
495 C TEST NT FOR THE OUTPUT OF U,V,W,D,X,Y FIELDS.
2075 TIM=NTIME*INTIME
ITIM=TIM
MINT=MINT+1
IF((ITIM/MINT)-ITIM/I-TEP) 2055,2055,2090
500 2055 CALL SAVE(2)
2090 IF(NT=IOUT) 2110,2175,2100
C OUTPUT U,V,W,D,X,Y FIELDS. RESET NT=0 AND STEP NN.
2105 IF(INFLD.FG.0) GO TO 2110
905 CALL CHANL(4)
GO TO 2110
2100 NT = 0
NN = NN + 1
HOUR = NTIME/NF
CALL CHANL(5)
910 2110 CONTINUE
C
2130 IF (NN= NTIME) 2160,2160,220
C STOP= COMPLETED. FINAL OUTPUT ON TAPES.
2140 PRINT 2150,NN
915 2150 FORMAT (21H STOP AS AT NTIME = ,I4)
STOP
C
2160 CALL SAVE(3)
CALL CONTIN(2)
920 RETURN
END

```

PT2=0402

PT2=0403

PT2=0404

PT2=0405

PT2=0406

PT2=0407

PT2=0408

PT2=0409

PT2=0410

PT2=0411

PT2=0412

PT2=0413

PT2=0414

PT2=0415

PT2=0416

PT2=0417

PT2=0418

PT2=0419

PT2=0420

PT2=0421

PT2=0422

PT2=0423

PT2=0424

PT2=0427

PT2=0428

PT2=0429

PT2=0430

PT2=0431

PT2=0432

PT2=0433

PT2=0434

PT2=0440

PT2=0441

```

1      C
2      C
3      SUBROUTINE CHANL(N)                                CHNL0001
4
5      COMMON/ALX1/ TR(100),JB(100),IZX(100),IZY(100),ICDX(100)    CHNL0004
6      1,ICDXY(100),ICDSX(100),ICDSY(100),LROI(A),LROJ(A),DIST(24)    CHNL0005
7      2,CHST(30),RO(A,30),MGH(8),XR(8,6),YR(8,6),MR(8)             CHNL0006
8      COMMON/ALX2/ IZ(28,20),U(28,20),V(28,20),M(28,20),NTIME    CHNL0007
9      COMMON/ALX3/ NM,MI,MMAX,NFU,INFLD,IM,JM,KM,KMAX,LMAX,DELX,DELT    CHNL0008
10     1,CD0,FX,MGI,IOU,KI,LIJ,KII,LJJ,JDL,IR,KYM,LM,RF,CONST,S    CHNL0009
11     2,IRG,JHRO,WR,ISTR,IND,NON,KIM,NORT,NTIME,INTYE,AO,IND,ORAV    CHNL0010
12     3,KCXP,DFU,INTER                                             CHNL0011
13     COMMON/ALX4/ HON(A),CRO(8),XB(24),X1(28,21),Y1(28,21),X2(28,21)    CHNL0012
14     1,YZ(28,21),X(28),Y(28),MG1(28),MG2(28),MB1(8),MH(9),VN(28)    CHNL0013
15     2,MG(28),MR(8)                                             CHNL0014
16     COMMON/ALX5/ ICG(130),JCG(130),I-CX(130),I-CY(130),IZCX(130)    CHNL0015
17     1,IZCY(130),QCXP(130),CCXN(130),QCYP(130),OCYN(130),MC(130),MP(130)    CHNL0016
18     2,KCM,KCY(130),KCY(130),KCB(130),UCT(130),UCF(130),MRT(130),IRON    CHNL0017
19     3,KEN(2,130),UCT(130),VCF(130),ACGX(130),AOGV(130),KXP(130)    CHNL0018
20     4,KCYP(130),KLB(40),KLM,IFC(130),FC                       CHNL0019
21     COMMON/ALX7/ IEND,NF,IBL, *J,?(40)                          CHNL0020
22     COMMON/ALX9/ KZ,LZ,NU,HO,C1,C2,C3,IMM,JMM,VT,VT,FTY,IT,KC,IFIRST    CHNL0021
23     1,J,IND,NE1,XNO,NE,3,XNORT, C4,WA1N,AJ,AI,LIJ,KIK          CHNL0022
24     EQUIVALENCE (DA,DAC)                                       CHNL0023
25
26     ABSF(X) = ABS(X)                                           CHNL0024
27     SQRTF(X) = SQRT(X)                                         CHNL0025
28
29     GO TO (1000,2000,3000,4000) ,N                             CHNL0026
30
31     C
32     C CHANNEL CODE 1 IS FOR READING CHANNEL DATA AND ESTABLISHING KEY ARRAYS
33     C CHANNEL CODE 2 IS FOR FLOW AND HEIGHT CALCULATIONS IN CHANNELS
34     C CHANNEL CODE 3 IS FOR CALCULATION OF H ON BLOCKS CONTAINING CHANNELS
35     C CHANNEL CODE 4 IS FOR LISTING OF CHANNEL OUTPUT
36
37     C
38     C ENTRY POINT 1 FOR READING CHANNEL DATA, INITIALIZATION AND FOR
39     C ESTABLISHING KEY ARRAYS FOR ROUTINE CALCULATIONS
40
41     C
42     1000 PRINT 500                                             CHNL0027
43     PRINT 500                                                 CHNL0028
44     500 FORMAT(' THE FOLLOWING ARE SUBGRID CHANNEL DATA- Z VALUES IN FEET')    CHNL0029
45     1ET(/)                                                    CHNL0030
46     CUR(DELX*2)/FELT                                         CHNL0031
47
48     C
49     C A NEGATIVE I CX OR I CY IDENTIFIES THOSE CHANNELS WITH BARRIERS OF
50     C EQUAL ELEVATION ON BOTH SIDES SUCH AS A JETTY SYSTEM
51     C FOR SINGLE BARRIERS, THE LATTER IS TAKEN ON THE INNER SIDE OF THE
52     C CHANNEL BLOCK IF IZC IS NEGATIVE, WHILE ON THE OUTER SIDE IF IZC IS
53     C POSITIVE
54
55     C
56     LD SL *21,NCM
57     READ 501, IEND, ICG(K),JCG(K),I-CX(K),IZCX(K),I-CY(K),IZCY(K)    CHNL0032
58     1,FC(K)                                                    CHNL0033
59     IF(IFC(K),EQ,0) IFC(K)= FC*10000                          CHNL0034
60     IF(IFC(K),EQ,0) IFC(K)= FC*10000                          CHNL0035

```

```

55      501 FORMAT(11,2X,15,9(3X,15))
        IF(IDEV.NE.8) GO TO 510
        90 CONTINUE
          DO 100 K=1,KM
            KEN(1,K)=N
            KEN(2,K)=N
            KR1(K)=N
            KCX(K) = 0
            KCY(K) = 0
            KXP(K) = 0
            KCP(K) = 0
            I = ICG(K)
            J = JCG(K)
            DO 80 L=1,KCM
              IF(ICG(L).EQ.(I+1).AND.JCG(L).EQ.J) KCP(K) = L
              IF(ICG(L).EQ.I.AND.JCG(L).EQ.(J+1)) KXP(K) = L
              IF(ICG(L).EQ.(I-1).AND.JCG(L).EQ.J) KCY(K) = L
              IF(ICG(L).EQ.I.AND.JCG(L).EQ.(J-1)) KCX(K) = L
            80 CONTINUE
              KCD(K) = 0
              IF(KM.EQ.0) GO TO 91
            75      DO 90 L=1,KM
                  IF(IB(L).EQ.I.AND.JB(L).EQ.J) KCB(K) = L
            90 CONTINUE
            91 CONTINUE
              UCT(K) = 0.0
              UCF(K) = 0.0
              VCT(K) = 0.0
              VCF(K) = 0.0
              MP(K) = M(I,J)
            85      502 FORMAT('  K=(.I3)( ICG=(.I3)( JCG=(.I3)( I-CX=(.I5)( IZCX=(.I4
                  1, I I-CY=(.I5)( IZCY=(.I4)( IFC=(.I4)
            100 CONTINUE
C
C      ARRAY KLB IDENTIFIES BARRIER BLOCKS WHICH ARE NOT COMMON
C      WITH CHANNEL BLOCKS
            90      LC = 0
                  DO 105 K=1,KM
                    I=IB(K)
                    J=JB(K)
            95      DO 102 L=1,KCM
                    IF(ICG(L).EQ.I.AND.JCG(L).EQ.J) GO TO 105
            102 CONTINUE
                    LC=LC+1
                    KLB(LC)=K
            100      105 CONTINUE
                    KLM=LC
C
C      THE FOLLOWING CREATES A SPECIAL INDEX FOR CHANNEL STARTING AND END
C      ANY BLOCK WITH NEGATIVE ICG OR JCG IDENTIFIES A CHANNEL END POINT
C      ARRAY KEN IDENTIFIES WHAT TYPE OF END POINT EXISTS ACCORDING TO THE
            105      1 KCM M          5 KCA 2
C

```

	C	2	KCY	M	6	KCY	Q	
	C	3	KCXP	M	7	KCXP	Q	
	C	4	KCYP	M	8	KCYP	Q	
110	C							
								CHNL0081
								CHNL0082
								CHNL0083
								CHNL0084
115	C							
								CHNL0085
								CHNL0086
								CHNL0087
120								CHNL0088
								CHNL0089
								CHNL0090
								CHNL0091
								CHNL0092
125								CHNL0093
								CHNL0094
	C							
110								CHNL0095
								CHNL0096
130								CHNL0097
								CHNL0098
								CHNL0099
								CHNL0100
								CHNL0101
135								CHNL0102
								CHNL0103
								CHNL0104
								CHNL0105
								CHNL0106
140								CHNL0107
								CHNL0108
								CHNL0109
	C							
120								CHNL0110
								CHNL0111
145								CHNL0112
								CHNL0113
								CHNL0114
121								CHNL0115
								CHNL0116
150								CHNL0117
								CHNL0118
125								CHNL0119
								CHNL0120
								CHNL0121
155								CHNL0122
								CHNL0123
								CHNL0124
								CHNL0125
								CHNL0126

```

160      C
      130 IF(I-CX(K),NE,0) GO TO 200
          IF(KX,EQ,0) GO TO 131
          IF(I-CX(KX),NE,0) GO TO 200
165      131 IF(KY,EQ,0) GO TO 135
          IF(I-CY(KY),NE,0) GO TO 200
          IABS=IG+1
          KCYP(K)=KCM+IAC
      135 IF(ICG(K),LT,0) JCG(K)=J
          ICG(K)=I
170      L=1
          IF(JCG(K),LT,0) L=2
          KEN(L,K)=0
          IF(I,GE,I**AND,0,J,GT,J**R) GO TO 200
          KEN(L,K)=0
175      IF(I,GE,I**R) GO TO 200
          Z=IZ(I+1,J)
          IF((K(I+1,J)=Z),LE,0) KEN(L,K)=0
      200 CONTINUE
      C
180      C      ISUM IS THE TOTAL NUMBER OF CHANNEL END POINTS OF ANY KIND
          ISUM = IS0
          KCM=KCM+IROM+1
          DO 210 K=1,KCM
185      KC(K)=MGI
          GCXP(K) = 0.
          QCYP(K) = 0.
          GCYN(K) = 0.
          QCYN(K) = 0.
          ACGX(K) = 0.
190      ACGY(K) = 0.
      210 CONTINUE
      C
      C
195      C      ARRAY KRI IDENTIFIES THE LOCATIONS OF RIVER INPUT FOR Q TYPE END POINTS
          LR=0
          LQ=0
          DO 300 K=1,KCM
200      I=ICG(K)
          J=IABS(I)
          J=JABS(J)
          ITOP=KCYP
          IF(KCX(K),EQ,0) KCX(K)=ITOP
          IF(KCY(K),EQ,0) KCY(K)=ITOP
205      IF(KCXP(K),EQ,0) KCXP(K)=ITOP
          IF(KCYP(K),EQ,0) KCYP(K)=ITOP
      C
210      KRI(K) = 0
          IF(I=RO,EQ,0) GO TO 460
          LS=1
          IF(ICG(K),GT,0) GO TO 460
          410 KJ=K*(LS+1)

```

CHNL0127  
 CHNL0128  
 CHNL0129  
 CHNL0130  
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 CHNL0168  
 CHNL0169  
 CHNL0170  
 CHNL0171  
 CHNL0172

```

      IF(KJ.LE.4) GO TO 460
      LQMLCM=1
215      LR=0
      DO 450 L=1,IMRN
      IF(LRGT(L).EQ.1.AND.LRGJ(L).EQ.J) LQML
450      CONTINUE
      KRI(K)=LR
220      IF(LR.GT.1) LQML=+1
460      IF(JCG(K).GT.0) GO TO 300
      IF(LS.EQ.2) GO TO 300
      LR=2
      GO TO 410
225      300 CONTINUE
      IF(LRM.EQ.IMRN) GO TO 480
C
      PRINT 470, LR4,IM40
230      470 FORMAT(1(//)('*****CHANNEL INPUTS***** ONLY(=13,
      1( CHANNEL END POINTS(//) MATCH THE(=13( RIVER INPUT POSITIONS(
      2(//)('*****))
C
      480 CONTINUE
235      PRINT 549
      549 FORMAT(1( )
      DO 600 K=1,KCM
      PRINT 550, K,KCX(K),KCY(K),KCP(K),KCB(K),ICG(K),JCG(K)
      1,KEN(1,K),KEN(2,K),KRI(K)
240      550 FORMAT(1( K=(=13( KCX=(=13( KCY=(=13( KCP=(=13( KCB=(=13(
      1( KEN1=(=13( ICG=(=13( JCG=(=13( KEN2=(=13(
      2( KRI=(=13(
      600 CONTINUE
      PRINT 551, KCM
245      551 FORMAT(1( (// 10X,(KCM=1, 15,//)
C
      RETURN
250      510 PRINT 503
      503 FORMAT(1( STOP BECAUSE CARDS WITH IDENT = 8 EXPECTED(//)
      PRINT 504, IDENT
      504 FORMAT(1X,(IDENT=(=14)
      STOP
C
C ENTRY POINT 2 FOR FLOW AND HEIGHT CALCULATIONS IN CHANNELS
C
255      2000 SDT = SDELT
      DO 2500 K=1,KCM
      I= ICG(K)
      J= IAPS(I)
      J= JCG(K)
      J= IAPS(J)
      PUSHU = SDT*(X1(I,J)+DFU*(X2(I,J)-X1(I,J)))
      PUSHV = SDT*(V1(I,J)+DFU*(V2(I,J)-V1(I,J)))
      H1 = H(I,J)
      Z1 = IZ(I,J)
      HCI = HC(K)
260
265

```

		DI = M1+21	CHNL0220
		CF = IFC(K)	CHNL0221
		CF = CF*DELTA/1000.	CHNL0222
270		KK = KCA(K)	CHNL0223
		ACS = I-CK(K)	CHNL0224
		ACS = ABS(ACS)	CHNL0225
		IF(ACS.EQ.0.) GO TO 2250	CHNL0226
		LS = 1	CHNL0227
	C		
275		ZZ = IZ/(I+1.J)	CHNL0228
		M2 = M(I+1.J)	CHNL0229
		O2 = M2-ZZ	CHNL0230
		QK = QCX(K)	CHNL0231
280		QP = QCXP(K)	CHNL0232
		GT = ICT(K)	CHNL0233
		GF = UCF(K)	CHNL0234
		POT = PUSMU	CHNL0235
		PUC = PUSMV*-C	CHNL0236
		KAKCK(K)	CHNL0237
285		ZCS = IZCK(K)	CHNL0238
		ZCS = ABS(ZCS)	CHNL0239
		IF(KK.EQ.0.) GO TO 2310	CHNL0240
		Z9C = IZX(KK)	CHNL0241
		Z9C = Z9C/10.	CHNL0242
290		COOI = ICDX(KK)	CHNL0243
		COCI = COOI/1000.	CHNL0244
		COSI = ICDX(KK)	CHNL0245
		COSI = COSI/1000.	CHNL0246
		GO TO 2020	CHNL0247
295	C		
		C****CUTER HF-ENTRY POINT (X AND Y CHANNELS)	
		2010 COOI = CCO	CHNL0248
		COSI = CCO	CHNL0249
	C		
300		2020 M = MC(KA)	CHNL0250
		MAC = (MCI+MN)/2.0	CHNL0251
		DAC = MAC-ZC	CHNL0252
		IF(DAC.GT. 0.0) GO TO 20205	CHNL0253
	C		
305		PRINT 20206, DAC, K	CHNL0254
		20206 FORMAT(1, DAC=(E7.2,1 AT CHANNEL BLOCK(,I4,////))	CHNL0255
		GO TO 4000	CHNL0256
	C		
310		20205 CEL = SQRT(GRAV*DAC)	CHNL0257
		ALP = C3*CF	CHNL0258
		CALP = 1.0 - ALP	CHNL0259
		MA = ALP*QV + CALP*CI	CHNL0260
		ME = CALP*QV + ALP*CI	CHNL0261
		GA = ALP*QV + CALP*QV	CHNL0262
315		GA = CALP*QV + ALP*QV	CHNL0263
		LPI = 1	CHNL0264
		LPI = 1	CHNL0265
	C		

320	OI = O1	CHNL0266
	OII = OAC	CHNL0267
	M1 = M1	CHNL0268
	MII = MAC	CHNL0269
	UI = OT	CHNL0270
	MI = DELX * -C	CHNL0271
325	MII = -C	CHNL0272
	IF(M1.GT.0) GO TO 2022	CHNL0273
	Z9=Z1	CHNL0274
	GO TO 2021	CHNL0275
330	2022 ZH=ZOC	CHNL0276
	IF(-CS.LT.0.) GO TO 2021	CHNL0277
	IF(ZCS.GT.0.) Z9=Z1	CHNL0278
335	2021 LQ=1	CHNL0279
	IF(Z1.GT.76) Z9=Z1	CHNL0280
	Z81=Z8	CHNL0281
	C	
	C*****INNER REFENTRY POINT (SIDES 1 AND 2 OF CHANNEL)	
	2025 IF(MII=78)2030,2030,2040	CHNL0282
	2030 IF (M1=Z9) 2060,2060,2070	CHNL0283
340	2040 IF(M1=Z8) 2075,2075,2080	CHNL0284
	2060 GOUT = 0.	CHNL0285
	GO TO 2100	CHNL0286
	C OVERFLOW FROM REGION I TO REGION II	
	2070 DM=M1-Z8	CHNL0287
	GO TO 2090	CHNL0288
345	C OVERFLOW FROM REGION II TO REGION I	
	2075 LM=Z=-MII	CHNL0289
	GO TO 2090	CHNL0290
	C SURFACED BARRIERS	
	2080 GO TO (2081,2082), LF	CHNL0291
350	2081 GOUT= (M1-MII*(M1-MII)/((M1+MII)*DELX)	CHNL0292
	LF= 2	CHNL0293
	GO TO (2110,2120), LU	CHNL0294
	2082 GO TO (2083,2084), LS	CHNL0295
	2083 GOUT= U(I+1,J)	CHNL0296
355	GO TO 2085	CHNL0297
	2084 GOUT= V(I,J+1)	CHNL0298
	2085 UT = GOUT * ((M1=2.*MAC+M2)*-C-(M1-MAC)*(-C+2)/DELX)/(2.*DELX)	CHNL0299
	MOQ = (QT-GOUT)/2.	
	QT = GOUT+MOQ	
360	GOUT= GOUT-MOQ	
	GO TO 2120	CHNL0302
	C	
	2090 GOUT = CDN1*DM*SHRT(GHAY*ABS(DM))	CHNL0303
	LO=LG	CHNL0304
365	2100 GO TO (2110,2120),LQ	CHNL0305
	C	
	2110 OI = OAC	CHNL0306
	OII = O2	CHNL0307
	M1 = MAC	CHNL0308
370	MII = M2	CHNL0309
	UI = UP	CHNL0310



```

      NI = AC
      N10 = DELX
      GT = QOUT
      375 IF (K4.GT.0) GO TO 2112
      Z0=Z2
      GO TO 2111
      2112 Z0=ZAC
      380 IF (MC5.LT.0.) GO TO 2111
      IF (ZC5.LT.0.) Z4=Z2
      2111 L0=Z
      IF (Z2.GT.Z0) Z0=Z2
      Z02=Z0
      GO TO 2025
      385 C*****END OF INNER RE-ENTRY
      C
      2120 OF=OOUT
      C
      C THE FOLLOWING TESTS CONSTRAIN THE CHANNEL OVERFLOW (GT AND/OR GF) SUCH
      390 C THAT (GF=GT) CANNOT PRODUCE AN IMPOSSIBLE CHANGE IN MC IN ONE TIME STEP
      C (IE MC SHOULD NOT FALL BELOW SILL DEPTH NOR RISE ABOVE THE HIGHER
      C OF THE ADJOINING BLOCK W DUE TO OVERFLOW ABOVE).
      C
      IF (LG.EQ.0) GO TO 2190
      IF (LF.EQ.0) GO TO 2100
      IF ((GF=GT).LE.0.0) GO TO 2140
      C NET OUTFLOW BARRIERS OVERTOPPING
      ZMIN=Z41
      IF (Z0P.LT.ZMIN) ZMIN=Z02
      400 GNET = (MAC-ZMIN)*AC/DELX
      IF ((GF=GT).LE.GNET) GO TO 2190
      IF ((GF=GT).GT.0.0) GO TO 2130
      QFS=JF002
      QTS=JUT002
      405 BUM=JNET/(QFS+QTS)
      QF=QF+QFS
      QTS=QTS+QTS
      GO TO 2190
      2130 IF (GF.LT.0.0) GO TO 2155
      410 2134 QF=QF+QF*GT
      GO TO 2100
      2135 WT = -(QNET-QF)
      GO TO 2190
      C NET INFLOW BARRIERS OVERTOPPING
      415 2140 MMAX = M1
      IF (-2.GT.MMAX) MMAX = M2
      JNET = (MMAX-MAP)*AC/DELX
      IF ((GT=GF).LE.QNET) GO TO 2190
      IF ((GF=GT).GT.0.0) GO TO 2151
      420 QFS = QF002
      QTS = QTS002
      BUM = JNET/(QFS+QTS)
      QF = QF+QFS
      QTS = QTS+QTS

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      CHNL0311
      CHNL0312
      CHNL0313
      CHNL0314
      CHNL0315
      CHNL0316
      CHNL0317
      CHNL0318
      CHNL0319
      CHNL0320
      CHNL0321
      CHNL0322
      CHNL0323
      CHNL0324
      CHNL0325
      CHNL0326
      CHNL0327
      CHNL0328
      CHNL0329
      CHNL0330
      CHNL0331
      CHNL0332
      CHNL0333
      CHNL0334
      CHNL0335
      CHNL0336
      CHNL0337
      CHNL0338
      CHNL0339
      CHNL0340
      CHNL0341
      CHNL0342
      CHNL0343
      CHNL0344
      CHNL0345
      CHNL0346
      CHNL0347
      CHNL0348
      CHNL0349
      CHNL0350
      CHNL0351
      CHNL0352
      CHNL0353

```

425	GO TO 219A	
	2150 IF(ZF.GT. 0.0) GO TO 2155	CHNL0354
	2154 GT = QNET + ZF	CHNL0355
	GO TO 219A	CHNL0356
	2155 GF = ZT = QNET	CHNL0357
430	GO TO 219A	CHNL0358
	2160 GO TO (217A+218A)+L0	CHNL0359
	2170 IF(QT.GT.A.) GO TO 2175	CHNL0360
	C BARRIER 1 (VERTICALLY OUTFWARDS- OTHER SIDE SUBMERGED)	CHNL0361
	QNET = (MAC-ZR1)+C/DELTA	
435	IF((GF-QT).LE.QNET) GO TO 2190	CHNL0362
	Q0 = GF-QNET	CHNL0363
	IF(Q0.GT.A.) Q0 = 0.	CHNL0364
	GO TO 2170	CHNL0365
	C BARRIER 1 OVERTOPPING INWARDS- OTHER SIDE SUBMERGED	CHNL0366
440	2175 QNET = (M1+MAC)+C/DELTA	CHNL0367
	IF((QT-ZF).LE.QNET) GO TO 2190	CHNL0368
	Q0 = QFT+GF	CHNL0369
	IF(Q0.LT.A.) Q0 = 0.	CHNL0370
	2179 QT = Q0	CHNL0371
445	GO TO 219A	CHNL0372
	2180 IF(QF.LT.A.) GO TO 2185	CHNL0373
	C BARRIER 2 (VERTICALLY OUTFWARDS- OTHER SIDE SUBMERGED)	
	QNET = (MAC-ZR2)+C/DELTA	CHNL0374
450	IF((GF-QT).LE.QNET) GO TO 2190	CHNL0375
	Q0 = QFT+GT	CHNL0376
	IF(Q0.LT.A.) Q0 = 0.	CHNL0377
	GO TO 2190	CHNL0378
	C BARRIER 2 OVERTOPPING INWARDS- OTHER SIDE SUBMERGED	
455	2185 QNET = (M2+MAC)+C/DELTA	CHNL0379
	IF((JT-QF).LE.QNET) GO TO 2190	CHNL0380
	Q0 = QT-QNET	CHNL0381
	IF(Q0.GT.A.) Q0 = 0.	CHNL0382
	2189 QF = Q0	CHNL0383
	C END OF ADJUSTMENT OF QT AND/OR GF	
460	2190 CONTINUE	CHNL0384
	C	
	C CHANNEL COMPUTATIONS	
	AA = C/CFL	CHNL0385
465	GA = 1.0+CF*SGDT*((G1**2+Q1**2)/2.)/(C*DACC**2)	CHNL0386
	AOG = AA/GAM	CHNL0387
	BQG = CE1*(DFLT*(GT-QF) + C*RA1A)	CHNL0388
	BP = (GA+AA*AA*PI*(C*ROD))/GAM	CHNL0389
	B = (GA+AA*AA*PI*(C*ROD))/GAM	CHNL0390
	GO TO (220A+230A)+LS	CHNL0391
470	C	
	2200 UCT(M) = QT	CHNL0392
	UCF(M) = ZF	CHNL0393
	U(I+1,J) = ZF	CHNL0394
475	QCXP(M) = RP	CHNL0395
	QCXN(M) = RN	CHNL0396
	ACGX(M) = ACG	CHNL0397
	C	

	2250	CS=IACY(K)	CHNL0398
		CCXAS(-CS)	CHNL0399
480		IF(-C.E0.0.) GO TO 2500	CHNL0400
		LS = 2	CHNL0401
		Z2 = IZ(I,J+1)	CHNL0402
		M2 = M(I,J+1)	CHNL0403
		D2 = M2-Z2	CHNL0404
485		GT = GCYK(K)	CHNL0405
		GP = GCYP(K)	CHNL0406
		GT = VCT(K)	CHNL0407
		GF = VCF(K)	CHNL0408
		PUT = PUSMV	CHNL0409
490		PUC = PUSMV=C	CHNL0410
		K4 = KCY(K)	CHNL0411
		ZCS=IZCY(K)	CHNL0412
		ZC=ABS(ZCS)	CHNL0413
		IF(KK.E0.0) GO TO 2010	CHNL0414
495		ZMC = IZV(KK)	CHNL0415
		ZAC = ZMC/10.	CHNL0416
		COOI = ICDOY(KK)	CHNL0417
		COUI = CDOT/1000.	CHNL0418
		COSI = ICDOY(KK)	CHNL0419
500		COST = CDST/1000.	CHNL0420
		GO TO 2024	CHNL0421
		C*****END OF OUTER REF-ENTRY	
		C	
505	2300	VCT(K) = GT	CHNL0422
		VCF(K) = GF	CHNL0423
		V(I,J+1) = GF	CHNL0424
		GCYP(K) = GP	CHNL0425
		GCYK(K) = GA	CHNL0426
		AGGY(K) = AOG	CHNL0427
510	2500	CONTINUE	CHNL0428
		C	
		DO 2700 K=1,KC4	CHNL0429
		I = ICG(K)	CHNL0430
515		I = IARS(I)	CHNL0431
		J = JCG(K)	CHNL0432
		J = IARS(J)	CHNL0433
		KX = I-CX(K)	CHNL0434
		KX = ABSF(-X)	CHNL0435
520		KY = I-CY(K)	CHNL0436
		KY = ABSF(-Y)	CHNL0437
		KX = KCYP(K)	CHNL0438
		KY = KCYP(K)	CHNL0439
		KA = GCYP(K)	CHNL0440
		KB = GCYP(K)	CHNL0441
525		AGA = AOGY(K)	CHNL0442
		AGB = AOGY(K)	CHNL0443
		ACB2CY(K)	CHNL0444
		AGCBAGX(K)	CHNL0445
		BCB2CY(K)	CHNL0446
530		AGCBAGY(K)	CHNL0447

```

C
  MC = (RA+RB+RC+RD)/(AGA+AGR+AGC+AGD)
  QA=RA-AGC*MC
  GB=RB-AGR*MC
  GCXN(KX) = RC+AGC*MC
  QCYN(KY) = RD+AGD*MC
  MC(K) = MC
  IF(JCG(K).LT.0) GO TO 2600
  GO TO 2695
935
C
  BOUNDARY CONDITIONS FOR Q END POINTS
  2600 L=1
  2605 KEYSNEX(L,K)
  GO TO(2690,2690,2630,2640,2650,26A0,2670,26C0), KEYSNEX(L,K)
  2630 QA=QCXP(K)
  GO TO 2A90
  2640 QB=QCYP(K)
  GO TO 269A
945
C
  THE FOLLOWING ASSUMES Q=0 AT END IF NO DISCHARGE DATA EXISTS
  2650 BA=QCXP(K)
  KS=KCX(K)
  KT=KI(K)
  QCXN(K) = 0
  IF(KT.GT.0) QCXN(K) = HRO(KT)
  MC(KS) = (QCXN(K)-BA)/AOGX(K)
  GO TO 269A
  2660 SA=QCYN(K)
  KS=KCX(K)
  KT=KI(K)
  QCYN(K) = 0
  IF(KT.GT.0) QCYN(K) = HRO(KT)
  MC(KS) = (QCYN(K)-SA)/AOGY(K)
  GO TO 2A9A
  2670 BA=QCXP(K)
  KT=KI(K)
  QA = 0
  IF(KT.GT.0) QA = HRO(KT)
  MC(K) = (BA-QA)/AOGX(K)
  GO TO 269A
  2680 BA=QCYP(K)
  KT=KI(K)
  QB = 0
  IF(KT.GT.0) QB = HRO(KT)
  MC(K) = (BA-QB)/AOGY(K)
955
C
  2690 IF(JCG(K).GT.0) GO TO 2695
  IF(L.EQ.2) GO TO 2645
  L=2
  GO TO 2A9A
960
C
  2695 J(XP(K))=0
  SCYP(K)=0
965

```

CHNL0448  
 CHNL0449  
 CHNL0450  
 CHNL0451  
 CHNL0452  
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 CHNL0490  
 CHNL0491  
 CHNL0492  
 CHNL0493

```

2700 CONTINUE
985 C
      RETURN
C
C ENTRY POINT 3 FOR HEIGHT CALCULATIONS ON BLOCKS WITH CHANNELS
C
990 3000 DO 3050 M=1,KC
      I=ICG(M)
      IA=IARS(I)
      JA=JCG(M)
      JB=IARS(J)
995 IF(I.EQ.I*09.J.EQ.JM) GO TO 3050
      Z=IZ(I,J)
      M(I,J)=MG(I)
      IF(J.EQ.1) GO TO 3050
      U=UUCT(M)
      V=VUCT(M)
      MX=I-CX(M)
      MY=ARSF(MX)
      NY=I-CY(M)
      NY=ARSF(NY)
      IF(M.EQ.N) U=U(I+1,J)
      IF(NY.EQ.N) V=V(I+1,J)
      SETUP=DELTA*((U(T+J)-U)/(DELX-MX)+(V(I,J)-V)/(DELY-MY))
      M(I,J)=M0(K)+SETUP*RAIN
      IF(M(I,J).LE.7) M(I,J)=Z
      MP(K)=M(I,J)
600 3050 DO 3500 M=1,KC
      I=ICG(M)
      IA=IARS(I)
      JA=JCG(M)
      JB=IARS(J)
      Z=IZ(I,J)
      IF(ICG(M).LT.N) GO TO 3100
605 C
      BOUNDARY CONDITIONS FOR M END POINTS
      IN THESE CALCULATIONS MC EQUALS THE H OF THE ADJOINING WATER BLOCK
      MC AND Q ARE SOLVED FROM SIMULTANEOUS EQUATIONS WHICH ALLOW FOR THE
      VOLUME TRANSPORT TO OR FROM THE ADJOINING BLOCK VIA CHANNEL FLG=0
      L=1
625 TCF=2.0C4
      KEY=KFN(L,K)
      GO TO (3110,3120,3130,3140,3300,3300,3300,3300),KEY
      MS=MCX(K)
      BA=BCXN(K)
630 IF(J.EQ.1) GO TO 3115
      MA=M(I+1,J)-QCKN(MS)/TCF
      DIV=1.0+ANGY(K)/TCF
      GCXN(K)=(RAM+ANGX(K)+MA*M)/DIV
      MC(MS)=(MA+RAM)/TCF/DIV
      GO TO 3114
635 3115 MC(MS)=MG(I)
      CHNL0494
      CHNL0495
      CHNL0497
      CHNL0498
      CHNL0499
      CHNL0500
      CHNL0501
      CHNL0502
      CHNL0503
      CHNL0504
      CHNL0505
      CHNL0506
      CHNL0507
      CHNL0508
      CHNL0509
      CHNL0510
      CHNL0511
      CHNL0512
      CHNL0513
      CHNL0514
      CHNL0515
      CHNL0517
      CHNL0518
      CHNL0519
      CHNL0520
      CHNL0521
      CHNL0522
      CHNL0523
      CHNL0524
      CHNL0525
      CHNL0526
      CHNL0527
      CHNL0528
      CHNL0529
      CHNL0530
      CHNL0531

```

		QCXN(K)=RA*+ADGY(K)*MC(KS)	CHNL0532
		GO TO 3300	CHNL0533
640	3110	M(I+J+1)=C(KS)	CHNL0534
		QCXN(KS)=QCXN(K)	CHNL0535
		GO TO 3300	CHNL0536
	3120	AS=ACY(K)	CHNL0537
		BA=BCY(K)	CHNL0538
645		IF(I.EQ.1) GO TO 3125	CHNL0539
		MA=MIN(I+J)=QCYN(KS)/TCP	CHNL0540
		DIV=1.0+ADGY(K)/TCP	CHNL0541
		QCYN(K)=BA+ADGY(K)*MA/DIV	CHNL0542
		MC(KS)=(MA+BA)/TCP/DIV	CHNL0543
		GO TO 3124	CHNL0544
650	3125	MC(KS)=G(1)	CHNL0545
		QCYN(K)=BA+ADGY(K)*MC(KS)	CHNL0546
		GO TO 3300	CHNL0547
	3126	M(I+J+1)=C(KS)	CHNL0548
655		QCYN(KS)=QCYN(K)	CHNL0549
		GO TO 3300	CHNL0550
	3130	KS=KX(K)	CHNL0551
		BA=BCX(K)	CHNL0552
		VAR=J.5/CU	CHNL0553
660		IF(KS.GT.WCM) GO TO 3132	CHNL0554
		AC=ICX(KS)	CHNL0555
		AC=ARSP(KC)	CHNL0556
		VAR=C300.9/(DELX=AC)	CHNL0557
665	3132	MA=MIN(I+J)+KX(KS)*VAR	CHNL0558
		DIV=1.0+VAR*ADGY(K)	CHNL0559
		QCXN(K)=BA+ADGY(K)*MA/DIV	CHNL0560
		MC(K)=BA+VAR*MA/DIV	CHNL0561
		M(I+J+1)=C(K)	CHNL0562
		MC(KS)=C(K)	CHNL0563
670		QCXN(KS)=QCXN(K)	CHNL0564
		GO TO 3300	CHNL0565
	3140	AS=ACY(K)	CHNL0566
		BA=BCY(K)	CHNL0567
675		IF(I.EQ.1) GO TO 3145	CHNL0568
		VAR=J.5/CU	CHNL0569
		IF(KS.GT.WCM) GO TO 3142	CHNL0570
		AC=ICX(K)	CHNL0571
		AC=ARSP(KC)	CHNL0572
		VAR=C300.9/(DELX=AC)	CHNL0573
680	3142	MA=MIN(I+J)+KX(KS)*VAR	CHNL0574
		DIV=1.0+VAR*ADGY(K)	CHNL0575
		QCYP(K)=BA+ADGY(K)*MA/DIV	CHNL0576
		MC(K)=BA+VAR*MA/DIV	CHNL0577
		GO TO 3146	CHNL0578
685	3145	MC(K)=G(T=U)	CHNL0579
		QCYP(K)=BA+ADGY(K)*MC(K)	CHNL0580
		GO TO 3300	CHNL0581
	3146	M(I+J+1)=C(K)	CHNL0582
		MC(KS)=C(K)	CHNL0583
		QCYP(KS)=QCYP(K)	CHNL0584

690	C		
		3300 IF(JCG(M),GT,1) GO TO 3500	CHANL0585
		IF(L.FQ.2) GO TO 3500	CHANL0586
		L=2	CHANL0587
		GO TO 3105	CHANL0588
695	C		
		3500 CONTINUE	CHANL0589
	C	RETURN	CHANL0591
700	C	ENTRY POINT 4 FOR LIST OF CHANNEL OUTPUT	
	C		
		4000 I=HOUR*NTIME//F	CHANL0591
		4010 FORMAT(1M1)	CHANL0592
		PRINT 4020, I, HOUR, NTIME	CHANL0593
705		4020 FORMAT(10X, 'CHANNEL OUTPUT FOR HOUR=(I3+40X, NTIME=(I5, //	CHANL0594
		1 20X, 'ALL M VALUES IN FEET, ALL Q VALUES IN CFS(, //)	CHANL0595
		PRINT 4030	CHANL0596
		4030 FORMAT(7X, (M(.7X, (I(.7X, (J(.0X, (M(.5X, (JXY(.5X, (QXP(.6X, (MY(	CHANL0597
		1.9X, (QYN(.5X, (QYP(.6X, (MC(.5X, (QXT(.5X, (QXF(.5X, (QYT(.5X, (QYF(.//)	CHANL0599
710	C		
		I=1	CHANL0599
		IS=ICG(I)	CHANL0600
		IS=IABS(IS)	CHANL0601
		JS=JCG(I)	CHANL0602
715		JS=IABS(JS)	CHANL0603
		DO 4100 #=1,KCM	CHANL0604
		KX=KCX(K)	CHANL0605
		KY=KCY(K)	CHANL0606
		QXT=UCT(K)*DELX	CHANL0607
720		QXF=UCF(K)*DELX	CHANL0608
		QYT=UCT(K)*DELX	CHANL0609
		QYF=UCF(K)*DELX	CHANL0610
		IT=ICG(K)	CHANL0611
		IT=IABS(IT)	CHANL0612
725		JT=JCG(K)	CHANL0613
		JT=IABS(JT)	CHANL0614
		IF((IT-IS)**2.EQ.1.AND.JT.EQ.JS) GO TO 4200	CHANL0615
		IF((JT-JS)**2.EQ.1.AND.IT.EQ.IS) GO TO 4200	CHANL0616
		PRINT 4050, IR	CHANL0617
730		IR=IR+1	CHANL0618
		4200 IS=IT	CHANL0619
		JS=JT	CHANL0620
		PRINT 4040, K, ICG(K), JCG(K), MC(KX), QCX(K), QXP(K), MC(KY),	CHANL0621
		1 QCY(K), QYP(K), MC(K), QXT, QXF, QYT, QYF	CHANL0622
735		4040 FORMAT(1A, F8.3, 2F8.0, F8.3, 2F8.0, F8.3, 4F8.0)	CHANL0623
		4050 FORMAT(1, // 4X, 'CHANNEL REACH=(I3, //)	CHANL0624
		4100 CONTINUE	CHANL0625
	C		
	C	VOLUME COMPUTATION	
740	C		
		9000 VOL=0,	CHANL0626
		CB=DELX**2	CHANL0627

	JL=JAL	CHNL0628
	IF(JC9.GT.JAL) JL=J+2	CHNL0629
745	JL=JL+1	CHNL0630
	DO 5500 I=1,IW	CHNL0631
	DO 5400 J=JL,JW	CHNL0632
	Z=IZ(I,J)	CHNL0633
	MJ=MI(I,J)	CHNL0634
750	IF(Z.GT.0) -IJA=IJ-Z	CHNL0635
	IF(KCM.EQ.0) GO TO 5200	CHNL0636
	DO 5100 K=1,KC=	CHNL0637
	IC=ICG(K)	CHNL0638
	JC=JCG(K)	CHNL0639
755	IF(IA9S(IC).EQ.I.AND.IABS(JC).EQ.J) GO TO 5300	CHNL0640
	5100 CONTINUE	CHNL0641
	5200 VOL=VOL+MTJ*CS	CHNL0642
	GO TO 5400	CHNL0643
	5300 M=MC(K)	CHNL0644
760	M=ABS(M)	CHNL0645
	M=I-CY(M)	CHNL0646
	M=ABS(M)	CHNL0647
	M=MC(K)	CHNL0648
	M=I-CY(M)	CHNL0649
765	VOL=VOL+MTJ*(DELX=-X)*(DELX=-Y)+((MC(K)+MC(KX))*X-	CHNL0650
	1*(MC(K)+MC(KY))*Y)*DELX/2.-MC(K)*X+MY	CHNL0651
	5400 CONTINUE	CHNL0652
	5500 CONTINUE	CHNL0653
	VOL=VOL/1000000.	CHNL0654
770	JL=JL-1	CHNL0655
	PRINT 5600, VOL, JL	CHNL0656
	5600 FORMAT(1, // 1X, (VOLUME OF WATER ABOVE MSL = (, F12.1,	CHNL0657
	1 ( MILLIONS OF CU FT (, // 10X, ((THE STANDARD RG-S THRU J= (, I3,	CHNL0658
	2 ( ARE EXCLUDED) (, //)	CHNL0659
775	PRINT 4910	CHNL0660
	C	
	IF(N. EQ. 4) RETURN	CHNL0661
	C	
	PRINT 5700	CHNL0662
780	5700 FORMAT(1, // ( PROBLEM TERMINATED BECAUSE A CHANNEL WAS GONE DRY (	CHNL0663
	1, // ( (	CHNL0664
	STOP	CHNL0665
	C	
	END	CHNL0666



```

1      C
      C
      SUBROUTINE SAVE(JIN)                                SAVE0001

5      COMMON/PLK2/ IZ(28,20),U(28,20),V(28,20),W(28,20),NTIME    SAVE0004
      COMMON/PLK3/ NM,MMIN,MMAX,MFU,INFLD,IM,J,K,KMAX,LMAX,DELX,DELT  SAVE0005
1     CDD,K,MC,I,IOUT,KI,LJ,KII,LJJ,JBL,JBR,MM1,MM2,RF,CONST,S    SAVE0006
2     IMRG,JMRD,KR,ISTR,IND,NOM,KIM,NORT,TIME,IZTIME,NONIND,GRAV   SAVE0007
3     KCM,DFU,INTER                                              SAVE0008
10     COMMON/PLK5/ ICR(130),JCG(130),I-CX(130),I-CY(130),IZCX(130)  SAVE0009
      1,IZCY(130),OCXP(130),OCXN(130),OCYP(130),OCYN(130),MC(130),MP(130) SAVE0010
      2,KCM,KCX(130),KCY(130),KCB(130),UCT(130),UCF(130),KRI(130),IBCM SAVE0011
      3,KEN(2,130),VCT(130),VCF(130),AOGX(130),AOGY(130),KEXP(130)  SAVE0012
      4,KCYP(130),KLB(50),KLM,IFC(130),FC                          SAVE0013
15     COMMON/PLK8/ MS(9,72),QS(6,72),TIME(72)                    SAVE0014
      COMMON/PLK9/ K2,L2,NUMHO,C1,C2,C3,IMM,JMM,NY,KK,NEXT1,IT,MC,IFIRST SAVE0015
      1,JIND,NEW1,XNDM,NEW3,XNORT, CUP,PAI,ALJ,ALJ,KIK              SAVE0016
      COMMON/PLK10/ NGAGE,NFLOW,IGAGE(12),JGAGE(12),KFLOW(6),XMIN,XMAX SAVE0017

20     C
      C
      C
      C
      C
      THIS ROUTINE SAVES WATER LEVELS AND FLOW RATES AT CERTAIN
      KEY POINTS AS SPECIFIED IN INPUT BY USER. THE TIME SEQUENCES OF
      THESE QUANTITIES ARE OUTPUTED BY THE THIRD PART OF THIS ROUTINE.

25     GO TO(100,2000,3000),JIN                                SAVE0018
1000  READ 135, (IGAGE(K),JGAGE(K),K=1,NGAGE)                   SAVE0019
135   FORMAT(20I4)                                             SAVE0020
      PRINT 136                                                SAVE0021
136   FORMAT(1 F,3X,(HYDROGRAPH GAGE LOCATIONS))              SAVE0022
      DO 100 K=1,NGAGE                                          SAVE0023
30     I= IGAGE(K)                                             SAVE0024
      J= JGAGE(K)                                             SAVE0025
      IF(J.NE.0) GO TO 105                                     SAVE0026
      PRINT 130, K,I                                           SAVE0027
35     130  FORMAT(5X,IGAGE(I,13,I) CHANNEL NO. K=I,I)        SAVE0028
      GO TO 100                                                SAVE0029
105   PRINT 137, K,I,J                                         SAVE0030
137   FORMAT(5X,IGAGE(I,13,I) BLOCK NO. I=I,13,I J=I,13)    SAVE0031
100   CONTINUE                                                SAVE0032
      PRINT 138                                                SAVE0033
40     138  FORMAT(1 F,3X,(KEY FLOW LOCATIONS))                SAVE0034
      C
      READ 135, (KFLOW(K), K=1,NFLOW)                          SAVE0035
      PRINT 139, (KFLOW(K), K=1,NFLOW)                          SAVE0036
45     139  FORMAT(5X,(CHANNEL BLOCKS(10UI4)))                SAVE0037
      RETURN                                                    SAVE0038

      C
2000  TT= NTIME-NTIME                                          SAVE0039
      NINT= INTER                                              SAVE0040
      N=I/NINT + 1                                             SAVE0041
50     IT= NTIME                                              SAVE0042
      TIME(N)= TT*DFLT/3600                                    SAVE0043
      DO 200 K=1,NGAGF                                         SAVE0044
      I=IGAGE(K)                                               SAVE0045

```

```

55      J=JGAGE(K)
      IF (I.GT.KCM) GO TO 199
      MS(K,N)=MC(I)
199    IF (J.NE.0) MS(K,N)=M(I,J)
200    CONTINUE
      C
60      DO 300 J=1,NFLC-
      K=KFLC(J)
      KEYS=KEN(I,K)
      IF (KEY.NE.0) GO TO 205
      KEYS=2
65      IF (I.CX(K).NE.0) KEYS=1
205    GO TO(210,220,230,240,210,220,230,240), KEYS
210    QS(J,N)=QCXN(K)/1000.
      GO TO 300
70      220    QS(J,N)=QCVN(K)/1000.
      GO TO 300
      230    QS(J,N)=QCXP(K)/1000.
      GO TO 300
      240    QS(J,N)=QCVP(K)/1000.
300    CONTINUE
75      NU=72
      IF (N.FQ.72) GO TO 310
      RETURN
      C
80      3000    NU=(N*NTIME)/INTER
      IF (NU.EQ.0) RETURN
310    PRINT 410
400    FORMAT(20X,'WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS) (SAVE0072)
      1 //)
      PRINT 410, (J,J=1,NGAGE), (K,K=1,NFLC-)
85      410    FORMAT(2X,'MOUR(=I0.15IA)
      DO 500 N=1,NU
      PRINT 420, TIME(N), (MS(J,N),J=1,NGAGE), (QS(K,N),K=1,NFLC-)
420    FORMAT(F6.1,15FA.2)
90      425    FORMAT(F6.1,12FA.2)
500    CONTINUE
      DO 510 N=1,NU
95      430    FORMAT(F6.1,10FA.2)
510    CONTINUE
      PRINT 10
      10    FORMAT(1M1)
      IF (NU.EQ.72) INTIME=NTIME
      RETURN
100     C
      END
      C
1      C
      C
      SUBROUTINE CONTI(L)
      C
5      COMMON/BLK1/4(822)/BLK2/5(1901)/BLK3/6(42)/BLK4/0(2585)/
      19LK5/6(2759)/BLK6/6(2800)/BLK7/6(44)/BLK9/6(25)/BLK10/6(34)
      C
      GO TO (100,200),L
      C
10     C
100    CONTINUE
170    FORMAT(21A6)
      RETURN
      C
15     C
200    CONTINUE
      RETURN
      C
      END
  
```

```

1      C
      C
      C
5      SUBROUTINE PLOT                                PLOT0005
      C
      C      PROGRAM TO PLOT CHANNELS AND BARRIERS
      C
10     COMMON/BLK1/ TA(100),JB(100),IZX(100),IZY(100),ICDX(100)    PLOT0010
      1,ICDY(100),ICDSX(100),ICDSY(100),LQJ(A),LQJ(B),NIST(24)    PLOT0015
      2,CMST(30),MC(A,30),MG(B),XP(E,e),YP(B,b),MRR(B)          PLOT0020
      COMMON/BLK2/ TZ(29,20),UC(29,20),V(29,20),M(29,20),KTIME    PLOT0025
      COMMON/BLK5/ ICG(130),JCG(130),ICX(130),I-CY(130),IZCX(130)    PLOT0030
15     1,IZCY(130),KCYX(130),KCY(130),KCY(130),KCY(130),KCY(130),KCY(130)    PLOT0035
      2,KCX(KCX(130),KCY(130),KCE(130),KCT(130),UCF(130),KPI(130),T50"    PLOT0040
      3,KEN(2,130),VCT(130),VCF(130),KGY(130),ADUY(130),KCR(130)    PLOT0045
      4,KCVP(130),VLR(50),KLM"                                     PLOT0050
      COMMON/BLK10/ 5,GAGE,VFLO,KIGAGE(12),JGAGE(12),KFLG-(e),XMIN,XMAX    MAIN0110
20     DIMENSION NUMBER(10),PAGE(114,135)
      LOGICAL XBAR,VAARR,X,ELNK,NUMBER,DE,PAGE,VLINE,MLINE,PLUS,PERIOD
      1,BLANK
      DATA BLANK/1/,X/1/,Y/1/,Z/1/,NUMBER/10/,11/,12/,13/,14/,15/,
25     16/,17/,18/,19/,ONE/11/,VLINE/11/,MLINE/10/,PLUS/10/,PERIOD/
      2/,1/
      C
      C
      C      DO 100 J=1,15390
      C      PAGE(I,1)=BLANK
100    CONTINUE
      C
      C
30     DO 101 J=1,114,4                                PLOT0105
      PAGE(I,3) = PERIOD                                PLOT0110
      PAGE(I,134) = PERIOD                              PLOT0115
35     DO 101 J=9,135,7                                PLOT0120
101    PAGE(I,J) = PERIOD                              PLOT0125
      C
      C
      C      DRAW THE BARRIERS
      C
40     DO 800 M=1,KC
      K = KC-M+1
      I = IABS(ICG(K))+J-1
      J = IABS(JCG(K))+7-4
45     I4 = I+3
      J4 = J+6
      IF ( KCR(M) .EQ. 0 ) GO TO 810
      C
      C
      C      TEST FOR BARRIER IN X DIRECTION, Y DIRECTION, OR BOTH
50     KR = KCR(K)
      II = IR(KR)
      JJ = JR(M)
      IZ1 = I7(II,JJ)+10

```

55	IZZ = IZ(I-1,JJ)*10 IZ6 = IZ(I*4) XBARR = .TRUE. IF ( IZ4 .EQ. IZ1 .OR. IZ2 .EQ. IZ2 ) XBARR = .FALSE. IZZ = IZ(I,JJ+1)*10 IZ6 = IZ(I*4) YBARR = .TRUE. IF ( IZ4 .EQ. IZ1 .OR. IZ4 .EQ. IZ2 ) YBARR = .FALSE.	PLOT0185 PLOT0190 PLOT0195 PLOT0200 PLOT0205 PLOT0210 PLOT0215 PLOT0220
	C IF ( .NOT. XBARR ) GO TO 250	PLOT0225
65	C X BARRIERS C IF ( I-CX(I) .LT. 0 ) GO TO 230 IF ( IZX(I) .LE. 0 ) GO TO 231	PLOT0230 PLOT0235
70	C OUTER BARRIER DO 202 L=1,IC 202 PAGE(I+L,J+L-3) = X GO TO 250	PLOT0240 PLOT0245 PLOT0250
75	C INNER BARRIER 231 DO 203 L=1,IO 203 PAGE(I+L-2,J+L-3) = X GO TO 250	PLOT0255 PLOT0260 PLOT0265
80	C BOTH BARRIERS 230 DO 204 L=1,IC PAGE(I+L,J+L-3) = X 204 PAGE(I+2,J+L-3) = X	PLOT0270 PLOT0275 PLOT0280
85	C Y BARRIERS C 250 IF ( .NOT. YBARR ) GO TO 270 IF ( I-CY(I) .LT. 0 ) GO TO 240 IF ( IZY(I) .LE. 0 ) GO TO 241	PLOT0285 PLOT0290 PLOT0295
90	C OUTER BARRIER DO 205 L=1,IS 205 PAGE(I+L-2,J+L-3) = X GO TO 300	PLOT0300 PLOT0305 PLOT0310
95	C INNER BARRIER 241 DO 206 L=1,IS 206 PAGE(I+L-2,J+L-3) = X GO TO 300	PLOT0315 PLOT0320 PLOT0325
100	C BOTH BARRIERS 240 DO 207 L=1,IS PAGE(I+L-2,J+L-3) = X 207 PAGE(I+L-2,J+L-3) = X 300 CONTINUE	PLOT0330 PLOT0335 PLOT0340 PLOT0345
105	C	

```

C      LAND BARRIERS
C
110  C      DO 804 K=1,KLM                                PLOT0350
C
C      TEST FOR BARRIER IN X DIRECTION, Y DIRECTION, OR BOTH
C
115  C      KB = KLR(K)                                PLOT0355
C      II = IR(KR)                                    PLOT0360
C      JJ = JR(KR)                                    PLOT0365
C      I = IARS(II)+1                                PLOT0370
C      J = IABS(JJ)+7-4                               PLOT0375
C      IZ1 = I7(II,JJ)*10                            PLOT0380
C      IZ2 = I7(II+1,JJ)*10                          PLOT0385
120  C      IZ0 = IZ(KR)                                PLOT0390
C      XBARR = .TRUE.                                  PLOT0395
C      IF ( IZP .EQ. IZ1 .OR. IZ0 .EQ. IZ2 ) XBARR = .FALSE. PLOT0400
C      IZ2 = IZ(II,JJ+1)*10                          PLOT0405
C      IZ0 = IZ(KR)                                    PLOT0410
125  C      YBARR = .TRUE.                              PLOT0415
C      IF ( IZR .EQ. IZ1 .OR. IZ0 .EQ. IZ2 ) YBARR = .FALSE. PLOT0420
C
C      X BARRIER
C
130  C      IF ( .NOT. YBARR ) GO TO 220                PLOT0425
C      DO 208 L=1,7                                    PLOT0430
C      208 PAGE(I+2,J+L-3) = X                         PLOT0435
C
C      Y BARRIER
C
135  C      220 IF ( .NOT. YBARR ) GO TO 804            PLOT0440
C      DO 209 L=1,5                                    PLOT0445
C      209 PAGE(I+L-3,J+4) = X                         PLOT0450
C      804 CONTINUE                                   PLOT0455
140  C
C      DRAW CHANNELS
C
145  C      251 DO 802 K=1,KCM                            PLOT0460
C      I = IABS(ICG(K))+4-1                            PLOT0465
C      J = IABS(JCG(K))+7-4                            PLOT0470
C      I4 = I+3                                        PLOT0475
C      J4 = J+6                                        PLOT0480
C      IF ( I-CX(K) .EQ. 0 ) GO TO 300                PLOT0485
C      DO 200 L=1,7                                    PLOT0490
150  C      200 PAGE(I4,J+L-1) = MLINE                  PLOT0495
C      IF ( KCX(K) .GT. KCM ) PAGE(I4,J) = PLUS      PLOT0500
C      300 IF ( I-CY(K) .EQ. 0 ) GO TO 301            PLOT0505
C      DO 201 L=1,3                                    PLOT0510
155  C      PAGE(I+L-1,J+5) = BLNK                      PLOT0515
C      PAGE(I+L-1,J+7) = BLNK                      PLOT0520
C      201 PAGE(I+L-1,J4) = VLINE                    PLOT0525
C      IF ( KCY(K) .GT. KCM ) PAGE(I,J4) = PLUS     PLOT0530
C      301 PAGE(I4,J4) = PLUS                        PLOT0535
C      802 CONTINUE                                   PLOT0540
160  C
C      WRITE OUT THE PAGE
C
165  C      WRITE(6,501)(J, I=1,19),((PAGE(4*K-1,J),J=3,134),K,(PAGE(4*K ,J),J=1,28),K=1,2R) PLOT0545
C      I = 25,134),((PAGE(4*K+1 ,J),J=3,134),I=1,2),K=1,2R) PLOT0550
C      501 FORMAT (1(1,1P(14,3X),14,/,1(1,5X,17(1,1,6X),1,1,5X,1,1,/,2(1X, PLOT0555
C      I 132A1,/,17,12,130A1,/,2(1X,132A1,/,),1(1) PLOT0560
C      RETURN                                         PLOT0565
C      END                                             PLOT0570

```

## APPENDIX B

### DESCRIPTION OF THE SURGE II CODED PROGRAM

The general strategy of the program is discussed and certain special features are pointed out which may not be apparent without detailed study of the program. Operational aspects of the program are discussed in some detail in Appendix C.

The version of the program adapted for use on the GE 400 computer system by the Corps of Engineers consists of the following parts or subroutines:

- MAIN            whose primary job is to read and check the sequencing of the basic data for the block computations;
- PART 2         which controls the basic computational sequencing, initialization, and updating of storage, interpolation of coarse wind fields for the actual grid, and routine computation of U, V, and H for all blocks, considering barriers (basically, the SURGE I program);
- CHANL(1)      which is called only once to read channel data and to establish certain key arrays for routine calculation;
- CHANL(2)      which is called routinely to compute flow and water levels in channels and at channel end points;
- CHANL(3)      whose task is the routine calculation of H on blocks containing channels;
- CHANL(4)      which is called for listing of channel computations;
- LIST(1)        which is called only once to read control data for block listings and to list the topographic Z field;
- LIST(2)        which lists the H field for blocks if called;
- LIST(3)        which lists the U, V, and H fields for blocks if called in place of LIST(2);
- SAVE(1)        which is called only once to read the positions of certain gage locations for water level or flow;
- SAVE(2)        which is called routinely at preselected time intervals to save water levels and flow for gage locations defined by SAVE(1);
- CONTIN(1)     which is called only once to read basic storage in COMMON BLOCKS 1 to 10 in the case of a continuation of a given problem;
- CONTIN(2)     which is called at the termination of a run to output the continuation data called for by CONTIN(1).

The version of the program used in the testing and calibration work, using an IBM 360/65 computer system, has an additional assembler language subroutine for plotting positions of barriers and channels (see Fig. 15). This is useful in checking input data for channels and barriers to spot possible errors in coding the positions of channel blocks and barrier blocks. Unfortunately, this subroutine is not compatible with the GE 400 system. Subroutine PLOT in Appendix A however can be used for this purpose. Subroutine LIST is not used in the version of the program in Appendix A.

## 1. Flow Diagram.

A schematic flow diagram for the SURGE II program is given in Figure B-1. If a new problem is being run then the first phase is reading in the basic data and checking the data sequencing to make sure it is in order and complete. This is carried out in MAIN and the beginning of PART 2 which calls subroutines CHANL(1), SAVE(1), and LIST(1).

Initialization of block arrays is carried out in PART 2; initialization of channel arrays and establishing of key arrays are carried out by CHANL(1). These key arrays are discussed in a subsequent subsection.

Step 4 of the flow diagram is the beginning (or reentry point) of the routine computations for each time. After generating, the detailed interpolated fields of  $x$  and  $y$  components of wind stress for the blocks (step 4) and all blocks (i.e., all  $I, J$ ) are swept to compute the flow components,  $U$  and  $V$ , ignoring at first the presence (if any) of subgrid scale channels, but considering barriers for any barrier blocks (step 5).

In step 6 CHANL(2) is called to sweep through all channel blocks to evaluate all channels  $Q$  and  $H$  except those for  $H$ -end points and all lateral flows to and from channels. In the latter operation, the flows  $U$  and  $V$  computed in step 5 are replaced by corrected  $U$  or  $V$  between blocks, considering the presence of the channels.

Step 7, which is carried out in PART 2, sweeps all  $I, J$  to compute water levels on blocks ignoring for the present, the presence of any subgrid scale channels.

In step 8, CHANL(3) is called to correct the block  $H$  values on those blocks containing channels and to compute the  $H$  and  $Q$  values at  $H$ -end points of channels. This also provides corrected  $H$  values for those blocks into which the channels discharge.

Steps 10 and 11 are output operations for block and channel computations carried out in PART 2 and CHANL(4). This is followed by a time updating and test for end, dependent upon a prescribed maximum number of time steps. Before termination of a run, the contents of all data in COMMON are saved for possible continuation of the problem, if desired.

## 2. Identification of Adjacent Channel Blocks.

To provide rapid access to values of  $H$  and  $Q$  in channels adjoining a given channel reach, special arrays are generated in subroutine

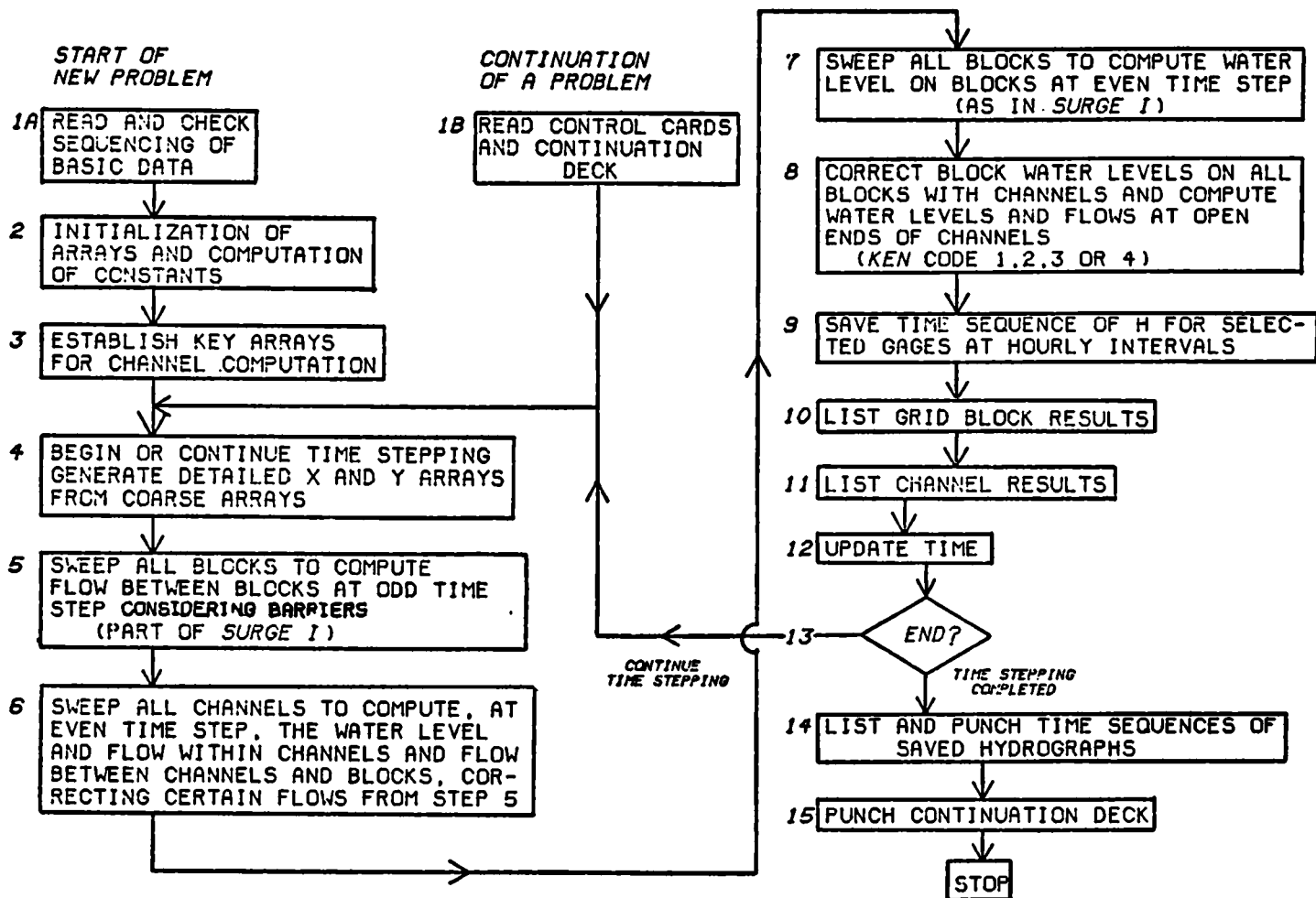


Figure B-1. Generalized flow diagram for SURGE II.



CHANL(1). There are four such arrays: KCX(K), KCY(K), KCXP(K), and KCYP(K). These give the channel block identification index for those channel blocks which are adjacent to the Kth channel block as indicated in Figure B-2. Thus, KCX(K) is the identification of the channel block which has an x-side channel adjoining channel block K on the negative characteristic side (i.e., on a preceding row), while KCXP(K) is the identification of the channel block which has an x-side channel adjoining channel block K on the positive side (i.e., on a following row). KCY(K) and KCYP(K) have analogous meanings for blocks with y-side channels adjoining that of block K. These arrays are generated by an appropriate series of tests in which the I,J values of blocks adjacent to that of channel block K are compared with the ICG and JCG values of all other channel blocks. This is carried out only once during any run, and is not particularly time consuming; moreover, it avoids any human error which may easily occur if such arrays were required as input.

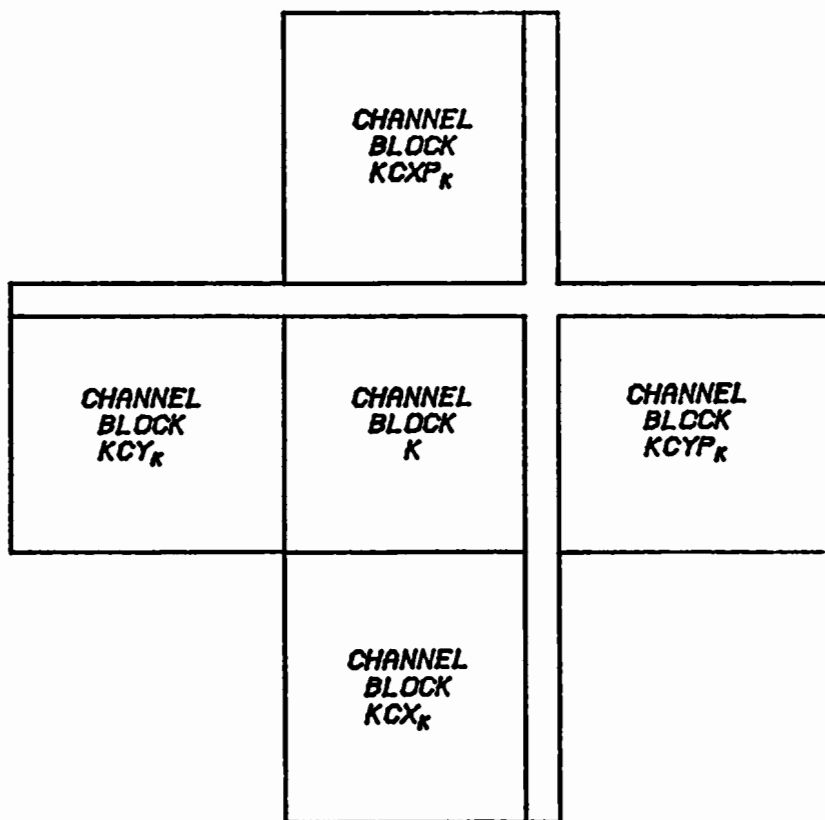


Figure B-2. Channel block identification for channels adjacent to those of block K.

The arrays KCX and KCXP have the properties  $KCXP(KCX(K)) = K$  and  $KCX(KCXP(K)) = K$  with similar relations for KCY and KCYP.

As an example of the use of such arrays, suppose the value of HC in an x channel adjoining that of channel block K is needed. This could

be addressed as HC(KX) where  $KX = KXC(K)$ . Using Figure 8 as an example, the values of channel flow entering the junction from channels 1 and 2 would be addressed by QCXP(K1) and QCYP(K1), respectively, where K1 designates the channel block containing channels 1 and 2. However, the flow leaving the junction would be addressed by QCYN(K2) where  $K2 = KCYP(K1)$  and QCXN(K3) where  $K3 = KCXP(K1)$ . While redundant storage of such H and Q values would also satisfy the requirement of rapid access to such values adjoining a given channel block, the use of the integral arrays KCX, KCY, KCXP, and KCYP saves storage for most computer systems.

An examination of the listings of the values of the arrays KCX, KCY, KCXP, and KCYP, as output by the program, indicates that the maximum value of any of these can and usually does exceed the number of input channel blocks (KCM). The reason for this is that dummy storage positions are created for blocks adjoining channel end points. This is an artifice of the program which allows routine computation for all channel reaches before special computation for channel end points.

### 3. Barrier Identification.

The position of the Kth barrier block is given by the array pair, IB(K) and JB(K), which is input to the program. It is convenient to have rapid access to barrier information for those barriers which happen to fall on a given channel block. The array KCB(K) gives the identification of the barrier block which coincides with channel block K. Thus,  $ICG(K) = IB(KCB(K))$  and  $JCG(K) = JB(KCB(K))$ . If no barriers exist in a given channel block then the corresponding value of KCB is zero. Thus, in the routine program, a test for zero value KCB is made; if nonzero, then a call can be made for barrier data such as elevation and barrier coefficients via the barrier index  $KB = KCB(KC)$  where KC is the channel block concerned.

The array KCB(K) is generated in CHANL(1), via a scan of all IB and JB values for given ICG and JCG for channel block K.

An array KLB(K) is also generated which identifies those barrier blocks not common to channel blocks. This is used only in the IBM 360/65 assembler language plotting routine, not in routine calculations.

### 4. Channel End-Point Identification.

As a signal that at least one channel end point occurs in a channel block K, the value of ICG(K) is negative. If two end points occur, the value of JCG(K) is also negative; otherwise, it is positive. If no channel end point occurs, then both ICG and JCG for the block are positive. This positive-negative coding is generated automatically in CHANL(1) by appropriate testing; namely, to check if a valid channel connects at each end of a valid channel in the block concerned.

In addition, the arrays KEN(1,K) and KEN(2,K) are generated in CHANL(1) to identify the type of end point for, at most, two potential

channel terminations in given channel block K. If there is no channel termination both  $KEN(1,K)$  and  $KEN(2,K)$  are zero; if one termination occurs for block K,  $KEN(1,K)$  will have an integral value from 1 to 8 and  $KEN(2,K)$  will be zero; if two terminations occur, both  $KEN$  arrays will have nonzero value. In use,  $KEN(2,K)$  is called only if  $JCG(K)$  is negative.

The coding for the type of end point is indicated schematically in Figure B-3. Values of  $KEN$  from 1 to 4 represent "H-end" type terminations where a ponding block immediately adjoins the channel end. Values of  $KEN$  from 5 to 8 are those for which  $Q$  is specified; e.g., river discharge. Values within either group indicate the relative orientation of the channel end point in question to assure calling the correct data and using the right signs in the routine calculations.

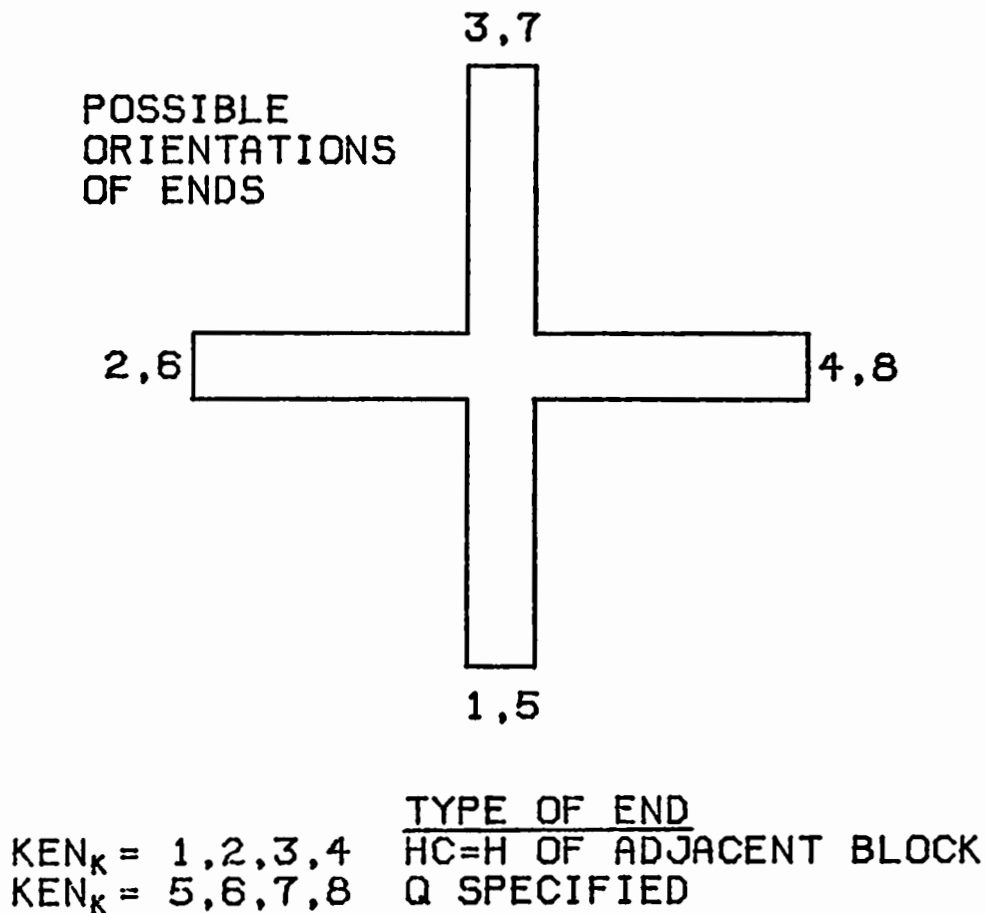


Figure B-3. Identification of type and orientation of a channel end point by the coded identifier  $KEN(K)$ .

## APPENDIX C

### USER'S GUIDE TO SURGE II

The coded program SURGE II is intended for use in the numerical simulation of storm surges or astronomical tides in bays and estuaries for specified time sequences of water level at the seaward boundary of the bay or estuary and specified wind stress and other storm data over the bay or estuary. The user may use one of two distinct modes of operation: (a) the storm mode, in which all storm data are required as well as seaward hydrograph data; or (b) the tide mode, in which no storm data are required, the only forcing being the input water level variation at the seaward boundary. Moreover, in both modes the user has the option of initiating a new simulation or continuing a previous simulation, the input requirements being different for each.

In general, the input consists of the following types of information:

(a) Control Data--For input-output operations, initialization, array size, time stepping, and run duration.

(b) Bay Schematization Data--including block topography, barrier data, and channel data.

(c) Forcing Data--including sequences of water level at seaward boundary, wind-stress components over bay, rainfall data over bay, and river discharge data.

(d) Problem Specification Information.

Certain checks are made as the data are read in, with regard to proper order of input, proper amount of sequential data, and proper size arrays. All stops resulting from these editing checks of input are identified.

In the subsequent subsections, the individual input parameters are identified (with appropriate units), the sequence of data input for the different modes of operation is given in some detail, and special requirements concerning data input for barriers and channels are discussed, followed by a summary of output information and output options.

#### 1. Definition of Input Variables.

The following variables are listed in the order in which they are input (asterisks separate data blocks):

ICARD      Control index: 0 for starting, 1 for continuation.

\*\*\*\*\*      *Block 0*

IDENT      Data block identification;

IBL starting column (I value) for listing of block H output  
(normally taken as 1);

KCM total number of blocks with channels (including null channels,  
see subsec. 6 of this app.);

NOWIND control for storm data input: 0 for normal input operation for  
wind stress, rainfall, and runoff; -1 for omitting such input  
for tide computations;

INTER interval in SAVE operation (time interval is INTER\*DELT);

NGAGE number of H gage locations saved;

NFLOW number of Q gage locations saved;

IMIN minimum expected H (feet);

IMAX maximum expected H (feet).

NOTE----IMIN and IMAX are used only in subroutine GRAF, applicable to  
IBM 360 or 370.

\*\*\*\*\* *Block 1*

NTIME Initial time level (normally 0, unless a continuation run is  
being carried out, in which case NTIME should equal the final  
value of the previous run);

NM maximum number of time steps for the problem;

MMIN minimum "map time" for wind-stress input;

MMAX maximum map time for wind-stress input;

NFU number of iterations per map time interval;

IOUT interval for routine output from blocks and channels equals  
IOUT + 1;

INFLD special output flag: 0 for standard output, 1 for extra  
listing of channel output for one iteration preceding normal  
listing.

\*\*\*\*\* *Block 2*

IM Total number of x-grid intervals;

JM total number of y-grid intervals;

KM total number of blocks having barriers;

KMAX        total number of coarse x-grid points for wind-stress input;

LMAX        total number of coarse y-grid points for wind-stress input.

\*\*\*\*\*     *Block 3*

DELX        Spatial grid interval or block size (nautical miles);

DELT        time interval between block H and flow computations (seconds);

CDO        overflow coefficient for natural low-lying ground such as  
barrier islands;

FK         bed-resistance coefficient for blocks;

FC         bed-resistance coefficient for channels (used only if values  
for individual channels are not entered);

HGI        initial water level above MSL in the bay (feet).

\*\*\*\*\*     *Block 4*

KI         Number of interpolation subdivisions of each coarse x-grid  
interval  $KI*(KMAX-1) = IM$ ;

LJ         number of interpolation subdivisions of each coarse y-grid  
interval  $LJ*(LMAX-1) = JM$ ;

KII        number of coarse x-grid intervals;

LJJ        number of coarse y-grid intervals;

JBL, JBR   number of "open boundary" J-intervals on left and right  
of system (not used in version in App. A).

\*\*\*\*\*     *Block 5*

IB(K)      I location index for barrier block K;

JB(K)      J location index for barrier block K;

IZX(K)    elevation of x-barrier (right side) on barrier block K (tenths  
of feet);

IZY(K)    elevation of y-barrier (upper side) on barrier block K (tenths  
of feet);

ICDOX(K)  overflow coefficient for x-barrier (value  $\times 1,000$ ) on Kth  
barrier block;

ICDOY(K) overflow coefficient for y-barrier (value  $\times 1,000$ ) on Kth barrier block;

ICDSX(K) submerged wier coefficient for x-barrier (value  $\times 1,000$ ) on Kth barrier block;

ICDSY(K) submerged wier coefficient for y-barrier (value  $\times 1,000$ ) on Kth barrier block.

\*\*\*\*\* *Block 6*

IZ(I,J) Elevation of ground or seabed (feet) relative to MSL datum for block location I,J.

\*\*\*\*\* *Block 7*

IMRO Number of river input (runoff) locations;

JMRO number of map times with runoff values;

KR number of channel-stress values (normally same as JMRO);

ISTR start of rain (map time);

IND end of rain (map time);

NOW number of iterations between river input values (normally same as NFU);

KIM number of iterations between channel-stress values (normally same as NFU);

NORT number of iterations per hour for rain (normally same as INTER).

\*\*\*\*\* *Block 8*

RF Total rainfall (inches);

CONST fraction of rainfall not absorbed by ground;

S conversion factor for wind stress  $(5,280/3,600)^2 \times 1.1/10$ .

\*\*\*\*\* *Block 9*

LROI(K) I location index for Kth river input block;

LROJ(K) J location index for Kth river input block.

\*\*\*\*\* *Block 10*

DIST(M) Percent of total rainfall per hour for 24 hours.

\*\*\*\*\* *Block 11*

CHST(M) Channel-stress values at map time M (entries are used only if KCM = 0).

\*\*\*\*\* *Block 12*

RO(K,M) Discharge (cubic feet per second) from Kth river input block at map time M.

\*\*\*\*\* *Block 13*

MTIME Map time for given block of wind-stress input and seaward water level.

\*\*\*\*\* *Block 14*

HGR(K) Seaward water level above MSL (feet) at MTIME for coarse grid position K.

\*\*\*\*\* *Block 15*

HBR(J) Water level on right open boundary above MSL (feet) at MTIME for grid position J (not used in version in App. A).

\*\*\*\*\* *Block 16*

XR(K,L) Wind-stress component in the x direction (units of (miles per hour)<sup>2</sup>/10) for coarse grid position K,L at time MTIME.

\*\*\*\*\* *Block 17*

YR(K,L) Wind-stress component in the y direction (units of (miles per hour)<sup>2</sup>/10) for coarse grid position K,L at time MTIME.

\*\*\*\*\* *Block 18*

ICG(K) I location index for channel block K;

JCG(K) J location index for channel block K;

IWCX(K) width of x channel (right side) on channel block K (feet), with sign (see subsec. 6 of this app.);

IZCX(K) depth of x channel bed on channel block K (feet), with sign (see subsec. 6 of this app.);



IWCY(K) width of y channel (upper side) on channel block K (feet), with sign (see subsec. 6 of this app.);

IZCY(K) depth of y channel bed on channel block K (feet), with sign (see subsec. 6 of this app.);

IFC(K) bed-resistance coefficient for channels on block K (value  $\times 10,000$ ), if entry is zero (blank) then IFC is taken as FC (entered in *Block 3*)  $\times 10,000$ .

\*\*\*\*\* *Block 19*

IGAGE(K) Location index for the Kth hydrograph, if JGAGE(K)  $\neq 0$  then IGAGE(K) is the I location of a block H; if JGAGE(K) = 0 then IGAGE(K) is the channel block index for a channel H;

JGAGE(K) if not zero, this is the J location of a block H; if zero, a channel H is indicated;

KFLOW(K) channel block index for the Kth flow gage, the flow being that of the lower end of the x channel, or the left end of a y channel if an x channel does not exist, or a channel end point if one exists in the identified channel block.

\*\*\*\*\* *Block 20*

IEND Maximum I in listing of block arrays of H, U, and V;

NF number of iterations between listings;

IBEGIN first I in listing of block arrays;

NJ maximum J in listing of block arrays;

NCARD total number of alphanumeric problem identification cards;

ALPHA(J) alphanumeric character data which identify the problem and gage locations by name.

## 2. Input for Initiating Storm Surge Simulation.

The sequence of input for starting a problem in the storm surge mode is given below in the form of a summary of the READ statements active in this mode, together with a summary of the appropriate FORMATS for data input in different blocks. For all data blocks requiring an entry of the identification integer IDENT, only the *wits digit* of the data block number is entered in column 1 of the data input card.

*Control Card*

READ 1 , ICARD (0 for starting)

*Block 0 (1 card)*

READ 1 , IDENT, IBL, KCM, NOWIND, INTER, NGAGE, NFLOW, IMIN, IMAX

NOTE-----IMIN and IMAX are left blank unless subroutine GRAF is used.

*Block 1 (1 card)*

READ 100 , IDENT, NTIME, NM, MMIN, MMAX, NFU, IOUT, INFLD

*Block 2 (1 card)*

READ 100 , IDENT, IM, JM, KM, KMAX, LMAX

*Block 3 (1 card)*

READ 250 , IDENT, DELX, DELT, CDO, FK, FC, HGI

*Block 4 (1 card)*

READ 100 , IDENT, KI, LJ, KII, LJJ, JBL, JBR

*Block 5 (total of KM cards of barrier data)*

DO 500 K = 1, KM

READ 100 , IDENT, IB(J), JB(K), IZX(K), IZY(K), ICDOX(K), ICDOY(K),  
ICDSX(K), ICDSY(K)

500 CONTINUE

*Block 6 (total of 2\*IM cards of block topography)*

DO 550 I = 1, IM

READ 100 , IDENT, (IZ(I,J), J = 1,10)

READ 100 , IDENT, (IZ(I,J), J = 11, JM)

550 CONTINUE

*Block 7* (1 card)

READ 100 , IDENT, IMRO, JMRO, KR, ISTR, IND, NOW, KIM, NORT

*Block 8* (1 card)

READ 250 , IDENT, RF, CONST, S

*Block 9* (1 or 2 cards, dependent on IMRO)

READ 100 , IDENT, (LROI(K), LROJ(K), K = 1,5)

IF (IMRO.LT.6) GO TO 575

READ 100 , IDENT, (LROI(K), LROJ(K), K = 6, IMRO)

575 CONTINUE

*Block 10* (3 cards)

READ 250 , IDENT, (DIST(M), M = 1,10)

READ 250 , IDENT, (DIST(M), M = 11,20)

READ 250 , IDENT, (DIST(M), M = 21,24)

*Block 11* (L + 1 card where L = KR/10. If KR = 0, block 11 input is omitted.)

READ 250 , IDENT, (CHST(K), K = 1,11)

READ 250 , IDENT, (CHST(K), K = 11,20)

...

READ 250 , IDENT, (CHST(K), K = KL, KR (KL = 10 \* L + 1)

*Block 12* (JMRO cards of river discharge data)

DO 700 M = 1, JMRO

READ 250 , IDENT, (RO(K,M), K = 1, IMRO)

700 CONTINUE

*Wind Stress and Water Level Forcing*

(MTL sets of blocks 13 to 17 where MTL = MMAX - MMIN + 1)

710 CONTINUE

*Block 13* (1 card)

```
READ 100 , IDENT, MTIME
```

*Block 14* (1 card)

```
READ 250 , IDENT, (IIGR(K), K = 1, KMAX)
```

*Block 15* (1 card)

```
READ 250 , IDENT, (HBR(J), J = 2,8)
```

*Block 16* (KMAX cards)

```
DO 790 K = 1, KMAX
```

```
READ 250 , IDENT, (XR(K,L), L = 1, LMAX)
```

```
790 CONTINUE
```

*Block 17* (KMAX cards)

```
DO 800 K = 1, KMAX
```

```
READ 250 , IDENT, (YR(K,L), L = 1, LMAX)
```

```
800 CONTINUE
```

```
IF (MTIME - MMAX) 710, 1,015, 1,015 (710 returns to read block 13)
```

```
1,015 (CONTINUE)
```

*Block 18* (KCM cards with channel data. If KCM = 0, the READ statement is bypassed and block 18 should be omitted.)

```
IF (KCM.GT.0) CALL CHANL(1)
```

```
DO 50 K = 1, KCM
```

```
READ 100 , IDENT, ICG(K), JCG(K), IWCX(K), IZCX(K), IWCY(K), IZCY(K),  
IFC(K)
```

```
50 CONTINUE
```

*Block 19 (2 cards)*

```
CALL SAVE(1)
READ 350 , (IGAGE(K), JGAGE(K), K = 1, NGAGE)
READ 350 , (KFLOW(K), K = 1, NFLOW)
```

*Block 20 (NCARD + 1 card)*

```
CALL LIST(1)
READ 1 , IDENT, IEND, NF, IBEGIN, NJ, NCARD
DO 250 J = 1, NCARD
READ 450 , (ALPHA(J), J = 1,40)
250 CONTINUE
```

Format Statements for Input. The following formats were used in all the testing operations. It is recommended, however, that for routine operations those READ statements using FORMAT 1 be replaced by FORMAT 100 to make all basic numerical input consistent in card column range.

```
1 FORMAT (I1, I3, 19, I4)
100 FORMAT (I1, 2X, I5, 9(3X, I5)
250 FORMAT (I1, F7.0, 9F 8.0)
350 FORMAT (20 I 4)
450 FORMAT (15A2, 15A2, 10A2)
```

### 3. Input for Tide Mode.

For calibration of a given bay system, under virtually no wind conditions, for its response to forcing by astronomical tide at the seaward boundary and a steady-state river discharge, allowance is made in the coded program to bypass the detailed input of wind-stress components, and rainfall and channel-stress data; moreover, since a steady river discharge is assumed only a single card is required to define this input. In essence, the data blocks 10 to 17 are replaced by a shortened version of block 12 plus a modified version of block 14 in which tide data at the seaward boundary are prescribed at hourly intervals as the map time intervals. The input is summarized as follows:

*Control Card:* 0 in column 1

*Block 0:* see Section IV,1, NOWIND = -1

*Blocks 1 to 9* see Section IV,2

*Block 12* (1 card for steady river discharges)

```
READ 250 , IDENT, (RO(K,M), K = 1, IMRO)
```

*Astrotide Block* (1 card for each 12 hours)

```
905 READ 910, IGA, MTIME, (H(1,J), J = 1,12)
```

```
MU = MTIME + 12
```

```
IF (MU.LT.MMAX) GO TO 905
```

```
910 FORMAT (I2, I4, 12F 6.2)
```

```
(IGA = 1)
```

*Blocks 18 to 20:* see Section IV,2

Comments on Tide Mode. The map time interval for the tide mode is 1 hour. The MTIME entry for the astrotide block is the time (hour) of the first of 12-hourly values of HG (entered as H(1,J)). The tide is assumed uniform along the seaward boundary of the bay system, hence one HG value per hour is sufficient.

In starting the tide mode from rest state ( $U = V = 0$  and  $H = HGI = 0$ ), usually one or two diurnal tide cycles are required for the numerical model to reach a nearly periodic response to an almost periodic input. Thus, if the final diurnal cycle is to be free of initial transients, at least 72 hours of HG data should be provided. This may require an adjustment in the dimensions given in COMMON/BLK6/ which appears in subprograms MAIN, PART 2, and CONTIN, if the full data set is to be stored for one run. An alternative is to make use of the continuation option, using less data input per run (e.g., 24 hours).

#### 4. Input for Continuation of a Run.

Since the main purpose of the tide mode is for calibration of the bed friction coefficients for blocks and channels, it is expected that many trial runs will be made for a given bay system. In order to keep the machine time to a minimum for each successive run, it is desirable to use an initial field of  $U$ ,  $V$ , or  $H$  which is close to the true response at the starting time. This can be accomplished by using the resulting  $U$ ,  $V$ , and  $H$  arrays from a previous tide run for the bay system as the initial values. (This should be done even if the previous run has different values of the bed friction coefficients.) The mechanism for

accomplishing this is the use of the continuation mode option, as controlled by ICARD. In this mode, the contents of common from a previous run are input along with any additional forcing function data.

To make the program as flexible as possible, the continuation option can be used for either storm surge problems or astrotide problems, the only difference in input being in the type of forcing function input. Such forcing function data should be consistent with the continuation time. Moreover, the value of NTIME input in data block 1 should be equal to the final NTIME in the previous run which is continued.

The sequence of input for continuation of a problem is as follows:

- Control Card:* 1 in column 1
- Contin Deck:* Contents of COMMON output from a previous run
- Blocks 0 to 3:* see subsection 2 of this app. (4 cards)
- Forcing Deck:* For storm surge mode, blocks 13 to 17, inclusive.  
For tide mode-astrotide deck.

A flow diagram summarizing the READ operations as controlled by ICARD and NOWIND is given in Figure C-1.

## 5. Comments on Barrier Input.

a. Possible Barrier Locations. All barriers in the schematization occur parallel to the sides of a given barrier block. Barrier data qualified by an X in the coded name (e.g., IZX, ICDOX, ICDXS) refer to barriers normal to the x-axis on the right side of the barrier block; those qualified by a Y in the coded name (e.g., IXY, etc.) refer to barriers normal to the y-axis on the upper side of the barrier block. If a channel exists parallel to either barrier, then such a barrier may occur on either or both sides of the parallel channel, depending upon the coding of the associated channel input data (as discussed in a subsequent subsection). Barriers which might exist along the left or lower side of a given block are represented by appropriate data coding of a barrier block in a previous row or column.

b. Precaution. It should be emphasized that for any barrier block it is up to the user to supply appropriate barrier elevations ZB for both the right and upper sides of the barrier block even if a real barrier occurs only on one side of the block. The important point to observe is that the specified ZB values should always equal or exceed the larger of the block elevations at or adjacent to the side of the barrier block in question. Otherwise, errors can occur in the computations.

c. Array Size. The number of barrier blocks KM is normally limited to less than 100.

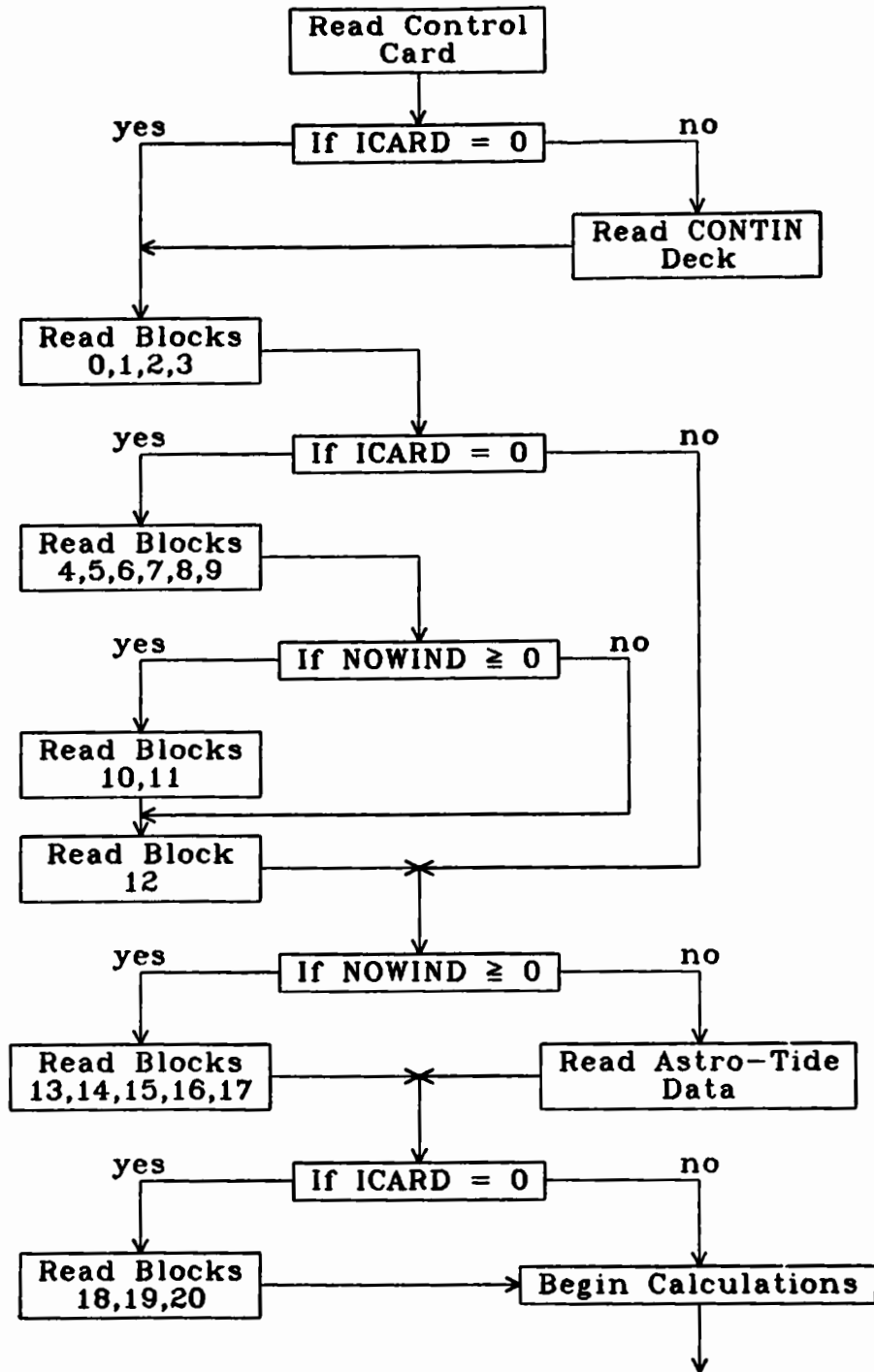


Figure C-1. Flow diagram for read statements.



6. Comments on Channel Input.

a. Possible Channel Locations. All channels in the schematization occur along the right side or the upper side of a given channel block. Channel data qualified by an X in the coded name (e.g., IWCX, IZCX) refer to channels normal to the X-axis on the right side of the channel block; those qualified by a Y in the coded name (e.g., IWCY, IZCY) refer to channels normal to the Y-axis on the upper side of the channel block. If a block has both an X and Y channel, one data card specifies both.

b. Channel Junctions. In the schematization of a channel system junctions can occur with adjoining channel reaches parallel to each other or perpendicular. Moreover, one-, two-, or three-way branches are possible.

Four possible right-angle channel junctions are illustrated in Figure C-2. The simplest junction is that shown in the upper right panel of the figure where the joining channel reaches are in the same channel block K1. Right-angle junctions involving two adjacent channel blocks are illustrated in the upper left and lower right panels of Figure C-2.

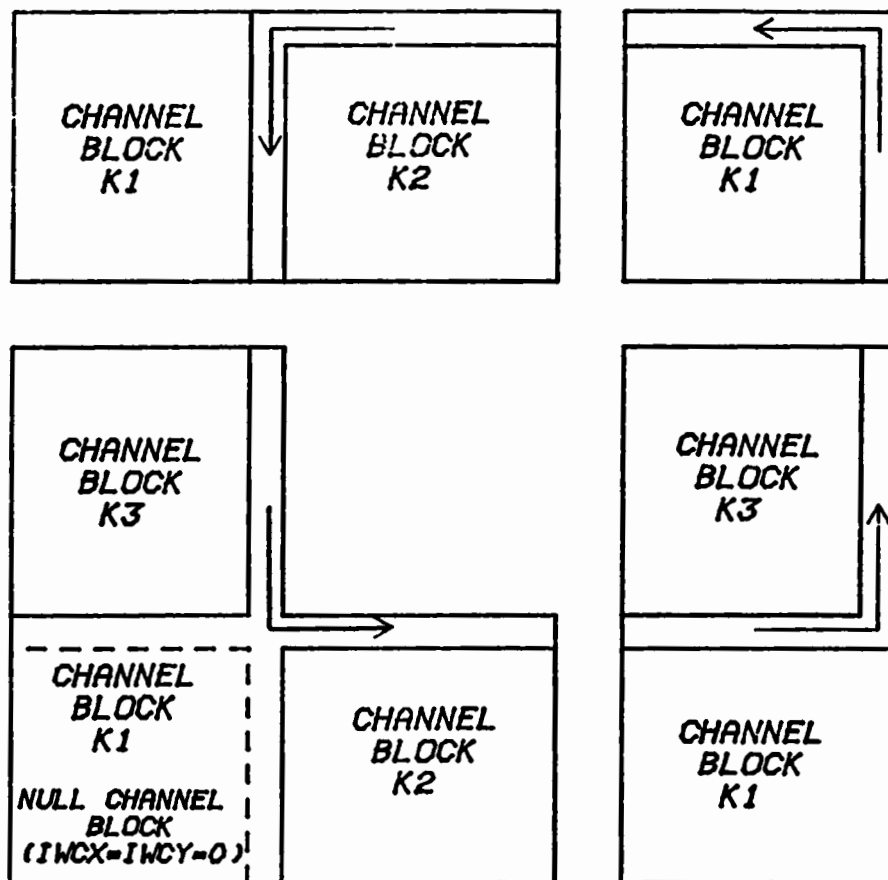


Figure C-2. Four possible simple bends for a channel reach.

The final possible right turn is illustrated in the lower left panel of the figure. In this case, the program requires that a channel block (K1) join the connecting channels of the nonadjoining channel blocks (K2 and K3) even though no channels exist on the joining block K1. In such circumstances, the required "null" channel block would have zero width for both the X and Y channels ( $IWCS = IWCY = 0$ ) as input. The H value at the junction of the connecting channel reaches for this case is stored as  $HC(K1)$ ; i.e., in association with the null channel block.

Colinear adjoining channels always involve two adjacent channel blocks. Four possible junctions of this type are illustrated in Figure B-2 in relation to central channel block K.

c. Channels with Levees. The program allows for the following possible situations with respect to barriers parallel to channels:

- (a) Single barrier on the "inner" lateral boundary of a channel;
- (b) single barrier on the "outer" lateral boundary of a channel;
- (c) barriers of *equal* elevation on both sides of a channel.

NOTE.--The term inner or outer side of a channel refers respectively to the side common to the channel block containing the channel or the side common to an adjacent block.

The barrier elevation information is input separately from the channel block data and allows only one elevation for the right side and one for the top side of a block (hence, the restriction of equal barrier heights for the double levee situation c above). The specification for situations a, b, or c is accomplished by a sign coding in the channel block data as follows:

- (a) Channel width (ICW) positive, channel-bed elevation (IZC)
- (b) channel width positive, channel-bed elevation positive;
- (c) channel width negative, channel-bed elevation negative.

It is understood that only the magnitude of IWC and IZC for a given channel is used in calculations.

d. Channel Terminations. A channel system can terminate at (a) a larger body of water representing a lake, bay, or sea; or (b) at a boundary or in a landlocked block within the system. In the second case, the program assumes that the flow at the channel end is zero unless a river discharge to the channel is specified (see input) and that the channel end block is one block inside the boundary block.

e. Restriction. Only channels with the channel bed below the mean water level (MWL) reference are allowed. The actual elevation used in calculations is  $-|IZC|$ , regardless of the sign on the input of IZC for a given channel.

f. Array Size. The number of channel blocks (including null channel blocks) is KCM. However, (CHANL(1)) creates arrays of length KCMP > KCM. The value of KCMP exceeds KCM by one plus the number of channels which terminate on the exterior boundary of the grid including the seaward boundary. Since KCMP is limited to 130, KCM should be less by the amount described above.

## 7. Output.

a. Listings of Input and Key Arrays. All input data are listed in easily identifiable form in the order in which the data are entered through block 18. Immediately following the basic channel input is a listing of the key arrays for channels, as discussed in Appendix A, including the assignment of sign coding for ICG and JCG.

Also printed out, in the same block format as the routine listings of H, are the block elevations.

b. Sequential Output. Normally, the routine output of computed values includes block H arrays and listings of all channel variables at pre-determined intervals of time (as determined by IOUT). It is possible to list the U, V, and H arrays for blocks by changing the CALL LIST(2) statement following statement 2,100 in PART 2 to CALL LIST(3).

For channel listings, refer to Figure 6 for notation; the listings are ordered by channel block number K. The block location I,J is repeated (negative signs indicating end points). This is followed by HX, the water level (feet) and QXN, the volume transport (cubic feet per second) at the lower end of the x channel, then QCP, the transport at the upper end of the x channel. These are followed by HY, QYN, and QYP representing, respectively, the water level and flow at the left end and flow at the right end of the y channel. Next is HC, the water level at the junction of the x and y channels. The last four entries in the channel listings are the transports (in cubic feet per second) to the channel from the channel block and from the channel to an adjacent block for the x and y channels. The HC value is meaningful for null channels only.

c. Saved Time Sequences. Subroutine SAVE, if used, saves sequences of water level and flow at preselected locations (as identified in block 19 of the input). In the original version of the subroutine used with an IBM 360-65 computer the saved information was punched on cards to facilitate later graphing of the sequences.

APPENDIX D

COMPLETE DATA LISTING OF INPUT FOR  
SABINE-CALCASIEU REGION WITH  
FORCING DATA FOR HURRICANE CARLA

ICARD# 0 IBL# 1 KCM# 121 NOWIND# 1 INTEN# 15 NGAGE# 9 NFLD# 2 IMIN# -1 IMAX# 10

IDNT# 1 NTIME# 0 NME# 900 MMJ# 0 NMAX# 24 NFUB# 45 IOUT# 449 INFLD# 0  
IDNT# 2 I# 28 J# 20 K# 91 LMAX# 4 LMAX# 6  
IDNT# 3 DELX# 2.0 N MI DELT# 240. SEC CDD# .200 FKB .0010 FCB .0010 HGI# 3.200 FT  
IDNT# 4 KJ# 4 LJ# 4 KII# 7 LJ# 5 JBL# 2 JBR# 1

MIN OR MAX IN ERROR

IDNT#	5	BARRIER	DATA	Z	VALUES	IN	TENTHS	OF	FEET.	CD	VALUES	ARE	TIMES	1000			
K#	1	I#	8	J#	1	Z#	50	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	2	I#	20	J#	1	Z#	-150	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	3	I#	21	J#	1	Z#	-150	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	4	I#	22	J#	1	Z#	50	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	5	I#	23	J#	1	Z#	-80	Z#	90	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	6	I#	24	J#	1	Z#	-100	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	7	I#	25	J#	1	Z#	-100	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	8	I#	26	J#	1	Z#	-100	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	9	I#	27	J#	1	Z#	-100	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	10	I#	28	J#	1	Z#	-100	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	11	I#	1	J#	2	Z#	-80	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	12	I#	2	J#	2	Z#	-100	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	13	I#	3	J#	2	Z#	-120	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	14	I#	4	J#	2	Z#	-100	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	15	I#	5	J#	2	Z#	-70	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	16	I#	6	J#	2	Z#	10	Z#	60	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	17	I#	8	J#	2	Z#	50	Z#	20	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	18	I#	13	J#	2	Z#	-130	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	19	I#	14	J#	2	Z#	-120	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	20	I#	15	J#	2	Z#	-120	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	21	I#	16	J#	2	Z#	-120	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	22	I#	17	J#	2	Z#	-110	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	23	I#	18	J#	2	Z#	-80	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	24	I#	19	J#	2	Z#	60	Z#	40	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	25	I#	22	J#	2	Z#	80	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	26	I#	6	J#	3	Z#	60	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	27	I#	7	J#	3	Z#	20	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	28	I#	8	J#	3	Z#	50	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	29	I#	9	J#	3	Z#	10	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	30	I#	10	J#	3	Z#	10	Z#	40	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	31	I#	11	J#	3	Z#	30	Z#	40	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	32	I#	12	J#	3	Z#	50	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	33	I#	22	J#	3	Z#	40	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	34	I#	1	J#	4	Z#	10	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	35	I#	2	J#	4	Z#	10	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	36	I#	3	J#	4	Z#	10	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	37	I#	7	J#	4	Z#	50	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	38	I#	22	J#	4	Z#	30	Z#	-30	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	39	I#	3	J#	5	Z#	50	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	40	I#	4	J#	5	Z#	10	Z#	50	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	41	I#	5	J#	5	Z#	10	Z#	30	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	42	I#	4	J#	6	Z#	30	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400
K#	43	I#	6	J#	5	Z#	50	Z#	10	CDD#	200	CDU#	200	CCS#	400	CNS#	400

K# 44	I#	5	J#	6	Z#	30	Z#	010	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 45	I#	6	J#	6	Z#	50	Z#	40	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 46	I#	15	J#	6	Z#	10	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 47	I#	10	J#	6	Z#	10	Z#	60	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 48	I#	17	J#	6	Z#	0	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 49	I#	23	J#	6	Z#	100	Z#	10	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 50	I#	4	J#	7	Z#	30	Z#	10	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 51	I#	6	J#	7	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 52	I#	5	J#	7	Z#	30	Z#	010	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 53	I#	7	J#	7	Z#	040	Z#	140	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 54	I#	8	J#	7	Z#	080	Z#	180	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 55	I#	14	J#	7	Z#	50	Z#	10	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 56	I#	17	J#	7	Z#	40	Z#	0	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 57	I#	23	J#	7	Z#	100	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 58	I#	4	J#	8	Z#	30	Z#	10	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 59	I#	5	J#	8	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 60	I#	8	J#	8	Z#	50	Z#	30	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 61	I#	7	J#	8	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 62	I#	8	J#	8	Z#	140	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 63	I#	9	J#	8	Z#	080	Z#	180	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 64	I#	14	J#	8	Z#	50	Z#	10	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 65	I#	15	J#	8	Z#	0	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 66	I#	16	J#	8	Z#	0	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 67	I#	17	J#	8	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 68	I#	23	J#	8	Z#	120	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 69	I#	6	J#	9	Z#	50	Z#	30	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 70	I#	7	J#	9	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 71	I#	9	J#	9	Z#	140	Z#	70	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 72	I#	23	J#	9	Z#	120	Z#	10	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 73	I#	9	J#	10	Z#	140	Z#	70	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 74	I#	20	J#	10	Z#	10	Z#	150	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 75	I#	21	J#	10	Z#	10	Z#	150	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 76	I#	22	J#	10	Z#	10	Z#	150	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 77	I#	23	J#	10	Z#	120	Z#	120	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 78	I#	15	J#	11	Z#	20	Z#	80	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 79	I#	16	J#	11	Z#	10	Z#	100	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 80	I#	17	J#	11	Z#	20	Z#	70	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 81	I#	18	J#	11	Z#	50	Z#	70	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 82	I#	14	J#	11	Z#	100	Z#	100	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 83	I#	6	J#	12	Z#	80	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 84	I#	7	J#	12	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 85	I#	14	J#	12	Z#	20	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 86	I#	5	J#	13	Z#	50	Z#	150	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 87	I#	13	J#	13	Z#	50	Z#	120	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 88	I#	16	J#	13	Z#	30	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 89	I#	5	J#	14	Z#	10	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 90	I#	5	J#	16	Z#	50	Z#	50	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400
K# 91	I#	4	J#	17	Z#	50	Z#	100	C00X#	200	C00Y#	200	C05X#	400	C05Y#	400

IDNT#	6	ALOCK	TOPOGRAPHY	Z	VALUES	IN	FEET						
1	-24	-8	1	1	2	3	3	8	7	16			
1	11	10	23	24	32	35	30	20	35	100			
2	-24	-10	0	1	2	2	3	8	8	11			
2	7	14	15	23	25	35	30	20	35	100			
3	-24	-13	1	1	1	1	1	2	6	9			
3	4	7	11	16	22	25	30	7	7	100			
4	-24	-12	1	1	1	0	1	1	1	4			
4	12	13	14	20	23	15	3	7	7	100			
5	-24	-10	-1	0	1	-1	-1	-1	5	8			
5	15	17	15	1	1	1	7	7	7	100			
6	-14	-7	1	1	0	1	2	3	3	3			
7	15	7	1	1	7	1	10	20	25	100			
7	-8	1	1	1	-5	-5	-4	5	-1	5			
7	13	1	3	14	14	15	20	25	25	100			
8	-4	1	2	0	1	-5	-6	2	5	12			
8	12	1	6	14	14	20	25	25	30	100			
9	-15	2	2	1	1	-8	-7	-6	2	7			
9	1	1	13	15	17	15	25	27	30	100			
10	-20	-8	1	1	1	-5	-8	-8	-6	-4			
10	1	7	11	12	14	21	22	28	30	100			
11	-22	-10	1	3	1	1	-6	-7	-7	-6			
11	1	7	8	11	14	20	22	30	35	100			
12	-22	-13	3	2	1	1	1	1	-4	-3			
12	4	6	12	12	14	20	22	25	30	100			
13	-23	-15	5	2	1	1	1	1	1	1			
13	1	4	10	12	13	14	22	30	32	100			
14	-23	-13	4	1	1	1	1	1	1	1			
14	1	1	3	7	13	18	20	25	20	100			
15	-23	-12	2	1	1	1	0	0	1	1			
15	2	2	1	2	3	7	7	10	12	100			
16	-23	-12	1	1	1	1	0	0	1	1			
16	1	1	3	8	8	10	10	15	15	100			
17	-23	-12	1	1	1	0	0	0	1	3			
17	1	1	1	9	14	20	15	25	30	100			
18	-22	-11	1	1	1	0	0	1	3	3			
18	2	2	4	10	15	18	20	25	30	100			
19	-10	-8	-1	1	1	1	1	1	1	4			
19	5	8	8	11	14	18	20	20	32	100			
20	-17	1	1	1	1	1	1	1	1	1			
20	3	8	10	14	18	18	20	22	25	100			
21	-15	1	1	1	-4	1	0	7	-1	1			
21	3	6	10	12	14	14	20	22	25	100			
22	-15	1	1	-3	-4	1	1	5	0	1			
22	5	6	10	12	14	14	20	22	25	100			
23	-8	1	1	-4	-5	1	1	5	1	1			
23	4	6	12	13	14	14	20	22	25	100			
24	-17	1	1	-6	-7	-7	-7	-6	-6	1			
24	3	1	10	12	15	15	20	22	25	100			
25	-10	1	1	-5	-4	-5	-2	-6	1	1			
25	5	8	8	12	15	15	20	22	25	100			
26	-10	1	1	1	1	1	1	1	1	1			
26	4	8	1	-5	15	15	20	22	25	100			
27	-10	1	1	1	1	1	1	1	1	1			
27	4	8	2	1	12	15	20	22	25	100			
28	-10	100	100	100	100	100	100	100	100	100			
28	100	100	100	100	100	100	100	100	100	100			

IDNT# 7 IMRO# 3 JMRO# 25 KF# 25 ISTR# 25 INDE 16 NUW# 45 KIM# 45 NORT# 15  
 IDNT# 8 RF# 4.000 CONST# .9000 S# .23662

IDNT# 9 3 PAIRS OF I,J FOR RUNOFF LOCATIONS  
 9 2A 15 4 19 14 19 0 0 0 0

IDNT# 10 PERCENT RAINFALL EACH MAPTIME  
 0 .006 .0070 .0070 .0080 .0100 .0140 .0180 .0230 .0240 .0280  
 0 .032 .0360 .0470 .0630 .0840 .1070 .1460 .1400 .0650 .0390  
 0 .030 .0250 .0210 .0200

IDNT# 11 CHANNEL STRESS VALUES AT MAPTIMES  
 1 0.000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000  
 1 0.000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000  
 1 0.000 0.0000 0.0000 0.0000 0.0000

IDNT# 12 3 SETS OF RUNOFF VALUES IN CFS FOR 25 MAPTIMES  
 2 800. 1107. 1520.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 800. 1099. 1480.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 900. 1152. 1560.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.  
 2 1310. 1812. 2060.



MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 0

4	4.500	4.5000	4.5000	4.5000	4.5000	4.5000	4.5000	4.5000
5	3.200	3.2000	3.2000	3.2000	3.2000	3.2000	3.2000	3.2000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 0

6	-.029	-.0101	-.0090	-.0080	-.0062	-.0054
6	-.027	-.0164	-.0091	-.0081	-.0063	-.0055
6	-.030	-.0128	-.0100	-.0073	-.0064	-.0055
6	-.028	-.0143	-.0116	-.0073	-.0064	-.0055
6	-.028	-.0130	-.0105	-.0073	-.0064	-.0056
6	-.026	-.0225	-.0094	-.0074	-.0065	-.0056
6	-.026	-.0225	-.0094	-.0074	-.0065	-.0056
6	-.026	-.0225	-.0094	-.0074	-.0065	-.0056
7	-.011	-.0037	-.0033	-.0029	-.0023	-.0020
7	-.009	-.0055	-.0030	-.0025	-.0020	-.0018
7	-.009	-.0037	-.0030	-.0020	-.0017	-.0015
7	-.007	-.0036	-.0031	-.0020	-.0017	-.0015
7	-.006	-.0030	-.0024	-.0017	-.0015	-.0013
7	-.005	-.0044	-.0018	-.0014	-.0013	-.0011
7	-.005	-.0044	-.0018	-.0014	-.0013	-.0011

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 1

4	5.500	5.5000	5.5000	5.5000	5.5000	5.5000	5.5000	5.5000
5	4.500	4.5000	4.5000	4.5000	4.5000	4.5000	4.5000	4.5000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 1

6	-.033	-.0112	-.0112	-.0090	-.0070	-.0054
6	-.031	-.0200	-.0102	-.0091	-.0071	-.0054
6	-.032	-.0141	-.0120	-.0091	-.0071	-.0055
6	-.032	-.0172	-.0142	-.0092	-.0072	-.0055
6	-.032	-.0144	-.0117	-.0093	-.0073	-.0055
6	-.030	-.0244	-.0116	-.0093	-.0073	-.0064
6	-.030	-.0244	-.0116	-.0093	-.0073	-.0064
6	-.030	-.0244	-.0116	-.0093	-.0073	-.0064
7	-.013	-.0043	-.0043	-.0035	-.0027	-.0021
7	-.011	-.0069	-.0035	-.0031	-.0025	-.0019
7	-.010	-.0043	-.0036	-.0030	-.0023	-.0018
7	-.009	-.0046	-.0041	-.0027	-.0022	-.0017
7	-.008	-.0033	-.0029	-.0023	-.0020	-.0015
7	-.006	-.0048	-.0025	-.0018	-.0017	-.0015
7	-.006	-.0048	-.0025	-.0018	-.0017	-.0015
7	-.006	-.0048	-.0025	-.0018	-.0017	-.0015

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 2  
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000  
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 2  
 6 -.037 -.0136 -.0123 -.0111 -.0093 -.0053  
 6 -.036 -.0216 -.0112 -.0101 -.0090 -.0080  
 6 -.036 -.0169 -.0130 -.0102 -.0091 -.0081  
 6 -.036 -.0167 -.0155 -.0103 -.0091 -.0081  
 6 -.034 -.0143 -.0142 -.0104 -.0092 -.0082  
 6 -.035 -.0282 -.0144 -.0094 -.0083 -.0073  
 6 -.035 -.0282 -.0144 -.0094 -.0083 -.0073  
 6 -.035 -.0282 -.0144 -.0094 -.0083 -.0073  
 7 -.015 -.0056 -.0052 -.0047 -.0038 -.0022  
 7 -.013 -.0079 -.0043 -.0039 -.0035 -.0031  
 7 -.012 -.0055 -.0045 -.0035 -.0031 -.0028  
 7 -.011 -.0054 -.0048 -.0032 -.0030 -.0026  
 7 -.009 -.0036 -.0038 -.0028 -.0027 -.0024  
 7 -.007 -.0060 -.0033 -.0022 -.0021 -.0018  
 7 -.007 -.0060 -.0033 -.0022 -.0021 -.0018  
 7 -.007 -.0060 -.0033 -.0022 -.0021 -.0016

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 3  
 4 6.000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000  
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 3  
 6 -.040 -.0138 -.0125 -.0112 -.0100 -.0089  
 6 -.038 -.0201 -.0126 -.0102 -.0091 -.0081  
 6 -.039 -.0203 -.0130 -.0103 -.0092 -.0081  
 6 -.037 -.0205 -.0142 -.0104 -.0092 -.0082  
 6 -.035 -.0174 -.0158 -.0105 -.0083 -.0073  
 6 -.033 -.0305 -.0145 -.0094 -.0083 -.0074  
 6 -.033 -.0305 -.0145 -.0094 -.0083 -.0074  
 6 -.033 -.0305 -.0145 -.0094 -.0083 -.0074  
 7 -.015 -.0053 -.0048 -.0043 -.0040 -.0036  
 7 -.013 -.0066 -.0044 -.0035 -.0031 -.0028  
 7 -.011 -.0054 -.0062 -.0032 -.0028 -.0025  
 7 -.009 -.0051 -.0038 -.0028 -.0027 -.0024  
 7 -.007 -.0037 -.0037 -.0024 -.0021 -.0018  
 7 -.005 -.0054 -.0026 -.0018 -.0018 -.0016  
 7 -.005 -.0054 -.0026 -.0018 -.0018 -.0016  
 7 -.005 -.0054 -.0026 -.0018 -.0018 -.0016

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 4  
 4 5.800 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000  
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 4  
 6 -.052 -.0205 -.0172 -.0156 -.0142 -.0128  
 6 -.053 -.0282 -.0173 -.0158 -.0130 -.0117  
 6 -.050 -.0226 -.0175 -.0145 -.0131 -.0119  
 6 -.048 -.0247 -.0170 -.0146 -.0118 -.0095  
 6 -.045 -.0229 -.0211 -.0133 -.0119 -.0106  
 6 -.043 -.0378 -.0194 -.0133 -.0107 -.0085  
 6 -.043 -.0378 -.0194 -.0133 -.0107 -.0085  
 7 -.012 -.0051 -.0046 -.0045 -.0041 -.0039  
 7 -.010 -.0060 -.0040 -.0037 -.0032 -.0029  
 7 -.008 -.0040 -.0047 -.0028 -.0028 -.0025  
 7 -.006 -.0031 -.0025 -.0023 -.0021 -.0017  
 7 -.003 -.0020 -.0022 -.0016 -.0017 -.0017  
 7 -.002 -.0020 -.0014 -.0012 -.0011 -.0010  
 7 -.002 -.0020 -.0014 -.0012 -.0011 -.0010

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 5  
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000  
 5 5.500 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 5  
 6 -.065 -.0244 -.0225 -.0189 -.0173 -.0158  
 6 -.042 -.0373 -.0208 -.0192 -.0159 -.0131  
 6 -.059 -.0308 -.0210 -.0176 -.0160 -.0145  
 6 -.057 -.0248 -.0229 -.0177 -.0161 -.0146  
 6 -.057 -.0268 -.0248 -.0162 -.0147 -.0133  
 6 -.051 -.0454 -.0212 -.0162 -.0147 -.0133  
 6 -.051 -.0454 -.0212 -.0162 -.0147 -.0133  
 6 -.051 -.0454 -.0212 -.0162 -.0147 -.0133  
 7 -.012 -.0048 -.0048 -.0044 -.0040 -.0039  
 7 -.009 -.0060 -.0037 -.0034 -.0031 -.0026  
 7 -.005 -.0033 -.0043 -.0028 -.0026 -.0026  
 7 -.003 -.0018 -.0020 -.0019 -.0020 -.0021  
 7 -.001 -.0010 -.0013 -.0011 -.0013 -.0014  
 7 .001 0.0000 -.0004 -.0006 -.0008 -.0009  
 7 .001 0.0000 -.0004 -.0006 -.0008 -.0009  
 7 .001 0.0000 -.0004 -.0006 -.0008 -.0009

MGR FOLLOWED BY MGR ARRAY (IN FFT) AT MTIME# 6  
 4 6.500 6.5000 6.5000 6.5000 6.5000 6.5000 6.5000 6.5000  
 5 5.600 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT M#PTIME# 6

6	-.069	-.0248	-.0211	-.0193	-.0161	-.0146
6	-.063	-.0332	-.0211	-.0177	-.0162	-.0146
6	-.060	-.0298	-.0220	-.0162	-.0147	-.0133
6	-.054	-.0230	-.0230	-.0162	-.0133	-.0106
6	-.051	-.0229	-.0229	-.0147	-.0120	-.0096
6	-.045	-.0352	-.0194	-.0133	-.0120	-.0108
6	-.045	-.0352	-.0194	-.0133	-.0120	-.0108
6	-.045	-.0352	-.0194	-.0133	-.0120	-.0108
7	-.005	-.0022	-.0022	-.0024	-.0023	-.0023
7	-.001	-.0012	-.0011	-.0016	-.0017	-.0016
7	.002	0.0000	-.0006	-.0006	-.0010	-.0012
7	.004	.0006	.0004	0.0000	-.0005	-.0006
7	.005	.0020	.0012	.0005	.0002	0.0000
7	.006	.0043	.0017	.0009	.0006	.0004
7	.006	.0043	.0017	.0009	.0006	.0004
7	.006	.0043	.0017	.0009	.0006	.0004

MGR FOLLOWED BY MGR ARRAY (IN FFT) AT MTIME# 7  
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000  
 5 5.400 5.4000 5.4000 5.4000 5.4000 5.4000 5.4000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT M#PTIME# 7

6	-.065	-.0247	-.0211	-.0194	-.0178	-.0162
6	-.059	-.0307	-.0211	-.0177	-.0162	-.0147
6	-.056	-.0286	-.0210	-.0162	-.0147	-.0133
6	-.050	-.0207	-.0209	-.0146	-.0132	-.0114
6	-.044	-.0205	-.0207	-.0131	-.0119	-.0107
6	-.039	-.0276	-.0189	-.0117	-.0106	-.0095
6	-.039	-.0276	-.0189	-.0117	-.0106	-.0095
6	-.039	-.0276	-.0189	-.0117	-.0106	-.0095
7	.008	.0026	.0015	.0010	.0006	.0003
7	.010	.0043	.0022	.0015	.0011	.0008
7	.010	.0040	.0030	.0014	.0010	.0007
7	.011	.0044	.0037	.0023	.0018	.0015
7	.012	.0051	.0044	.0025	.0019	.0015
7	.012	.0079	.0047	.0027	.0020	.0017
7	.012	.0079	.0047	.0027	.0020	.0017
7	.012	.0079	.0047	.0027	.0020	.0017

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 8  
 4 6.000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000  
 5 5.300 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 8  
 6 -.048 -.0145 -.0156 -.0143 -.0130 -.0118  
 6 -.042 -.0216 -.0155 -.0124 -.0116 -.0105  
 6 -.039 -.0198 -.0150 -.0114 -.0103 -.0092  
 6 -.034 -.0150 -.0138 -.0114 -.0091 -.0072  
 6 -.030 -.0134 -.0136 -.0089 -.0080 -.0072  
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080  
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080  
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080  
 7 .017 .0040 .0045 .0034 .0036 .0025  
 7 .016 .0078 .0050 .0037 .0031 .0026  
 7 .017 .0076 .0040 .0037 .0031 .0026  
 7 .016 .0063 .0053 .0042 .0030 .0022  
 7 .015 .0042 .0056 .0036 .0029 .0023  
 7 .014 .0045 .0051 .0035 .0032 .0024  
 7 .014 .0045 .0051 .0035 .0032 .0024  
 7 .014 .0045 .0051 .0035 .0032 .0024

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 9  
 4 7.200 7.2000 7.2000 7.2000 7.2000 7.2000 7.2000 7.2000  
 5 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 9  
 6 -.060 -.0249 -.0216 -.0201 -.0186 -.0171  
 6 -.057 -.0303 -.0213 -.0183 -.0148 -.0155  
 6 -.050 -.0296 -.0240 -.0165 -.0152 -.0139  
 6 -.045 -.0222 -.0204 -.0148 -.0137 -.0126  
 6 -.037 -.0170 -.0184 -.0133 -.0122 -.0110  
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161  
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161  
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161  
 7 .027 .0100 .0078 .0065 .0057 .0049  
 7 .026 .0135 .0086 .0066 .0058 .0050  
 7 .027 .0150 .0090 .0067 .0054 .0050  
 7 .025 .0113 .0097 .0066 .0055 .0046  
 7 .022 .0094 .0096 .0065 .0054 .0049  
 7 .023 .0148 .0079 .0061 .0053 .0075  
 7 .023 .0148 .0079 .0061 .0053 .0075  
 7 .023 .0148 .0079 .0061 .0053 .0075

MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 10  
 4 7.000 7.0000 7.0000 7.0000 7.0000 7.0000 7.0000 7.0000  
 5 6.000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 10  
 6 -.048 -.0194 -.0174 -.0166 -.0160 -.0115  
 6 -.042 -.0224 -.0176 -.0137 -.0125 -.0113  
 6 -.037 -.0187 -.0150 -.0122 -.0112 -.0101  
 6 -.032 -.0170 -.0132 -.0109 -.0099 -.0090  
 6 -.028 -.0141 -.0157 -.0096 -.0087 -.0078  
 6 -.024 -.0165 -.0104 -.0085 -.0076 -.0068  
 6 -.024 -.0165 -.0104 -.0085 -.0076 -.0068  
 7 .024 .0086 .0076 .0064 .0048 .0035  
 7 .023 .0109 .0079 .0055 .0044 .0041  
 7 .021 .0099 .0080 .0054 .0045 .0037  
 7 .020 .0094 .0067 .0051 .0042 .0034  
 7 .018 .0081 .0083 .0049 .0041 .0035  
 7 .016 .0103 .0060 .0045 .0039 .0033  
 7 .016 .0103 .0060 .0045 .0039 .0033  
 7 .016 .0103 .0060 .0045 .0039 .0033

MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 11  
 4 5.800 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000  
 5 5.800 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 11  
 6 -.054 -.0213 -.0199 -.0169 -.0156 -.0143  
 6 -.046 -.0245 -.0196 -.0153 -.0140 -.0128  
 6 -.040 -.0225 -.0180 -.0138 -.0126 -.0114  
 6 -.037 -.0159 -.0147 -.0123 -.0112 -.0102  
 6 -.033 -.0157 -.0175 -.0110 -.0099 -.0089  
 6 -.028 -.0199 -.0130 -.0098 -.0088 -.0074  
 6 -.028 -.0199 -.0130 -.0098 -.0088 -.0074  
 7 .024 .0086 .0072 .0055 .0045 .0036  
 7 .022 .0109 .0074 .0055 .0045 .0037  
 7 .021 .0105 .0075 .0053 .0046 .0039  
 7 .021 .0096 .0069 .0052 .0043 .0035  
 7 .019 .0083 .0085 .0049 .0042 .0036  
 7 .018 .0115 .0069 .0045 .0039 .0033  
 7 .018 .0115 .0069 .0045 .0039 .0033  
 7 .018 .0115 .0069 .0045 .0039 .0033

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME# 12  
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000  
 5 5.300 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 12  
 6 -.040 -.0186 -.0148 -.0139 -.0129 -.0114  
 6 -.035 -.0169 -.0146 -.0124 -.0115 -.0105  
 6 -.029 -.0165 -.0130 -.0100 -.0091 -.0083  
 6 -.025 -.0136 -.0110 -.0087 -.0081 -.0074  
 6 -.021 -.0101 -.0115 -.0077 -.0070 -.0063  
 6 -.018 -.0121 -.0092 -.0059 -.0061 -.0055  
 6 -.018 -.0121 -.0092 -.0059 -.0061 -.0055  
 7 .031 .0135 .0094 .0084 .0071 .0061  
 7 .029 .0127 .0102 .0080 .0069 .0058  
 7 .025 .0133 .0095 .0067 .0057 .0048  
 7 .022 .0114 .0091 .0063 .0052 .0043  
 7 .020 .0088 .0093 .0058 .0049 .0041  
 7 .018 .0109 .0077 .0046 .0044 .0037  
 7 .018 .0109 .0077 .0046 .0044 .0037

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME# 13  
 4 6.600 6.6000 6.6000 6.6000 6.6000 6.7000 6.7000 6.7000  
 5 5.800 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 13  
 6 -.049 -.0255 -.0218 -.0211 -.0201 -.0191  
 6 -.042 -.0251 -.0194 -.0179 -.0169 -.0159  
 6 -.037 -.0247 -.0180 -.0174 -.0153 -.0133  
 6 -.033 -.0197 -.0179 -.0145 -.0126 -.0109  
 6 -.028 -.0154 -.0150 -.0108 -.0111 -.0102  
 6 -.024 -.0193 -.0135 -.0096 -.0079 -.0060  
 6 -.024 -.0193 -.0135 -.0096 -.0079 -.0060  
 6 -.024 -.0193 -.0135 -.0096 -.0079 -.0060  
 7 .053 .0240 .0189 .0165 .0146 .0128  
 7 .047 .0251 .0179 .0144 .0127 .0111  
 7 .043 .0255 .0170 .0151 .0120 .0093  
 7 .039 .0211 .0173 .0130 .0102 .0076  
 7 .033 .0171 .0150 .0101 .0097 .0086  
 7 .029 .0214 .0140 .0093 .0091 .0085  
 7 .029 .0214 .0140 .0093 .0091 .0085  
 7 .029 .0214 .0140 .0093 .0091 .0085

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 14  
 4 5.800 5.8000 5.8000 5.9000 6.0000 6.1000 6.2000 6.2000  
 5 5.600 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 14  
 6 -.041 -.0223 -.0188 -.0179 -.0171 -.0163  
 6 -.036 -.0235 -.0157 -.0151 -.0155 -.0146  
 6 -.032 -.0201 -.0150 -.0162 -.0142 -.0121  
 6 -.029 -.0196 -.0155 -.0134 -.0128 -.0121  
 6 -.025 -.0147 -.0152 -.0110 -.0115 -.0097  
 6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105  
 6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105  
 7 .044 .0215 .0163 .0144 .0124 .0106  
 7 .040 .0234 .0142 .0122 .0117 .0102  
 7 .036 .0207 .0140 .0136 .0107 .0085  
 7 .032 .0193 .0144 .0117 .0100 .0085  
 7 .029 .0152 .0147 .0099 .0093 .0071  
 7 .025 .0196 .0126 .0089 .0086 .0082  
 7 .025 .0196 .0126 .0089 .0086 .0082

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 15  
 4 4.300 4.3000 4.3000 4.5000 4.7000 4.9000 5.1000 5.1000  
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 15  
 6 -.040 -.0222 -.0190 -.0185 -.0191 -.0181  
 6 -.036 -.0264 -.0173 -.0168 -.0176 -.0169  
 6 -.031 -.0233 -.0170 -.0182 -.0160 -.0138  
 6 -.030 -.0203 -.0173 -.0152 -.0147 -.0140  
 6 -.029 -.0163 -.0170 -.0128 -.0132 -.0113  
 6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102  
 6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102  
 7 .049 .0246 .0190 .0166 .0160 .0141  
 7 .044 .0304 .0179 .0157 .0149 .0127  
 7 .040 .0267 .0180 .0169 .0139 .0112  
 7 .037 .0234 .0179 .0147 .0127 .0110  
 7 .035 .0187 .0182 .0124 .0119 .0095  
 7 .032 .0250 .0155 .0115 .0111 .0086  
 7 .032 .0250 .0155 .0115 .0111 .0086  
 7 .032 .0250 .0155 .0115 .0111 .0086



MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 16  
 4 4.600 4.6000 4.6000 4.7000 4.9000 5.0000 5.1000 5.1000  
 5 5.100 5.1000 5.1000 5.1000 5.1000 5.1000 5.1000 5.1000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MTIME# 16  
 6 -.027 -.0156 -.0149 -.0162 -.0174 -.0160  
 6 -.026 -.0201 -.0138 -.0150 -.0150 -.0160  
 6 -.025 -.0184 -.0140 -.0150 -.0138 -.0125  
 6 -.023 -.0161 -.0153 -.0139 -.0128 -.0115  
 6 -.021 -.0150 -.0142 -.0109 -.0117 -.0125  
 6 -.019 -.0155 -.0132 -.0100 -.0110 -.0099  
 6 -.019 -.0155 -.0132 -.0100 -.0110 -.0099  
 6 -.019 -.0155 -.0132 -.0100 -.0110 -.0099  
 7 .057 .0293 .0247 .0239 .0230 .0190  
 7 .054 .0378 .0230 .0222 .0198 .0190  
 7 .051 .0331 .0240 .0222 .0183 .0149  
 7 .045 .0290 .0245 .0206 .0169 .0136  
 7 .040 .0271 .0227 .0161 .0155 .0149  
 7 .038 .0268 .0211 .0148 .0140 .0110  
 7 .038 .0268 .0211 .0148 .0140 .0110  
 7 .038 .0268 .0211 .0148 .0140 .0110

MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 17  
 4 5.200 5.4000 5.4000 5.6000 5.8000 6.0000 6.0000 6.0000  
 5 5.300 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MTIME# 17  
 6 -.015 -.0139 -.0149 -.0138 -.0124 -.0114  
 6 -.015 -.0125 -.0089 -.0111 -.0121 -.0131  
 6 -.013 -.0118 -.0089 -.0111 -.0118 -.0122  
 6 -.014 -.0111 -.0108 -.0104 -.0101 -.0096  
 6 -.013 -.0097 -.0101 -.0086 -.0094 -.0093  
 6 -.013 -.0103 -.0092 -.0086 -.0086 -.0085  
 6 -.013 -.0103 -.0092 -.0086 -.0086 -.0085  
 7 .071 .0518 .0486 .0374 .0308 .0243  
 7 .064 .0464 .0274 .0289 .0285 .0281  
 7 .058 .0411 .0280 .0289 .0264 .0239  
 7 .055 .0362 .0314 .0269 .0227 .0189  
 7 .049 .0317 .0293 .0213 .0210 .0190  
 7 .044 .0316 .0252 .0213 .0193 .0175  
 7 .044 .0316 .0252 .0213 .0193 .0175  
 7 .044 .0316 .0252 .0213 .0193 .0175

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 18

4	4.700	4.7000	4.7000	4.9000	5.1000	5.3000	5.4000	5.4000
5	5.000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 18

6	.002	.0000	-.0007	-.0020	-.0028	-.0040	
6	.001	-.0006	-.0011	-.0022	-.0030	-.0037	
6	0.000	-.0011	-.0028	-.0024	-.0031	-.0031	
6	-.001	-.0014	-.0019	-.0024	-.0028	-.0026	
6	-.001	-.0015	-.0022	-.0023	-.0028	-.0026	
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025	
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025	
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025	
7	.048	.0249	.0212	.0229	.0228	.0226	
7	.045	.0332	.0211	.0211	.0210	.0208	
7	.040	.0310	.0210	.0193	.0192	.0175	
7	.038	.0268	.0211	.0193	.0176	.0180	
7	.033	.0211	.0211	.0161	.0160	.0131	
7	.031	.0229	.0193	.0146	.0145	.0116	
7	.031	.0229	.0193	.0146	.0145	.0116	
7	.031	.0229	.0193	.0146	.0145	.0116	

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 19

4	3.200	3.2000	3.2000	3.5000	3.8000	4.0000	4.1000	4.1000
5	4.400	4.4000	4.4000	4.4000	4.4000	4.4000	4.4000	

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 19

6	.016	.0075	.0050	.0050	.0035	.0025	
6	.014	.0096	.0040	.0037	.0028	.0019	
6	.012	.0072	.0050	.0030	.0022	.0013	
6	.011	.0061	.0043	.0030	.0017	.0007	
6	.009	.0048	.0035	.0020	.0012	.0004	
6	.008	.0046	.0032	.0015	.0011	.0007	
6	.008	.0046	.0032	.0015	.0011	.0007	
6	.008	.0046	.0032	.0015	.0011	.0007	
7	.097	.0301	.0282	.0284	.0286	.0288	
7	.055	.0417	.0284	.0266	.0267	.0268	
7	.052	.0371	.0280	.0247	.0248	.0248	
7	.050	.0349	.0307	.0247	.0248	.0212	
7	.047	.0306	.0280	.0229	.0229	.0230	
7	.045	.0329	.0306	.0211	.0211	.0212	
7	.045	.0329	.0306	.0211	.0211	.0212	
7	.045	.0329	.0306	.0211	.0211	.0212	

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 20  
 4 2.700 2.7000 2.7000 2.9000 3.2000 3.4000 3.6000 3.6000  
 5 4.000 4.0000 4.0000 4.0000 4.0000 4.0000 4.0000

XR VALUES (ICNT#6) AND YR VALUES (ICNT#7) AT MPTIME# 20

6	.018	.0086	.0073	.0076	.0075	.0073
6	.015	.0113	.0064	.0063	.0062	.0059
6	.014	.0082	.0060	.0055	.0054	.0051
6	.012	.0078	.0065	.0054	.0046	.0039
6	.011	.0060	.0057	.0042	.0036	.0028
6	.009	.0062	.0049	.0032	.0033	.0034
6	.009	.0062	.0049	.0032	.0033	.0034
6	.009	.0062	.0049	.0032	.0033	.0034
7	.036	.0193	.0140	.0108	.0217	.0236
7	.035	.0266	.0166	.0144	.0203	.0222
7	.033	.0215	.0145	.0169	.0187	.0205
7	.031	.0216	.0201	.0147	.0172	.0154
7	.029	.0185	.0146	.0157	.0154	.0145
7	.027	.0243	.0171	.0130	.0144	.0159
7	.027	.0203	.0171	.0130	.0144	.0159
7	.027	.0203	.0171	.0130	.0144	.0159

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 21  
 4 3.700 3.7000 3.7000 3.9000 3.9000 4.0000 4.1000 4.1000  
 5 4.600 4.6000 4.6000 4.6000 4.6000 4.6000 4.6000

XR VALUES (ICNT#6) AND YR VALUES (ICNT#7) AT MPTIME# 21

6	.016	.0074	.0062	.0075	.0070	.0063
6	.013	.0093	.0062	.0043	.0067	.0070
6	.012	.0072	.0060	.0053	.0055	.0058
6	.011	.0070	.0044	.0061	.0044	.0037
6	.010	.0053	.0053	.0043	.0039	.0031
6	.008	.0061	.0043	.0037	.0035	.0030
6	.008	.0061	.0043	.0037	.0035	.0030
6	.008	.0061	.0043	.0037	.0035	.0030
7	.024	.0145	.0134	.0161	.0164	.0195
7	.026	.0190	.0134	.0156	.0165	.0142
7	.026	.0163	.0150	.0134	.0153	.0168
7	.025	.0164	.0166	.0167	.0140	.0114
7	.023	.0136	.0138	.0126	.0124	.0116
7	.022	.0167	.0126	.0114	.0115	.0104
7	.022	.0167	.0126	.0114	.0115	.0104
7	.022	.0167	.0126	.0114	.0115	.0104

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 22  
 4 3.500 3.5000 3.5000 3.7000 3.9000 4.0000 4.1000 4.1000  
 5 4.400 4.4000 4.4000 4.4000 4.4000 4.4000 4.4000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 22

6	.013	.0060	.0058	.0063	.0067	.0071
6	.012	.0079	.0058	.0061	.0058	.0062
6	.010	.0067	.0060	.0051	.0054	.0058
6	.010	.0065	.0069	.0060	.0047	.0030
6	.008	.0049	.0049	.0040	.0039	.0033
6	.007	.0058	.0040	.0034	.0037	.0039
6	.007	.0058	.0040	.0034	.0037	.0039
6	.007	.0058	.0040	.0034	.0037	.0039
7	.021	.0104	.0105	.0118	.0132	.0146
7	.020	.0142	.0146	.0119	.0120	.0134
7	.019	.0132	.0120	.0109	.0122	.0130
7	.019	.0133	.0147	.0135	.0111	.0089
7	.018	.0110	.0110	.0100	.0101	.0090
7	.016	.0130	.0100	.0090	.0101	.0114
7	.016	.0130	.0100	.0090	.0101	.0114
7	.016	.0130	.0100	.0090	.0101	.0114

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME# 23  
 4 2.200 2.2000 2.2000 2.4000 2.6000 2.8000 2.9000 2.9000  
 5 3.500 3.5000 3.5000 3.5000 3.5000 3.5000 3.5000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 23

6	.009	.0041	.0040	.0052	.0055	.0058
6	.008	.0052	.0044	.0049	.0048	.0047
6	.007	.0049	.0050	.0043	.0047	.0051
6	.007	.0047	.0050	.0045	.0044	.0033
6	.006	.0035	.0039	.0033	.0036	.0039
6	.006	.0044	.0032	.0032	.0031	.0029
6	.006	.0044	.0032	.0032	.0031	.0029
6	.006	.0044	.0032	.0032	.0031	.0029
7	.013	.0063	.0072	.0091	.0002	.0105
7	.012	.0081	.0073	.0082	.0083	.0084
7	.011	.0082	.0090	.0074	.0084	.0095
7	.013	.0084	.0105	.0085	.0086	.0088
7	.011	.0080	.0076	.0088	.0077	.0088
7	.011	.0086	.0068	.0068	.0069	.0069
7	.011	.0086	.0068	.0068	.0069	.0069
7	.011	.0086	.0068	.0068	.0069	.0069

MGP FOLLOWED BY MGR ARRAY (21 FEET) AT MTIME= 24  
 4 1.300 1.3000 1.3000 1.5000 1.7000 1.9000 2.1000  
 5 3.500 3.5000 3.5000 3.5000 3.5000 3.5000 3.5000

XR VALUES (IDNT=6) AND YH VALUES (IDNT=7) AT MPTIME= 24

6	.000	.0030	.0035	.0040	.0040	.0049
6	.006	.0034	.0033	.0038	.0042	.0040
6	.005	.0037	.0037	.0032	.0036	.0040
6	.005	.0031	.0045	.0030	.0034	.0038
6	.004	.0026	.0030	.0029	.0033	.0038
6	.004	.0033	.0024	.0024	.0027	.0027
6	.004	.0033	.0024	.0024	.0027	.0027
6	.004	.0033	.0024	.0024	.0027	.0027
7	.008	.0040	.0040	.0053	.0061	.0070
7	.008	.0046	.0047	.0054	.0062	.0072
7	.007	.0055	.0055	.0044	.0055	.0064
7	.007	.0048	.0072	.0049	.0056	.0065
7	.006	.0042	.0049	.0050	.0057	.0065
7	.006	.0057	.0043	.0043	.0051	.0051
7	.006	.0057	.0043	.0043	.0051	.0051
7	.006	.0057	.0043	.0043	.0051	.0051

THE FOLLOWING ARE SURGRID CHANNEL DATA - Z VALUES IN FEET

K#	1	ICG#	A	JCG#	1	IwCX#	-2330	IzCY#	-20	IwCY#	0	IzCY#	0	IFC#	15
K#	2	ICG#	A	JCG#	2	IwCX#	-2330	IzCY#	-20	IwCY#	0	IzCY#	0	IFC#	15
K#	3	ICG#	A	JCG#	3	IwCX#	2860	IzCY#	-21	IwCY#	2860	IzCY#	-21	IFC#	15
K#	4	ICG#	7	JCG#	3	IwCX#	0	IzCY#	0	IwCY#	0	IzCY#	0	IFC#	15
K#	5	ICG#	7	JCG#	4	IwCX#	2860	IzCY#	-21	IwCY#	1000	IzCY#	26	IFC#	15
K#	6	ICG#	6	JCG#	4	IwCX#	0	IzCY#	0	IwCY#	0	IzCY#	0	IFC#	15
K#	7	ICG#	6	JCG#	5	IwCX#	1000	IzCY#	26	IwCY#	0	IzCY#	0	IFC#	15
K#	8	ICG#	6	JCG#	6	IwCX#	900	IzCY#	26	IwCY#	300	IzCY#	12	IFC#	20
K#	9	ICG#	6	JCG#	7	IwCX#	-900	IzCY#	-21	IwCY#	-300	IzCY#	-15	IFC#	20
K#	10	ICG#	7	JCG#	7	IwCX#	0	IzCY#	0	IwCY#	-900	IzCY#	-26	IFC#	20
K#	11	ICG#	A	JCG#	7	IwCX#	0	IzCY#	0	IwCY#	-900	IzCY#	-26	IFC#	20
K#	12	ICG#	8	JCG#	A	IwCX#	-900	IzCY#	-26	IwCY#	0	IzCY#	0	IFC#	20
K#	13	ICG#	9	JCG#	A	IwCX#	0	IzCY#	0	IwCY#	-900	IzCY#	-26	IFC#	20
K#	14	ICG#	9	JCG#	9	IwCX#	-900	IzCY#	-26	IwCY#	0	IzCY#	0	IFC#	20
K#	15	ICG#	9	JCG#	10	IwCX#	900	IzCY#	-26	IwCY#	0	IzCY#	0	IFC#	20
K#	16	ICG#	9	JCG#	11	IwCX#	400	IzCY#	-35	IwCY#	400	IzCY#	-35	IFC#	25
K#	17	ICG#	8	JCG#	11	IwCX#	0	IzCY#	0	IwCY#	400	IzCY#	-35	IFC#	25
K#	18	ICG#	7	JCG#	11	IwCX#	0	IzCY#	0	IwCY#	0	IzCY#	0	IFC#	25
K#	19	ICG#	7	JCG#	12	IwCX#	-400	IzCY#	-35	IwCY#	-400	IzCY#	-35	IFC#	25
K#	20	ICG#	6	JCG#	12	IwCX#	0	IzCY#	0	IwCY#	350	IzCY#	43	IFC#	25
K#	21	ICG#	5	JCG#	12	IwCX#	0	IzCY#	0	IwCY#	0	IzCY#	0	IFC#	25
K#	22	ICG#	5	JCG#	13	IwCX#	350	IzCY#	43	IwCY#	0	IzCY#	0	IFC#	25
K#	23	ICG#	5	JCG#	14	IwCX#	350	IzCY#	-43	IwCY#	350	IzCY#	-43	IFC#	25
K#	24	ICG#	4	JCG#	14	IwCX#	0	IzCY#	0	IwCY#	0	IzCY#	0	IFC#	25
K#	25	ICG#	4	JCG#	15	IwCX#	300	IzCY#	-25	IwCY#	0	IzCY#	0	IFC#	25
K#	26	ICG#	5	JCG#	15	IwCX#	0	IzCY#	0	IwCY#	400	IzCY#	-40	IFC#	25
K#	27	ICG#	5	JCG#	16	IwCX#	-400	IzCY#	-40	IwCY#	-400	IzCY#	-30	IFC#	25
K#	28	ICG#	4	JCG#	16	IwCX#	0	IzCY#	0	IwCY#	0	IzCY#	0	IFC#	25
K#	29	ICG#	4	JCG#	17	IwCX#	400	IzCY#	-30	IwCY#	150	IzCY#	-20	IFC#	25
K#	30	ICG#	5	JCG#	17	IwCX#	0	IzCY#	0	IwCY#	300	IzCY#	-30	IFC#	25

Kz 31	ICGz	5	JCGz	1A	IwCXz	400	IzCYz	-30	IwCYz	300	IzCYz	-30	IFCz	25
Kz 32	ICGz	4	JCGz	1A	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	25
Kz 33	ICGz	4	JCGz	1A	IwCXz	300	IzCYz	-30	IwCYz	0	IzCYz	0	IFCz	25
Kz 34	ICGz	3	JCGz	17	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	25
Kz 35	ICGz	3	JCGz	18	IwCXz	150	IzCYz	-20	IwCYz	100	IzCYz	-20	IFCz	25
Kz 36	ICGz	2	JCGz	1A	IwCXz	0	IzCYz	0	IwCYz	100	IzCYz	-20	IFCz	25
Kz 37	ICGz	10	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	200	IzCYz	-12	IFCz	25
Kz 3A	ICGz	11	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	200	IzCYz	-12	IFCz	25
Kz 39	ICGz	12	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	200	IzCYz	-12	IFCz	25
Kz 40	ICGz	12	JCGz	11	IwCXz	200	IzCYz	-27	IwCYz	200	IzCYz	-20	IFCz	25
Kz 41	ICGz	13	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	200	IzCYz	-27	IFCz	25
Kz 42	ICGz	14	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	200	IzCYz	-27	IFCz	25
Kz 43	ICGz	14	JCGz	12	IwCXz	200	IzCYz	-27	IwCYz	-200	IzCYz	-27	IFCz	25
Kz 44	ICGz	13	JCGz	12	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	25
Kz 45	ICGz	13	JCGz	13	IwCXz	200	IzCYz	27	IwCYz	0	IzCYz	0	IFCz	25
Kz 46	ICGz	14	JCGz	13	IwCXz	0	IzCYz	0	IwCYz	200	IzCYz	27	IFCz	25
Kz 47	ICGz	14	JCGz	14	IwCXz	500	IzCYz	-20	IwCYz	0	IzCYz	0	IFCz	25
Kz 4A	ICGz	15	JCGz	14	IwCXz	0	IzCYz	0	IwCYz	350	IzCYz	-20	IFCz	25
Kz 49	ICGz	15	JCGz	15	IwCXz	350	IzCYz	-20	IwCYz	200	IzCYz	-20	IFCz	25
Kz 50	ICGz	14	JCGz	15	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	25
Kz 51	ICGz	14	JCGz	16	IwCXz	200	IzCYz	-20	IwCYz	0	IzCYz	0	IFCz	25
Kz 52	ICGz	14	JCGz	17	IwCXz	200	IzCYz	-15	IwCYz	0	IzCYz	0	IFCz	25
Kz 53	ICGz	14	JCGz	1A	IwCXz	100	IzCYz	-10	IwCYz	0	IzCYz	0	IFCz	25
Kz 54	ICGz	14	JCGz	19	IwCXz	100	IzCYz	-10	IwCYz	0	IzCYz	0	IFCz	25
Kz 55	ICGz	11	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	9
Kz 56	ICGz	11	JCGz	12	IwCXz	200	IzCYz	-20	IwCYz	200	IzCYz	-20	IFCz	9
Kz 57	ICGz	10	JCGz	12	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	9
Kz 5A	ICGz	10	JCGz	13	IwCXz	200	IzCYz	-20	IwCYz	100	IzCYz	-20	IFCz	9
Kz 59	ICGz	9	JCGz	13	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	9
Kz 60	ICGz	9	JCGz	14	IwCXz	100	IzCYz	-20	IwCYz	0	IzCYz	0	IFCz	9
Kz 61	ICGz	15	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	300	IzCYz	-12	IFCz	9
Kz 62	ICGz	16	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	300	IzCYz	-12	IFCz	9
Kz 63	ICGz	17	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	-300	IzCYz	-12	IFCz	9
Kz 64	ICGz	18	JCGz	11	IwCXz	0	IzCYz	0	IwCYz	-300	IzCYz	-12	IFCz	9
Kz 65	ICGz	19	JCGz	11	IwCXz	300	IzCYz	-12	IwCYz	-300	IzCYz	-12	IFCz	9
Kz 66	ICGz	19	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	0	IzCYz	0	IFCz	9
Kz 67	ICGz	20	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	300	IzCYz	-12	IFCz	9
Kz 6A	ICGz	21	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	300	IzCYz	-12	IFCz	9
Kz 69	ICGz	22	JCGz	10	IwCXz	0	IzCYz	0	IwCYz	300	IzCYz	-12	IFCz	9
Kz 70	ICGz	23	JCGz	10	IwCXz	-400	IzCYz	-40	IwCYz	300	IzCYz	-12	IFCz	9
Kz 71	ICGz	22	JCGz	1	IwCXz	-800	IzCYz	-32	IwCYz	0	IzCYz	0	IFCz	9
Kz 72	ICGz	23	JCGz	1	IwCXz	0	IzCYz	0	IwCYz	1000	IzCYz	-16	IFCz	15
Kz 73	ICGz	23	JCGz	2	IwCXz	1000	IzCYz	-16	IwCYz	1000	IzCYz	-16	IFCz	15
Kz 74	ICGz	22	JCGz	2	IwCXz	500	IzCYz	-40	IwCYz	0	IzCYz	0	IFCz	15
Kz 75	ICGz	22	JCGz	3	IwCXz	800	IzCYz	-32	IwCYz	0	IzCYz	0	IFCz	15
Kz 76	ICGz	22	JCGz	4	IwCXz	1000	IzCYz	-20	IwCYz	0	IzCYz	0	IFCz	15
Kz 77	ICGz	22	JCGz	5	IwCXz	1000	IzCYz	-20	IwCYz	0	IzCYz	0	IFCz	5
Kz 78	ICGz	23	JCGz	5	IwCXz	0	IzCYz	0	IwCYz	400	IzCYz	-40	IFCz	5
Kz 79	ICGz	23	JCGz	6	IwCXz	400	IzCYz	40	IwCYz	0	IzCYz	0	IFCz	5
Kz 80	ICGz	23	JCGz	7	IwCXz	400	IzCYz	40	IwCYz	0	IzCYz	0	IFCz	5
Kz 81	ICGz	23	JCGz	8	IwCXz	400	IzCYz	40	IwCYz	0	IzCYz	0	IFCz	5
Kz 82	ICGz	23	JCGz	9	IwCXz	400	IzCYz	40	IwCYz	0	IzCYz	0	IFCz	5
Kz 83	ICGz	24	JCGz	9	IwCXz	0	IzCYz	0	IwCYz	800	IzCYz	-25	IFCz	5
Kz 84	ICGz	24	JCGz	10	IwCXz	800	IzCYz	-25	IwCYz	1000	IzCYz	-20	IFCz	5

K# 85	ICG# 24	JCG# 11	IwCX# 800	IZCX# -25	IwCY# 0	IZCY# 0	IFC# 5
K# 86	ICG# 24	JCG# 12	IwCX# 900	IZCX# -20	IwCY# 0	IZCY# 0	IFC# 5
K# 87	ICG# 24	JCG# 13	IwCX# 1000	IZCX# -25	IwCY# 0	IZCY# 0	IFC# 5
K# 88	ICG# 25	JCG# 13	IwCX# 0	IZCX# 0	IwCY# 1000	IZCY# -25	IFC# 5
K# 89	ICG# 25	JCG# 14	IwCX# 400	IZCX# -40	IwCY# 0	IZCY# 0	IFC# 5
K# 90	ICG# 26	JCG# 14	IwCX# 0	IZCX# 0	IwCY# 800	IZCY# -20	IFC# 5
K# 91	ICG# 27	JCG# 14	IwCX# 0	IZCX# 0	IwCY# 800	IZCY# -20	IFC# 5
K# 92	ICG# 27	JCG# 15	IwCX# 800	IZCX# -20	IwCY# 0	IZCY# 0	IFC# 5
K# 93	ICG# 28	JCG# 15	IwCX# 0	IZCX# 0	IwCY# 800	IZCY# -20	IFC# 5
K# 94	ICG# 25	JCG# 10	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 95	ICG# 26	JCG# 10	IwCX# 300	IZCX# -12	IwCY# 300	IZCY# -12	IFC# 9
K# 96	ICG# 26	JCG# 9	IwCX# 0	IZCX# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 97	ICG# 27	JCG# 9	IwCX# 300	IZCX# -12	IwCY# 300	IZCY# -12	IFC# 9
K# 98	ICG# 27	JCG# 8	IwCX# 0	IZCX# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 99	ICG# 28	JCG# 8	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 100	ICG# 1	JCG# 4	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 101	ICG# 2	JCG# 4	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 102	ICG# 3	JCG# 4	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 103	ICG# 3	JCG# 5	IwCX# 300	IZCX# 12	IwCY# 0	IZCY# 0	IFC# 9
K# 104	ICG# 4	JCG# 5	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 105	ICG# 5	JCG# 5	IwCX# 0	IZCX# 0	IwCY# 300	IZCY# 12	IFC# 9
K# 106	ICG# 5	JCG# 6	IwCX# 300	IZCX# -12	IwCY# 0	IZCY# 0	IFC# 9
K# 107	ICG# 5	JCG# 7	IwCX# 0	IZCX# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 108	ICG# 5	JCG# 8	IwCX# 400	IZCX# 15	IwCY# 400	IZCY# 15	IFC# 9
K# 109	ICG# 4	JCG# 9	IwCX# 0	IZCX# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 110	ICG# 4	JCG# 9	IwCX# 200	IZCX# -12	IwCY# 200	IZCY# -10	IFC# 9
K# 111	ICG# 3	JCG# 9	IwCX# 200	IZCX# 10	IwCY# 0	IZCY# 0	IFC# 9
K# 112	ICG# 3	JCG# 8	IwCX# 0	IZCX# 0	IwCY# 200	IZCY# -10	IFC# 9
K# 113	ICG# 2	JCG# 8	IwCX# 0	IZCX# 0	IwCY# 200	IZCY# -10	IFC# 9
K# 114	ICG# 1	JCG# 8	IwCX# 0	IZCX# 0	IwCY# 200	IZCY# -10	IFC# 9
K# 115	ICG# 3	JCG# 10	IwCX# 200	IZCX# -10	IwCY# 200	IZCY# -10	IFC# 9
K# 116	ICG# 2	JCG# 10	IwCX# 0	IZCX# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 117	ICG# 2	JCG# 11	IwCX# 200	IZCX# -8	IwCY# 0	IZCY# 0	IFC# 9
K# 118	ICG# 2	JCG# 12	IwCX# 200	IZCX# -8	IwCY# 0	IZCY# 0	IFC# 9
K# 119	ICG# 6	JCG# 8	IwCX# -400	IZCX# -20	IwCY# 0	IZCY# 0	IFC# 9
K# 120	ICG# 7	JCG# 8	IwCX# 0	IZCX# 0	IwCY# 200	IZCY# -20	IFC# 9
K# 121	ICG# 7	JCG# 5	IwCX# 3000	IZCX# -12	IwCY# 0	IZCY# 0	IFC# 9

#### HYDROGRAPH GAGE LOCATIONS

GAGE 1 BLOCK M, I# 8 J# 1  
 GAGE 2 CHANNEL M, K# 11  
 GAGE 3 BLOCK M, I# 11 J# 10  
 GAGE 4 CHANNEL M, K# 25  
 GAGE 5 CHANNEL M, K# 46  
 GAGE 6 CHANNEL M, K# 72  
 GAGE 7 CHANNEL M, K# 100  
 GAGE 8 CHANNEL M, K# 3  
 GAGE 9 CHANNEL M, K# 92

#### KEY FLOW LOCATIONS

CHANNEL BLOCKS 1 71

**APPENDIX E**

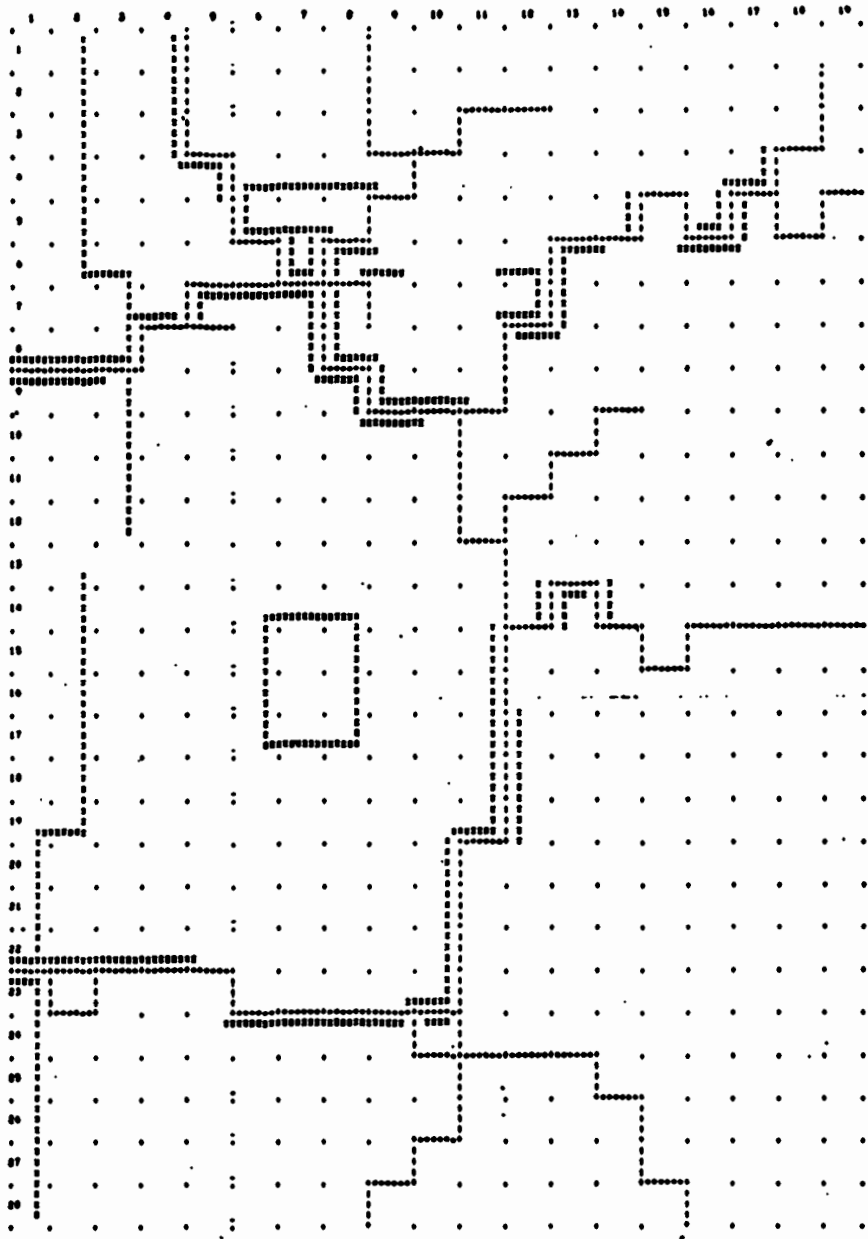
**LISTING OF KEY ARRAYS AND CHANNEL AND BARRIER PLOT  
FOR SABINE-CALCASIEU REGION**





K# 64	KCX#128	KCY# 63	KCX#12A	KCY# 65	KCB# 81	ICG# 18	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0	0
K# 65	KCX# 66	KCY# 64	KCX#12A	KCY#12A	KCB# 82	ICG# 19	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0	0
K# 66	KCX#128	KCY#128	KCX# 65	KCY# 67	KCB# 0	ICG# 19	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 67	KCX#12A	KCY# 66	KCX#12A	KCY# 68	KCB# 74	ICG# 20	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 68	KCX#128	KCY# 67	KCX#12A	KCY# 69	KCB# 75	ICG# 21	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 69	KCX#128	KCY# 68	KCX#12A	KCY# 70	KCB# 76	ICG# 22	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 70	KCX# 82	KCY# 69	KCX#128	KCY# 71	KCB# 77	ICG# 23	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 71	KCX#124	KCY#128	KCX# 74	KCY# 72	KCB# 4	ICG#22	JCG# 1	KEN1# 1	KEN2# 0	KRI# 0	0
K# 72	KCX#128	KCY# 71	KCX# 73	KCY#12A	KCB# 5	ICG# 23	JCG# 1	KEN1# 0	KEN2# 0	KRI# 0	0
K# 73	KCX# 72	KCY# 74	KCX#128	KCY#128	KCB# 0	ICG# 23	JCG# 2	KEN1# 0	KEN2# 0	KRI# 0	0
K# 74	KCX# 71	KCY#128	KCX# 75	KCY# 73	KCB# 25	ICG# 22	JCG# 2	KEN1# 0	KEN2# 0	KRI# 0	0
K# 75	KCX# 74	KCY#128	KCX# 76	KCY#128	KCB# 33	ICG# 22	JCG# 3	KEN1# 0	KEN2# 0	KRI# 0	0
K# 76	KCX# 75	KCY#128	KCX# 77	KCY#12A	KCB# 38	ICG# 22	JCG# 4	KEN1# 0	KEN2# 0	KRI# 0	0
K# 77	KCX# 76	KCY#128	KCX#12A	KCY# 78	KCB# 0	ICG# 22	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0	0
K# 78	KCX#128	KCY# 77	KCX# 79	KCY#128	KCB# 0	ICG# 23	JCG# 5	KEN1# 5	KEN2# 0	KRI# 0	0
K# 79	KCX# 79	KCY#128	KCX# 80	KCY#12A	KCB# 49	ICG# 23	JCG# 6	KEN1# 0	KEN2# 0	KRI# 0	0
K# 80	KCX# 78	KCY#128	KCX# 81	KCY#12A	KCB# 57	ICG# 23	JCG# 7	KEN1# 0	KEN2# 0	KRI# 0	0
K# 81	KCX# 80	KCY#128	KCX# 82	KCY#12A	KCB# 68	ICG# 23	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K# 82	KCX# 81	KCY#128	KCX# 70	KCY# 83	KCB# 72	ICG# 23	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0	0
K# 83	KCX#128	KCY# 82	KCX# 84	KCY#128	KCB# 0	ICG# 24	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0	0
K# 84	KCX# 83	KCY# 70	KCX# 85	KCY# 94	KCB# 0	ICG# 24	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 85	KCX# 84	KCY#128	KCX# 86	KCY#12A	KCB# 0	ICG# 24	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0	0
K# 86	KCX# 85	KCY#128	KCX# 87	KCY#12A	KCB# 0	ICG# 24	JCG# 12	KEN1# 0	KEN2# 0	KRI# 0	0
K# 87	KCX# 86	KCY#128	KCX#12A	KCY# 88	KCB# 0	ICG# 24	JCG# 13	KEN1# 0	KEN2# 0	KRI# 0	0
K# 88	KCX#128	KCY# 87	KCX# 89	KCY#12A	KCB# 0	ICG# 25	JCG# 13	KEN1# 0	KEN2# 0	KRI# 0	0
K# 89	KCX# 88	KCY#128	KCX#12A	KCY# 90	KCB# 0	ICG# 25	JCG# 14	KEN1# 0	KEN2# 0	KRI# 0	0
K# 90	KCX#128	KCY# 89	KCX#12A	KCY# 91	KCB# 0	ICG# 26	JCG# 14	KEN1# 0	KEN2# 0	KRI# 0	0
K# 91	KCX#128	KCY# 90	KCX# 92	KCY#128	KCB# 0	ICG# 27	JCG# 14	KEN1# 0	KEN2# 0	KRI# 0	0
K# 92	KCX# 91	KCY#128	KCX#128	KCY# 93	KCB# 0	ICG# 27	JCG# 15	KEN1# 0	KEN2# 0	KRI# 0	0
K# 93	KCX#128	KCY# 92	KCX#128	KCY#12A	KCB# 0	ICG#28	JCG# 15	KEN1# 8	KEN2# 0	KRI# 1	0
K# 94	KCX#128	KCY# 94	KCX#128	KCY# 95	KCB# 0	ICG# 25	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 95	KCX# 96	KCY# 94	KCX#12A	KCY#128	KCB# 0	ICG# 26	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K# 96	KCX#128	KCY#128	KCX# 95	KCY# 97	KCB# 0	ICG# 26	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0	0
K# 97	KCX# 98	KCY# 96	KCX#12A	KCY#128	KCB# 0	ICG# 27	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0	0
K# 98	KCX#128	KCY#128	KCX# 97	KCY# 99	KCB# 0	ICG# 27	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K# 99	KCX#128	KCY# 98	KCX#12A	KCY#128	KCB# 0	ICG#28	JCG# 8	KEN1# 4	KEN2# 0	KRI# 0	0
K#100	KCX#128	KCY#125	KCX#12A	KCY#101	KCB# 34	ICG# -1	JCG# 4	KEN1# 6	KEN2# 0	KRI# 0	0
K#101	KCX#128	KCY#100	KCX#128	KCY#102	KCB# 35	ICG# 2	JCG# 4	KEN1# 0	KEN2# 0	KRI# 0	0
K#102	KCX#128	KCY#101	KCX#103	KCY#128	KCB# 36	ICG# 3	JCG# 4	KEN1# 0	KEN2# 0	KRI# 0	0
K#103	KCX#102	KCY#128	KCX#12A	KCY#104	KCB# 39	ICG# 3	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0	0
K#104	KCX#128	KCY#103	KCX#128	KCY#105	KCB# 40	ICG# 4	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0	0
K#105	KCX#128	KCY#104	KCX#106	KCY# 7	KCB# 41	ICG# 5	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0	0
K#106	KCX#105	KCY#128	KCX#107	KCY# 8	KCB# 44	ICG# 5	JCG# 6	KEN1# 0	KEN2# 0	KRI# 0	0
K#107	KCX#106	KCY#128	KCX#108	KCY# 9	KCB# 52	ICG# 5	JCG# 7	KEN1# 0	KEN2# 0	KRI# 0	0
K#108	KCX#107	KCY#109	KCX#12A	KCY#109	KCB# 59	ICG# 5	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K#109	KCX#128	KCY#112	KCX#110	KCY#108	KCB# 58	ICG# 4	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K#110	KCX#109	KCY#111	KCX#12A	KCY#128	KCB# 0	ICG# 4	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0	0
K#111	KCX#112	KCY#128	KCX#115	KCY#110	KCB# 0	ICG# 3	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0	0
K#112	KCX#128	KCY#113	KCX#111	KCY#109	KCB# 0	ICG# 3	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K#113	KCX#128	KCY#114	KCX#12A	KCY#112	KCY# 0	ICG# 2	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K#114	KCX#128	KCY#126	KCX#128	KCY#113	KCB# 0	ICG# -1	JCG# 8	KEN1# 6	KEN2# 0	KRI# 0	0
K#115	KCX#111	KCY#116	KCX#12A	KCY#128	KCB# 0	ICG# 3	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K#116	KCX#128	KCY#128	KCX#117	KCY#115	KCB# 0	ICG# 2	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0	0
K#117	KCX#116	KCY#128	KCX#11A	KCY#12A	KCB# 0	ICG# 2	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0	0
K#118	KCX#117	KCY#128	KCX#12A	KCY#12A	KCB# 0	ICG# -2	JCG# 12	KEN1# 7	KEN2# 0	KRI# 0	0
K#119	KCX# 9	KCY#108	KCX#12A	KCY#120	KCB# 60	ICG# 6	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0	0
K#120	KCX# 10	KCY#119	KCX#12A	KCY#127	KCB# 61	ICG# -7	JCG# 8	KEN1# 4	KEN2# 0	KRI# 0	0
K#121	KCX# 5	KCY# 7	KCX#12A	KCY#128	KCB# 0	ICG# -7	JCG# 5	KEN1# 3	KEN2# 0	KRI# 0	0

KC# 128



## APPENDIX F

### IDENTIFICATION OF GAGES FOR SABINE-CALCASIEU ASTROTIDE CALIBRATION

Gages for Sabine-Calcasieu astrotide calibration and time sequences of accepted astrotide simulation at those gages for 72 hours are identified. Also included are listings of the channel output at  $t = 30, 60,$  and 90 hours. For explanation of each column see Appendix C,7,b.

ASTRO TIDE CALIBRATION FOR SABINE-CALCASIEU AREA

SABINE PASS TIDES USED AS INPUT

PERIOD OF RECORD- 0000 AUG.22 TO 2400 AUG.26,1973

CALCULATIONS ALLOW FOR SUB-GRID SCALE CHANNELS AND BARRIERS

TIME SEQUENCES OF WATER LEVEL AND FLOW ARE SAVED FOR THE FOLLOWING PLACES-

GAGE 1 SABINE PASS, SOUTHWEST JETTY

GAGE 2 PORT ARTHUR, CE AREA OFFICE

GAGE 3 NORTH SABINE LAKE

GAGE 4 BEAUMONT, NECHES RIVER AND BRAKES BAYOU

GAGE 5 ORANGE NAVAL STATION, SABINE RIVER

GAGE 6 CAMERON, CALCASIEU PASS

GAGE 7 HACKBERRY, CALCASIEU RIVER AND PASS

GAGE 8 I.M.H. AT CALCASIEU LOCK, WEST

GAGE 9 LAKE CHARLES, CALCASIEU RIVER

FLOW 1 SABINE PASS INFLOW

FLOW 2 CALCASIEU PASS INFLOW

FLOW 3 FLOW TO NECHES RIVER FROM SABINE LAKE AND INTRACOASTAL WATERWAY

FLOW 4 EASTWARD FLOW VIA INTRACOASTAL CANAL JUST EAST OF SABINE RIVER

FLOW 5 FLOW TO SABINE RIVER FROM SABINE LAKE AND INTRACOASTAL WATERWAY

FLOW 6 FLOW TO CALCASIEU RIVER FROM CALCASIEU LAKE AND INTRACOASTAL W.

WATER LEVEL HYDROGRAPHS (FT) AND dV FLOWS (1000 CF)

HOUE	1	2	3	4	5	6	7	8	9	1	2	3	4	5	6
0.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1.0	.49	.00	.00	.07	.09	.33	-.01	.01	.02	79.97	18.72	.00	.01	-.01	-.15
2.0	.47	.05	.00	.04	.21	.36	.14	.03	.02	46.82	23.77	-1.23	.80	-1.92	-.15
3.0	.44	.14	.03	.06	.12	.42	.19	.11	.03	57.49	30.78	-.74	.53	-2.05	2.17
4.0	.42	.20	.18	.10	.11	.38	.24	.22	.14	62.72	33.49	.55	.29	-1.45	5.70
5.0	.37	.25	.25	.19	.17	.38	.28	.24	.30	60.82	30.52	1.35	.07	-.57	6.02
6.0	.35	.26	.27	.32	.29	.36	.32	.33	.37	55.42	26.94	1.88	.20	-.30	4.17
7.0	.32	.33	.33	.43	.39	.33	.36	.39	.42	42.43	23.21	.75	.14	-.69	2.63
8.0	.29	.38	.39	.47	.46	.31	.37	.42	.45	31.83	18.45	-.67	-.02	-1.01	-.98
9.0	.26	.39	.41	.48	.51	.28	.39	.45	.45	22.13	13.83	-1.03	.04	-1.17	-.05
10.0	.21	.40	.41	.42	.54	.25	.39	.43	.46	8.64	7.45	-2.01	.14	-1.36	-1.21
11.0	.18	.40	.41	.38	.53	.18	.36	.40	.43	-13.69	-1.02	-1.79	.24	-1.79	-2.53
12.0	.04	.37	.40	.36	.68	.10	.37	.38	.38	-35.07	-11.20	-1.52	.65	-2.06	-3.22
13.0	-.24	.31	.35	.35	.62	-.13	.25	.31	.34	-63.84	-27.77	-1.69	.79	-2.01	-4.20
14.0	-.58	.23	.27	.32	.36	-.42	.14	.23	.27	-93.03	-47.50	-2.32	.78	-2.08	-5.72
15.0	-.92	.12	.18	.25	.29	-.72	-.00	.11	.17	-114.74	-65.01	-3.52	1.09	-2.29	-7.98
16.0	-1.25	-.03	.06	.12	.19	-1.02	-.15	-.05	.02	-129.41	-79.12	-5.09	1.42	-2.58	-10.64
17.0	-1.58	-.17	-.09	-.07	.05	-1.31	-.29	-.20	-.18	-181.23	-89.37	-5.69	1.66	-2.86	-12.12
18.0	-1.61	-.35	-.24	-.27	-.10	-1.40	-.44	-.37	-.36	-137.71	-80.46	-5.90	1.83	-3.03	-12.47
19.0	-1.24	-.47	-.34	-.44	-.27	-1.19	-.56	-.59	-.55	-117.89	-76.95	-5.45	1.96	-3.07	-12.10
20.0	-.95	-.57	-.53	-.60	-.44	-.83	-.67	-.74	-.72	-95.99	-63.63	-5.38	2.11	-3.02	-10.36
21.0	-.51	-.64	-.64	-.74	-.60	-.59	-.67	-.74	-.63	-63.77	-42.83	-3.12	2.09	-2.87	-7.16
22.0	-.15	-.64	-.70	-.81	-.73	-.45	-.64	-.74	-.67	-49.03	-49.70	-1.50	1.93	-2.53	-3.09
23.0	.57	-.54	-.72	-.79	-.81	.32	-.58	-.75	-.68	57.34	24.11	.40	1.74	-2.05	1.00
24.0	.74	-.40	-.63	-.64	-.62	.55	-.31	-.57	-.73	108.51	54.93	2.84	1.53	-1.27	8.15
25.0	.90	-.24	-.41	-.47	-.73	.71	-.15	-.36	-.68	133.20	72.92	5.14	1.11	-.09	14.48
26.0	.97	-.09	-.24	-.16	-.49	.81	-.00	-.12	-.15	140.55	80.63	6.09	.72	1.33	17.46
27.0	.92	.02	-.10	.13	-.15	.80	.18	.10	.14	135.10	78.59	6.59	.59	1.69	16.25
28.0	.74	.16	.06	.30	.41	.68	.27	.29	.37	118.35	70.53	7.54	-1.07	1.21	12.65
29.0	.63	.27	.21	.43	.41	.60	.34	.43	.52	103.01	61.91	.19	-1.01	.48	8.77
30.0	.59	.35	.31	.44	.55	.57	.48	.57	.59	90.72	54.56	-1.23	-1.19	-.45	4.42
31.0	.56	.41	.39	.42	.64	.55	.58	.58	.62	77.36	47.37	-1.31	-1.30	-.69	1.57
32.0	.64	.46	.45	.41	.68	.62	.54	.59	.63	69.49	43.74	-.60	-1.16	-1.39	-.48
33.0	.74	.52	.52	.46	.67	.70	.58	.60	.61	47.43	42.31	-.43	-.81	-1.79	-1.31
34.0	.77	.57	.57	.55	.64	.74	.63	.62	.60	44.20	40.34	1.11	-.61	-1.62	-.75
35.0	.68	.63	.63	.65	.65	.68	.66	.65	.61	54.31	34.88	1.22	-.58	-1.34	-.59
36.0	.60	.64	.69	.74	.68	.63	.66	.67	.66	41.14	27.70	.64	-.45	-1.10	1.50
37.0	.64	.64	.73	.79	.74	.50	.66	.68	.70	20.35	16.22	-.46	-.22	-.99	1.16
38.0	.64	.69	.73	.84	.79	.18	.67	.68	.72	-25.89	-6.86	-1.60	-.03	-1.09	-.11
39.0	-.02	.63	.71	.75	.81	-.22	.69	.65	.71	-86.20	-36.66	-2.73	.17	-1.03	-2.51
40.0	-.48	.64	.61	.64	.79	-.63	.31	.52	.64	-130.84	-67.81	-4.22	.51	-1.94	-7.08
41.0	-1.53	.32	.43	.50	.70	-1.14	.14	.33	.45	-162.62	-95.66	-5.70	1.03	-2.74	-12.67
42.0	-1.71	.13	.26	.28	.51	-1.39	-.05	.11	.18	-166.83	-101.88	-6.51	1.71	-3.56	-16.62
43.0	-1.69	-.02	.09	.04	.27	-1.43	-.21	-.12	-.10	-159.69	-100.78	-6.94	2.16	-3.86	-17.69
44.0	-1.01	-.19	-.10	-.21	.02	-1.25	-.35	-.33	-.37	-141.85	-90.37	-6.07	2.30	-3.67	-16.17
45.0	-1.02	-.32	-.25	-.39	-.19	-.97	-.49	-.49	-.57	-118.83	-75.32	-6.76	2.46	-3.36	-12.82
46.0	-.49	-.40	-.38	-.52	-.38	-.55	-.52	-.62	-.70	-84.58	-53.32	-3.36	2.69	-3.06	-8.71
47.0	-.04	-.44	-.47	-.58	-.53	-.12	-.52	-.66	-.76	-60.92	-26.45	-1.90	2.45	-2.67	-4.23
48.0	.07	-.41	-.52	-.59	-.62	.27	-.44	-.63	-.75	15.30	5.78	-.45	2.32	-2.16	-.89
49.0	.79	-.31	-.48	-.52	-.66	.57	-.27	-.51	-.64	82.72	40.97	1.38	2.03	-1.49	6.55
50.0	.69	-.14	-.33	-.38	-.60	.72	-.04	-.31	-.44	117.75	61.79	3.49	1.70	-.54	12.30
51.0	.64	-.00	-.14	-.14	-.43	.75	.06	-.07	-.14	125.52	69.90	5.07	1.10	-.64	16.37
52.0	.77	.11	.01	.14	.14	.69	.19	.14	.17	119.14	68.04	4.89	.07	1.51	16.45
53.0	.62	.20	.14	.38	.18	.59	.29	.32	.41	104.62	60.50	3.33	-.59	1.41	12.99
54.0	.54	.29	.26	.51	.44	.55	.34	.45	.55	92.50	54.33	.69	-.65	.52	8.07
55.0	.64	.34	.35	.52	.60	.60	.45	.54	.61	74.28	45.26	-1.08	-.73	-.44	3.79
56.0	.64	.43	.42	.47	.64	.45	.50	.59	.63	61.48	37.97	-2.14	-.91	-1.08	-.94
57.0	.66	.47	.48	.42	.69	.47	.53	.58	.63	49.73	32.20	-1.59	-.86	-1.50	-.99
58.0	.69	.54	.51	.43	.68	.58	.59	.57	.60	46.61	30.48	-.39	-.59	-1.77	-2.39
59.0	.63	.54	.54	.49	.64	.61	.57	.57	.59	42.99	28.44	-.53	-.25	-1.89	-2.44
60.0	.53	.58	.57	.57	.60	.55	.59	.54	.58	32.09	22.96	.90	-.10	-1.76	-1.10
61.0	.53	.60	.62	.66	.60	.55	.58	.59	.57	25.56	19.52	.52	-.06	-1.62	-.19
62.0	.33	.62	.64	.71	.62	.39	.57	.59	.60	4.89	7.57	-.58	.10	-1.12	.31
63.0	.02	.60	.67	.70	.66	.15	.52	.58	.60	-34.09	-12.04	-1.75	.31	-1.18	-.70
64.0	-.31	.53	.60	.65	.68	-.15	.41	.54	.59	-79.16	-35.58	-2.92	.52	-1.51	-2.75
65.0	-1.06	.42	.50	.55	.66	-.75	.26	.42	.52	-133.89	-71.70	-4.20	.76	-1.93	-6.40
66.0	-1.46	.23	.35	.40	.57	-1.13	.04	.25	.34	-152.76	-90.80	-5.22	1.15	-2.96	-11.23
67.0	-1.70	.03	.19	.21	.42	-1.39	-.11	.04	.13	-161.49	-100.21	-6.21	1.65	-3.26	-15.32
68.0	-1.69	-.10	.00	.03	.20	-1.43	-.26	-.18	-.15	-155.55	-98.93	-6.51	2.07	-3.69	-16.95
69.0	-1.03	-.25	-.16	-.25	.04	-1.20	-.39	-.37	-.41	-138.90	-88.68	-5.98	2.35	-3.65	-15.97
70.0	-1.01	-.38	-.32	-.45	-.26	-.97	-.59	-.56	-.60	-113.96	-72.81	-4.95	2.35	-3.35	-12.43
71.0	-.51	-.47	-.44	-.54	-.43	-.57	-.56	-.65	-.73	-80.52	-51.98	-3.38	2.35	-2.99	-8.67

CHANNEL OUTPUT FOR HOUR 30

NTIME= 450

ALL M VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	MX	QX4	QXP	MY	QY4	QYP	MC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	08	1	.590	98718.	90903.	0.000	0.	0.	.518	0.	0.	0.	0.
2	8	2	.518	98985.	90852.	0.000	0.	0.	.445	0.	0.	0.	0.
3	8	3	.445	98652.	90518.	.375	-87013.	-40538.	.409	0.	0.	0.	3105.
4	7	3	0.000	0.	0.	0.000	0.	0.	.375	0.	0.	0.	0.
5	7	4	.375	87013.	87151.	.338	-21560.	-21809.	.344	0.	-809.	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.338	0.	0.	0.	0.
7	6	5	.338	21560.	19184.	.272	0.	0.	.334	-2172.	0.	0.	0.
8	6	6	.334	19184.	18953.	.294	-2163.	-2232.	.328	0.	0.	0.	0.
9	6	7	.328	16721.	18507.	.178	-7511.	-7577.	.319	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.319	5488.	5234.	.357	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.332	5234.	5033.	.346	0.	0.	0.	0.
12	8	8	.346	4033.	4844.	.314	0.	0.	.360	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.380	4844.	4665.	.373	0.	0.	0.	0.
14	9	9	.373	4665.	4493.	0.000	0.	0.	.386	0.	0.	0.	0.
15	9	10	.386	4493.	472.	0.000	0.	0.	.396	0.	3879.	0.	0.
16	9	11	.396	-1235.	-1296.	.405	1343.	1296.	.401	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.410	1375.	1343.	.405	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.410	0.	0.	0.	0.
19	7	12	.410	-1375.	-1395.	.418	1494.	1395.	.414	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.422	1494.	1464.	.414	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.422	0.	0.	0.	0.
22	5	13	.422	-1464.	-1482.	0.000	0.	0.	.425	0.	0.	0.	0.
23	5	14	.425	-1482.	-1394.	.432	1382.	1394.	.429	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.432	0.	0.	0.	0.
25	4	15	.432	-1392.	-1369.	0.000	0.	0.	.439	0.	0.	0.	0.
26	5	15	.429	0.	0.	.439	-1369.	-1368.	.442	0.	0.	0.	0.
27	5	16	.442	-1368.	-1324.	.447	1297.	1324.	.444	0.	0.	0.	0.
28	4	16	.439	0.	0.	0.000	0.	0.	.447	0.	0.	0.	0.
29	4	17	.447	-1297.	-1267.	.451	12.	84.	.449	0.	0.	0.	0.
30	5	17	.444	0.	0.	.449	-1223.	-1197.	.451	0.	0.	0.	0.
31	5	18	.451	-1197.	-1180.	.453	1130.	1180.	.452	0.	0.	0.	0.
32	4	18	.449	0.	0.	.452	0.	0.	.453	0.	0.	0.	0.
33	4	19	.453	-1130.	-1100.	0.000	0.	0.	.454	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.451	0.	0.	0.	0.
35	3	18	.451	-32.	-19.	.453	10.	19.	.452	0.	0.	0.	0.
36	2	18	0.000	0.	0.	.453	0.	10.	.453	0.	0.	0.	0.

WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)

HOUR	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
72.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
73.0	.04	-.50	-.54	-.04	-.58	-.12	-.55	-.04	-.80	-35.43	-23.90	-1.96	2.38	-2.63	-.14	-.14
74.0	.94	-.45	-.57	-.04	-.67	-.31	-.27	-.67	-.78	27.12	11.66	-.28	2.17	-2.17	-.88	-.88
75.0	.79	-.33	-.52	-.54	-.71	-.57	-.27	-.51	-.67	90.96	44.98	1.83	1.92	-1.48	6.87	6.87
76.0	.92	-.14	-.38	-.41	-.05	-.74	-.09	-.51	-.88	124.02	66.02	4.04	1.60	-.04	13.15	13.15
77.0	.94	-.02	-.17	-.15	-.05	-.80	-.05	-.08	-.15	131.81	74.06	5.51	.85	-.86	17.08	17.08
78.0	.84	.09	-.02	.15	-.14	-.79	.18	.14	.17	125.83	72.07	4.99	-.71	1.82	16.77	16.77
79.0	.88	.20	.13	.39	.19	-.61	.30	.32	.41	108.02	63.03	3.18	-.71	1.82	13.18	13.18
80.0	.88	.30	.26	.51	.45	-.45	.34	.46	.56	86.31	51.12	.52	-.78	.87	9.30	9.30
81.0	.85	.38	.35	.51	.60	-.45	.43	.58	.62	74.22	46.69	-1.81	-.69	-.82	3.88	3.88
82.0	.80	.51	.41	.60	.68	-.49	.49	.57	.63	65.23	39.88	-2.33	-1.01	-1.06	.21	.21
83.0	.69	.66	.44	.60	.70	-.57	.53	.54	.61	58.68	37.24	-1.77	-.89	-1.62	-1.79	-1.79
84.0	.69	.51	.50	.40	.68	-.66	.57	.58	.57	57.11	36.96	-.25	-.56	-1.98	-2.10	-2.10
85.0	.76	.56	.56	.47	.61	-.73	.60	.59	.56	58.34	36.99	1.12	-.33	-1.84	-1.18	-1.18
86.0	.81	.60	.61	.59	.60	-.78	.64	.61	.58	59.85	37.05	1.75	-.29	-1.41	-.65	-.65
87.0	.88	.66	.66	.72	.63	-.81	.68	.65	.62	58.95	36.55	1.40	-.23	-1.09	1.90	1.90
88.0	.93	.65	.72	.72	.69	-.84	.71	.68	.68	42.36	27.00	.33	-.09	-.92	2.73	2.73
89.0	.94	.75	.77	.84	.76	-.80	.69	.73	.76	9.84	9.88	-.73	.01	-.81	2.54	2.54
90.0	.90	.72	.79	.83	.83	-.87	.61	.73	.76	43.11	-18.38	-1.79	.06	-.80	-.67	-.67
91.0	.71	.81	.73	.78	.89	-.88	.67	.85	.77	-110.07	-94.85	-3.18	-.25	-1.24	-3.71	-3.71
92.0	-1.31	.85	.58	.66	.84	-.94	.77	.50	.64	-151.77	-86.82	-.92	.76	-2.24	-.80	-.80
93.0	-1.49	.80	.41	.47	.71	-1.19	.68	.24	.80	-183.02	-96.84	-.83	1.89	-3.58	-15.89	-15.89
94.0	-1.59	.11	.22	.21	.46	-1.31	.67	.85	.89	-183.83	-100.74	-7.06	1.98	-3.46	-18.49	-18.49
95.0	-1.44	-.04	.05	-.04	.20	-1.27	-.22	-.17	-.20	-151.49	-95.13	-.87	2.37	-3.93	-17.85	-17.85
96.0	-1.06	-.19	-.11	-.26	-.04	-.98	-.35	-.36	-.43	-127.23	-79.63	-.53	2.47	-3.55	-14.55	-14.55

CHANNEL REACH 3

37	10	10	0.000	0.	0.	.396	2106.	1266.	.390	0.	0.	-823.	0.
38	11	10	0.000	0.	0.	.390	1266.	348.	.416	0.	0.	-866.	0.
39	12	10	0.000	0.	0.	.416	348.	-924.	.467	0.	0.	-1211.	0.
40	12	11	.467	-924.	-1013.	.505	-355.	-443.	.494	0.	0.	0.	0.
41	13	11	0.000	0.	0.	.494	-1456.	-1542.	.511	0.	0.	0.	0.
42	14	11	0.000	0.	0.	.511	-1542.	-1027.	.527	0.	0.	0.	0.
43	14	12	.527	-452.	-535.	.543	616.	535.	.535	0.	0.	0.	0.
44	13	12	.511	0.	0.	0.000	0.	0.	.543	0.	0.	0.	0.
45	13	13	.543	-616.	-695.	0.000	0.	0.	.549	0.	0.	0.	0.
46	14	13	.535	0.	0.	.549	-695.	-772.	.559	0.	0.	0.	0.
47	14	14	.559	-772.	-962.	0.000	0.	0.	.557	0.	0.	0.	0.
48	15	14	0.000	0.	0.	.557	-962.	-1094.	.560	0.	0.	0.	0.
49	15	15	.560	-1094.	-1224.	.569	1224.	1224.	.562	0.	0.	0.	0.
50	14	15	.557	0.	0.	0.000	0.	0.	.568	0.	0.	0.	0.
51	14	16	.569	-1224.	-1368.	0.000	0.	0.	.573	0.	0.	0.	0.
52	14	17	.573	-1368.	-1439.	0.000	0.	0.	.586	0.	0.	0.	0.
53	14	18	.586	-1439.	-1470.	0.000	0.	0.	.752	0.	0.	0.	0.
54	-14	19	.752	-1470.	-1500.	0.000	0.	0.	.917	0.	0.	0.	0.

CHANNEL REACH 4

55	11	11	.416	0.	0.	0.000	0.	0.	.505	0.	0.	0.	0.
56	11	12	.505	355.	266.	.522	-178.	-266.	.515	0.	0.	0.	0.
57	10	12	0.000	0.	0.	0.000	0.	0.	.522	0.	0.	0.	0.
58	10	13	.522	178.	89.	.531	-89.	-99.	.526	0.	0.	0.	0.
59	9	13	0.000	0.	0.	0.000	0.	0.	.531	0.	0.	0.	0.
60	-9	14	.531	89.	0.	0.000	0.	0.	.532	0.	0.	0.	0.

CHANNEL REACH 5

61	15	11	0.000	0.	0.	.527	-1175.	-1304.	.534	0.	0.	0.	0.
62	16	11	0.000	0.	0.	.534	-1304.	-1434.	.538	0.	0.	0.	0.
63	17	11	0.000	0.	0.	.538	-1434.	-1564.	.540	0.	0.	0.	0.
64	18	11	0.000	0.	0.	.540	-1564.	-1693.	.540	0.	0.	0.	0.
65	19	11	.532	1947.	1821.	.540	-1693.	-1821.	.537	0.	0.	0.	0.
66	19	10	0.000	0.	0.	0.000	0.	0.	.532	0.	0.	0.	0.
67	20	10	0.000	0.	0.	.532	-1947.	-2067.	.526	0.	0.	0.	0.
68	21	10	0.000	0.	0.	.526	-2067.	-2180.	.518	0.	0.	0.	0.
69	22	10	0.000	0.	0.	.518	-2180.	-2243.	.509	0.	0.	0.	0.
70	23	10	.488	4749.	5677.	.509	-2243.	-2373.	.500	0.	0.	0.	0.

CHANNEL REACH 6

71	-22	1	.569	54557.	54640.	0.000	0.	0.	.574	0.	0.	0.	0.
72	23	1	0.000	0.	0.	.574	12749.	12863.	.565	0.	0.	0.	0.
73	23	2	.565	12863.	12668.	.544	-12863.	-12868.	.555	0.	0.	0.	0.
74	22	2	.574	41852.	41875.	0.000	0.	0.	.544	0.	0.	0.	0.
75	22	3	.544	54759.	54752.	0.000	0.	0.	.495	0.	0.	0.	0.
76	22	4	.495	54752.	41901.	0.000	0.	0.	.371	0.	13240.	0.	0.
77	22	5	.371	41901.	25348.	0.000	0.	0.	.372	-11303.	5237.	0.	0.
78	23	5	0.000	0.	0.	.372	25348.	20711.	.393	0.	0.	-4574.	0.
79	23	6	.393	20711.	20625.	0.000	0.	0.	.415	0.	0.	0.	0.
80	23	7	.415	20625.	20533.	0.000	0.	0.	.438	0.	0.	0.	0.
81	23	8	.438	20533.	20434.	0.000	0.	0.	.461	0.	0.	0.	0.
82	23	9	.461	20434.	20327.	0.000	0.	0.	.484	0.	0.	0.	0.
83	24	9	0.000	0.	0.	.484	14539.	2173.	.495	0.	0.	-12163.	0.
84	24	10	.495	2173.	1949.	.500	3304.	3027.	.504	0.	0.	0.	0.
85	24	11	.504	4023.	4210.	0.000	0.	0.	.524	0.	0.	0.	0.
86	24	12	.524	4210.	3994.	0.000	0.	0.	.542	0.	0.	0.	0.
87	24	13	.542	3994.	3750.	0.000	0.	0.	.554	0.	0.	0.	0.
88	25	13	0.000	0.	0.	.554	3750.	3532.	.566	0.	0.	0.	0.
89	25	14	.566	3532.	2353.	0.000	0.	0.	.581	0.	1115.	0.	0.
90	26	14	0.000	0.	0.	.581	2353.	-389.	.589	0.	0.	-2599.	0.
91	27	14	0.000	0.	0.	.589	-389.	-528.	.591	0.	0.	0.	0.
92	27	15	.591	-528.	-665.	0.000	0.	0.	.592	0.	0.	0.	0.
93	-28	15	0.000	0.	0.	.592	-665.	-660.	.592	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	0.000	0.	0.	.594	553.	468.	.523	0.	0.	0.	0.
95	26	10	.547	-245.	-377.	.523	468.	377.	.536	0.	0.	0.	0.
96	26	9	0.000	0.	0.	0.000	0.	0.	.547	0.	0.	0.	0.
97	27	9	.547	-377.	-190.	.547	285.	190.	.556	0.	0.	0.	0.
98	27	8	0.000	0.	0.	0.000	0.	0.	.561	0.	0.	0.	0.
99	-28	8	0.000	0.	0.	.561	190.	0.	.563	0.	0.	0.	0.



CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.255	0.	-75.	.258	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.254	-75.	-151.	.252	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.252	-151.	-230.	.248	0.	0.	0.	0.
103	3	5	.248	-230.	-310.	0.000	0.	0.	.244	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.244	-310.	-2015.	.251	0.	0.	0.	1651.
105	5	5	0.000	0.	0.	.251	-2015.	-2042.	.272	0.	0.	0.	0.
106	5	6	.272	-2042.	-2163.	0.000	0.	0.	.244	0.	0.	0.	0.
107	5	7	.244	0.	0.	0.000	0.	0.	.178	0.	0.	0.	0.
108	5	8	.178	7511.	3773.	.150	-240.	-3773.	.154	-3711.	0.	-3434.	0.
109	4	8	0.000	0.	0.	.143	0.	0.	.150	0.	0.	0.	0.
110	4	9	.150	240.	241.	.145	-208.	-241.	.147	0.	0.	0.	0.
111	3	9	.143	-77.	-105.	0.000	0.	0.	.145	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.143	-90.	-77.	.143	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.142	-25.	-90.	.143	0.	0.	0.	0.
114	-1	9	0.000	0.	0.	.142	0.	-25.	.142	0.	0.	0.	0.

CHANNEL REACH 9

115	3	10	.145	103.	74.	.142	-48.	-74.	.143	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.142	0.	0.	0.	0.
117	2	11	.142	48.	23.	0.000	0.	0.	.142	0.	0.	0.	0.
118	-2	12	.142	23.	0.	0.000	0.	0.	.141	0.	0.	0.	0.

CHANNEL REACH 10

119	6	8	.319	3441.	3383.	.154	0.	0.	.317	0.	0.	0.	0.
120	-7	8	.332	0.	0.	.317	3383.	0.	.314	0.	0.	0.	3354.

CHANNEL REACH 11

121	-7	9	.344	65342.	61083.	.334	0.	0.	.296	-3498.	0.	0.	0.
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VOLUME OF WATER ABOVE MSL = 1535.4 MILLIONS OF CU FT  
(THE SEWARD 20-S TUNNELS 2 ARE EXCLUDED)

CHANNEL OUTPUT FOR HOUR# 60

HYTIME# 900

ALL M VALUES IN FEET, ALL Q VALUES IN CFS

#	I	J	M	QM	QP	MY	QYM	QYP	MC	QMT	QMF	QYT	QYP
CHANNEL REACH 1													
1	-8	1	.530	32046.	32799.	0.000	0.	0.	.544	0.	0.	0.	0.
2	8	2	.544	32799.	33388.	0.000	0.	0.	.537	0.	0.	0.	0.
3	8	3	.537	33388.	33998.	.570	-35602.	-33998.	.569	0.	-0.	0.	-1108.
4	7	3	0.000	0.	0.	0.000	0.	0.	.578	0.	0.	0.	0.
5	7	4	.578	34602.	36317.	.589	-12523.	-12459.	.584	0.	-369.	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.589	0.	0.	0.	0.
7	6	5	.589	12523.	11319.	.501	0.	0.	.592	-1242.	0.	0.	0.
8	6	6	.592	11319.	11298.	.508	-2377.	-2348.	.588	0.	0.	0.	0.
9	6	7	.588	4900.	8842.	.488	-6672.	-6634.	.581	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.501	35.	-50.	.579	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.579	-90.	-154.	.577	0.	0.	0.	0.
12	8	8	.577	-154.	-272.	.584	0.	0.	.574	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.574	-272.	-404.	.572	0.	0.	0.	0.
14	9	9	.572	-404.	-548.	0.000	0.	0.	.569	0.	0.	0.	0.
15	9	10	.569	-548.	-394.	0.000	0.	0.	.568	0.	-313.	0.	0.
16	9	11	.568	904.	829.	.567	-745.	-829.	.567	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.567	-655.	-765.	.567	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.567	0.	0.	0.	0.
19	7	12	.567	655.	558.	.567	-496.	-558.	.567	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.568	-362.	-445.	.567	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.568	0.	0.	0.	0.
22	5	13	.568	362.	265.	0.000	0.	0.	.568	0.	0.	0.	0.
23	5	14	.568	265.	165.	.570	-63.	-165.	.569	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.570	0.	0.	0.	0.
25	4	15	.570	63.	-27.	0.000	0.	0.	.572	0.	0.	0.	0.
26	5	15	.569	0.	0.	.572	-27.	-149.	.572	0.	0.	0.	0.
27	5	16	.572	-149.	-272.	.574	397.	272.	.573	0.	0.	0.	0.
28	4	16	.572	0.	0.	0.000	0.	0.	.574	0.	0.	0.	0.
29	4	17	.574	-397.	-522.	.576	-114.	-161.	.575	0.	0.	0.	0.
30	5	17	.573	0.	0.	.575	-683.	-778.	.576	0.	0.	0.	0.
31	5	18	.576	-778.	-906.	.578	1003.	906.	.577	0.	0.	0.	0.
32	4	18	.575	0.	0.	.577	0.	0.	.578	0.	0.	0.	0.
33	-4	19	.578	-1003.	-1100.	0.000	0.	0.	.578	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.576	0.	0.	0.	0.
35	3	18	.576	110.	65.	.578	-33.	-65.	.577	0.	0.	0.	0.
36	-2	18	0.000	0.	0.	.578	0.	-33.	.578	0.	0.	0.	0.

CHANNEL REACH 3

37	10	10	0.000	0.	0.	.598	-1298.	-1319.	.582	0.	0.	9.	0.
38	11	10	0.000	0.	0.	.582	-1319.	-1499.	.598	0.	0.	-161.	0.
39	12	10	0.000	0.	0.	.598	-1499.	-1868.	.617	0.	0.	-368.	0.
40	12	11	.617	-1868.	-1870.	.609	0.	0.	.612	0.	0.	0.	0.
41	13	11	0.000	0.	0.	.612	-1862.	-1861.	.609	0.	0.	0.	0.
42	14	11	0.000	0.	0.	.609	-1861.	-1858.	.607	0.	0.	0.	0.
43	14	12	.607	-1759.	-1753.	.605	1744.	1753.	.606	0.	0.	0.	0.
44	13	12	.609	0.	0.	0.000	0.	0.	.605	0.	0.	0.	0.
45	13	13	.605	-1744.	-1731.	0.000	0.	0.	.605	0.	0.	0.	0.
46	14	13	.606	0.	0.	.605	-1731.	-1715.	.604	0.	0.	0.	0.
47	14	14	.604	-1715.	-1668.	0.000	0.	0.	.604	0.	0.	0.	0.
48	15	14	0.000	0.	0.	.604	-1668.	-1634.	.605	0.	0.	0.	0.
49	15	15	.605	-1634.	-1598.	.611	1574.	1598.	.606	0.	0.	0.	0.
50	14	15	.608	0.	0.	0.000	0.	0.	.611	0.	0.	0.	0.
51	14	16	.611	-1574.	-1550.	0.000	0.	0.	.616	0.	0.	0.	0.
52	14	17	.616	-1550.	-1524.	0.000	0.	0.	.630	0.	0.	0.	0.
53	14	18	.630	-1524.	-1512.	0.000	0.	0.	.606	0.	0.	0.	0.
54	-14	19	.606	-1512.	-1500.	0.000	0.	0.	.972	0.	0.	0.	0.

CHANNEL REACH 4

55	11	11	.598	0.	0.	0.000	0.	0.	.609	0.	0.	0.	0.
56	11	12	.609	-8.	-8.	.604	5.	0.	.606	0.	0.	0.	0.
57	10	12	0.000	0.	0.	0.000	0.	0.	.608	0.	0.	0.	0.
58	10	13	.604	-5.	-2.	.602	1.	2.	.603	0.	0.	0.	0.
59	9	13	0.000	0.	0.	0.000	0.	0.	.602	0.	0.	0.	0.
60	-9	14	.602	-1.	0.	0.000	0.	0.	.602	0.	0.	0.	0.

CHANNEL REACH 5

61	15	11	0.000	0.	0.	.607	-89.	-85.	.606	0.	0.	0.	0.
62	16	11	0.000	0.	0.	.606	-85.	-85.	.605	0.	0.	0.	0.
63	17	11	0.000	0.	0.	.605	-85.	-100.	.608	0.	0.	0.	0.
64	18	11	0.000	0.	0.	.604	-100.	-108.	.604	0.	0.	0.	0.
65	19	11	.602	130.	118.	.604	-108.	-118.	.603	0.	0.	0.	0.
66	19	10	0.000	0.	0.	0.000	0.	0.	.602	0.	0.	0.	0.
67	20	10	0.000	0.	0.	.602	-130.	-142.	.600	0.	0.	0.	0.
68	21	10	0.000	0.	0.	.600	-142.	-152.	.598	0.	0.	0.	0.
69	22	10	0.000	0.	0.	.598	-152.	-162.	.596	0.	0.	0.	0.
70	23	10	.595	571.	560.	.596	-162.	-172.	.593	0.	0.	0.	0.

CHANNEL REACH 6

71	-22	1	.530	27960.	23228.	0.000	0.	0.	.545	0.	0.	0.	0.
72	23	1	0.000	0.	0.	.545	5143.	5529.	.548	0.	0.	0.	0.
73	23	2	.544	4579.	5882.	.559	-8203.	-5882.	.553	0.	0.	0.	0.
74	22	2	.545	18045.	18201.	0.000	0.	0.	.559	0.	0.	0.	0.
75	22	3	.559	24604.	24605.	0.000	0.	0.	.570	0.	0.	0.	0.
76	27	4	.570	24605.	18355.	0.000	0.	0.	.565	0.	6515.	0.	0.
77	22	5	.565	18355.	7960.	0.000	0.	0.	.573	-226.	8225.	0.	0.
78	23	5	0.000	0.	0.	.573	7960.	5086.	.560	0.	0.	-2895.	0.
79	23	6	.580	4086.	5091.	0.000	0.	0.	.585	0.	0.	0.	0.
80	23	7	.584	4091.	5091.	0.000	0.	0.	.589	0.	0.	0.	0.
81	23	8	.589	4091.	5088.	0.000	0.	0.	.592	0.	0.	0.	0.
82	23	9	.592	4088.	5080.	0.000	0.	0.	.595	0.	0.	0.	0.
83	24	9	0.000	0.	0.	.595	4509.	-1042.	.595	0.	0.	-5598.	0.
84	24	10	.594	-1042.	-1116.	.593	389.	388.	.590	0.	0.	0.	0.
85	24	11	.590	-1100.	-1123.	0.000	0.	0.	.582	0.	0.	0.	0.
86	24	12	.582	-1123.	-1144.	0.000	0.	0.	.573	0.	0.	0.	0.
87	24	13	.573	-1144.	-1163.	0.000	0.	0.	.566	0.	0.	0.	0.
88	25	13	0.000	0.	0.	.566	-1163.	-1183.	.560	0.	0.	0.	0.
89	25	14	.560	-1183.	-1121.	0.000	0.	0.	.552	0.	-78.	0.	0.
90	26	14	0.000	0.	0.	.552	-1121.	-740.	.547	0.	0.	390.	0.
91	27	14	0.000	0.	0.	.547	-740.	-759.	.545	0.	0.	0.	0.
92	27	15	.545	-759.	-779.	0.000	0.	0.	.544	0.	0.	0.	0.
93	-28	15	0.000	0.	0.	.544	-779.	-800.	.548	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	0.000	0.	0.	.590	343.	325.	.592	0.	0.	0.	0.
95	26	10	.594	-234.	-228.	.592	325.	288.	.593	0.	0.	0.	0.
96	26	9	0.000	0.	0.	0.000	0.	0.	.594	0.	0.	0.	0.
97	27	9	.596	-86.	-166.	.594	-234.	166.	.595	0.	0.	0.	0.
98	27	8	0.000	0.	0.	0.000	0.	0.	.596	0.	0.	0.	0.
99	-28	8	0.000	0.	0.	.596	46.	0.	.596	0.	0.	0.	0.

CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.551	0.	.53.	.551	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.551	-.93.	-100.	.551	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.551	-100.	-150.	.551	0.	0.	0.	0.
103	3	5	.551	-150.	-205.	0.000	0.	0.	.551	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.551	-205.	-2305.	.553	0.	0.	0.	2073.
105	5	5	0.000	0.	0.	.553	-2305.	-2305.	.561	0.	0.	0.	0.
106	5	6	.501	-2305.	-2377.	0.000	0.	0.	.500	0.	0.	0.	0.
107	5	7	.500	0.	0.	0.000	0.	0.	.488	0.	0.	0.	0.
108	5	8	.488	4602.	3371.	.474	-244.	-3371.	.475	-3215.	0.	-3075.	0.
109	6	8	0.000	0.	0.	.475	0.	0.	.474	0.	0.	0.	0.
110	6	9	.474	254.	224.	.475	-190.	-224.	.474	0.	0.	0.	0.
111	3	9	.475	-73.	-98.	0.000	0.	0.	.475	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.475	-48.	-73.	.475	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.475	-24.	-48.	.475	0.	0.	0.	0.
114	-1	8	0.000.	0.	0.	.475	0.	-24.	.475	0.	0.	0.	0.

CHANNEL REACH 9

115	3	10	.475	98.	72.	.475	-.47.	-.72.	.475	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.475	0.	0.	0.	0.
117	2	11	.475	.47.	23.	0.000	0.	0.	.475	0.	0.	0.	0.
118	-2	12	.475	23.	0.	0.000	0.	0.	.475	0.	0.	0.	0.

CHANNEL REACH 10

119	6	8	.501	2172.	2135.	.475	0.	0.	.503	0.	0.	0.	0.
120	-7	8	.574	0.	0.	.503	2135.	0.	.504	0.	0.	0.	2121.

CHANNEL REACH 11

121	-7	5	.504	23050.	23003.	.502	0.	0.	.505	30.	0.	0.	0.
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VOLUME OF WATER ABOVE MBL = 3-37.0 MILLIONS OF CU FT  
(THE BEAARD RU-S THRU J# 2 ARE EXCLUDED)

CHANNEL OUTPUT FOR HOUR# 00

HTIME# 1350

ALL M VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	MX	QXM	QXP	MY	QYM	QYP	MC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	-8	1	-.710	-110067.	-106011.	0.000	0.	0.	-.811	0.	0.	0.	0.
2	0	2	-.411	-110011.	-102903.	0.000	0.	0.	-.131	0.	0.	0.	0.
3	0	3	-.131	-102903.	-100004.	.211	83805.	100004.	.040	0.	-0.	0.	-13912.
4	0	4	0.000	0.	0.	0.000	0.	0.	.211	0.	0.	0.	0.
5	7	4	.211	-83805.	-85742.	.410	11010.	12451.	.361	0.	4031.	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.410	0.	0.	0.	0.
7	6	5	.410	-11810.	-8459.	.617	0.	0.	.440	4763.	0.	0.	0.
8	6	6	.440	-8459.	-5977.	.500	40.	105.	.510	0.	0.	0.	0.
9	6	7	.510	-5792.	-5358.	.604	-3537.	-3442.	.500	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.500	-8673.	-8214.	.502	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.502	-8214.	-7809.	.613	0.	0.	0.	0.
12	8	8	.613	-7809.	-7405.	.604	0.	0.	.633	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.633	-7405.	-7002.	.653	0.	0.	0.	0.
14	9	9	.653	-7002.	-6603.	0.000	0.	0.	.671	0.	0.	0.	0.
15	9	10	.671	-6603.	-4292.	0.000	0.	0.	.685	0.	-1915.	0.	0.
16	9	11	.685	-3181.	-3010.	.710	2045.	3010.	.690	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.721	2087.	2045.	.710	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.721	0.	0.	0.	0.
19	7	12	.721	-2087.	-2535.	.740	2390.	2535.	.751	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.747	2249.	2390.	.740	0.	0.	0.	0.
21	6	12	0.000	0.	0.	0.000	0.	0.	.747	0.	0.	0.	0.
22	5	13	.747	-2249.	-2153.	0.000	0.	0.	.754	0.	0.	0.	0.
23	5	14	.754	-2153.	-2044.	.707	1939.	2044.	.761	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.767	0.	0.	0.	0.
25	4	15	.767	-1939.	-1858.	0.000	0.	0.	.770	0.	0.	0.	0.
26	5	15	.701	0.	0.	.779	-1858.	-1750.	.783	0.	0.	0.	0.
27	5	16	.783	-1750.	-1658.	.792	1565.	1658.	.787	0.	0.	0.	0.
28	4	16	.779	0.	0.	0.000	0.	0.	.792	0.	0.	0.	0.
29	4	17	.792	-1565.	-1470.	.798	71.	103.	.796	0.	0.	0.	0.
30	5	17	.787	0.	0.	.796	-1373.	-1309.	.790	0.	0.	0.	0.
31	5	18	.799	-1309.	-1225.	.802	1162.	1225.	.800	0.	0.	0.	0.
32	4	18	.796	0.	0.	.800	0.	0.	.802	0.	0.	0.	0.
33	-4	19	.802	-1162.	-1100.	0.000	0.	0.	.803	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.798	0.	0.	0.	0.
35	3	18	.798	-71.	-40.	.802	20.	40.	.800	0.	0.	0.	0.
36	-2	18	0.000	0.	0.	.802	0.	20.	.802	0.	0.	0.	0.

CHANNEL REACH 3

37	10	10	0.000	0.	0.	.665	-1111.	-836.	.722	0.	0.	204.	0.
38	11	10	0.200	0.	0.	.722	-836.	-860.	.753	0.	0.	-86.	0.
39	12	10	0.000	0.	0.	.753	-858.	-1149.	.793	0.	0.	-390.	0.
40	12	11	.744	-1149.	-1157.	.422	100.	127.	.814	0.	0.	0.	0.
41	13	11	0.000	0.	0.	.814	-1031.	-1045.	.824	0.	0.	0.	0.
42	13	11	0.000	0.	0.	.824	-1005.	-943.	.833	0.	0.	0.	0.
43	14	12	.433	-1235.	-1221.	.822	1214.	1221.	.848	0.	0.	0.	0.
44	13	12	.824	0.	0.	0.000	0.	0.	.862	0.	0.	0.	0.
45	13	13	.822	-1214.	-1220.	0.000	0.	0.	.875	0.	0.	0.	0.
46	14	13	.824	0.	0.	.875	-1270.	-1212.	.886	0.	0.	0.	0.
47	14	14	.824	-1232.	-1278.	0.000	0.	0.	.891	0.	0.	0.	0.
48	15	14	0.000	0.	0.	.891	-1278.	-1317.	.897	0.	0.	0.	0.
49	15	15	.897	-1317.	-1363.	.910	1362.	1363.	.902	0.	0.	0.	0.
50	14	15	.891	0.	0.	0.000	0.	0.	.910	0.	0.	0.	0.
51	14	16	.910	-1392.	-1426.	0.000	0.	0.	.918	0.	0.	0.	0.
52	14	17	.918	-1426.	-1462.	0.000	0.	0.	.932	0.	0.	0.	0.
53	14	18	.932	-1462.	-1481.	0.000	0.	0.	1.000	0.	0.	0.	0.
54	-14	19	1.000	-1481.	-1560.	0.000	0.	0.	1.239	0.	0.	0.	0.

CHANNEL REACH 4

55	11	11	.753	0.	0.	0.000	0.	0.	.822	0.	0.	0.	0.
56	11	12	.422	-1149.	-74.	.833	0.	74.	.828	0.	0.	0.	0.
57	10	12	0.000	0.	0.	0.000	0.	0.	.833	0.	0.	0.	0.
58	10	13	.433	-49.	-25.	.839	12.	25.	.836	0.	0.	0.	0.
59	9	13	0.000	0.	0.	0.000	0.	0.	.839	0.	0.	0.	0.
60	-9	14	.839	-12.	0.	0.000	0.	0.	.840	0.	0.	0.	0.

CHANNEL REACH 5

61	15	11	0.000	0.	0.	.833	292.	294.	.823	0.	0.	0.	0.
62	16	11	0.000	0.	0.	.823	294.	328.	.810	0.	0.	0.	0.
63	17	11	0.000	0.	0.	.810	328.	384.	.794	0.	0.	0.	0.
64	18	11	0.000	0.	0.	.794	384.	450.	.775	0.	0.	0.	0.
65	19	11	.730	-610.	-520.	.775	450.	520.	.754	0.	0.	0.	0.
66	19	10	0.000	0.	0.	0.000	0.	0.	.730	0.	0.	0.	0.
67	20	10	0.000	0.	0.	.730	610.	702.	.704	0.	0.	0.	0.
68	21	10	0.000	0.	0.	.704	702.	802.	.676	0.	0.	0.	0.
69	22	10	0.000	0.	0.	.676	802.	911.	.646	0.	0.	0.	0.
70	23	10	.587	-2249.	-2678.	.646	911.	1027.	.613	0.	0.	0.	0.

CHANNEL REACH 6

71	-22	1	-.710	-58807.	-53253.	0.000	0.	0.	-.512	0.	0.	0.	-0.
72	23	1	0.000	0.	0.	-.512	-13162.	-11233.	-.439	0.	0.	0.	0.
73	23	2	-.439	-11233.	-9330.	-.303	7512.	9330.	-.370	0.	0.	0.	0.
74	22	2	-.512	-48092.	-39165.	0.000	0.	0.	-.303	0.	0.	0.	0.
75	22	3	-.303	-48092.	-45377.	0.000	0.	0.	-.091	0.	0.	0.	0.
76	22	4	-.091	-45377.	-25130.	0.000	0.	0.	.161	0.	-14621.	0.	0.
77	22	5	.161	-24136.	-7846.	0.000	0.	0.	.268	-10942.	-27390.	0.	0.
78	23	5	0.000	0.	0.	.268	-7846.	-4313.	.342	0.	0.	3142.	0.
79	23	6	.342	-4313.	-4014.	0.000	0.	0.	.405	0.	0.	0.	0.
80	23	7	.405	-4014.	-3749.	0.000	0.	0.	.467	0.	0.	0.	0.
81	23	8	.467	-3749.	-3519.	0.000	0.	0.	.527	0.	0.	0.	0.
82	23	9	.527	-3519.	-3325.	0.000	0.	0.	.587	0.	0.	0.	0.
83	24	9	0.000	0.	0.	.587	-475.	-3615.	.609	0.	0.	-3491.	0.
84	24	10	.609	-3615.	-3292.	.613	-1451.	-1249.	.627	0.	0.	0.	0.
85	24	11	.627	-3292.	-3407.	0.000	0.	0.	.654	0.	0.	0.	0.
86	24	12	.654	-3407.	-3099.	0.000	0.	0.	.683	0.	0.	0.	0.
87	24	13	.683	-3099.	-2784.	0.000	0.	0.	.703	0.	0.	0.	0.
88	25	13	0.000	0.	0.	.703	-2784.	-2442.	.722	0.	0.	0.	0.
89	25	14	.722	-2442.	-1854.	0.000	0.	0.	.766	0.	-508.	0.	0.
90	26	14	0.000	0.	0.	.766	-1854.	-1369.	.759	0.	0.	287.	0.
91	27	14	0.000	0.	0.	.759	-1369.	-1179.	.764	0.	0.	0.	0.
92	27	15	.764	-1179.	-989.	0.000	0.	0.	.767	0.	0.	0.	0.
93	-28	15	0.000	0.	0.	.767	-989.	-800.	.768	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	0.000	0.	0.	.627	-831.	-710.	.644	0.	0.	0.	0.
95	26	10	.670	445.	582.	.644	-710.	-582.	.658	0.	0.	0.	0.
96	26	9	0.000	0.	0.	0.000	0.	0.	.670	0.	0.	0.	0.
97	27	9	.683	153.	302.	.670	-445.	-302.	.678	0.	0.	0.	0.
98	27	8	0.000	0.	0.	0.000	0.	0.	.683	0.	0.	0.	0.
99	-28	8	0.000	0.	0.	.683	-193.	0.	.685	0.	0.	0.	0.

CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.703	0.	69.	.702	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.702	69.	138.	.699	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.699	138.	208.	.695	0.	0.	0.	0.
103	3	5	.695	208.	280.	0.000	0.	0.	.688	0.	280.	0.	0.
104	4	5	0.000	0.	0.	.688	290.	-161.	.662	0.	0.	0.	.697.
105	5	5	0.000	0.	0.	.662	-161.	-68.	.617	0.	0.	0.	0.
106	5	6	.617	-68.	40.	0.000	0.	0.	.569	0.	0.	0.	0.
107	5	7	.569	0.	0.	0.000	0.	0.	.604	0.	0.	0.	0.
108	5	8	.604	3537.	1958.	.649	71.	-1958.	.635	-1643.	0.	-2068.	0.
109	4	8	0.000	0.	0.	.690	0.	0.	.649	0.	0.	0.	0.
110	4	9	.649	-71.	-50.	.662	65.	50.	.665	0.	0.	0.	0.
111	3	9	.665	16.	23.	0.000	0.	0.	.682	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.695	10.	16.	.690	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.698	5.	10.	.695	0.	0.	0.	0.
114	-1	8	0.000	0.	0.	.699	0.	5.	.698	0.	0.	0.	0.

CHANNEL REACH 9

115	3	10	.682	-22.	-14.	.695	8.	14.	.690	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.695	0.	0.	0.	0.
117	2	11	.695	-8.	-4.	0.000	0.	0.	.698	0.	0.	0.	0.
118	-2	12	.698	-8.	0.	0.000	0.	0.	.700	0.	0.	0.	0.

CHANNEL REACH 10

119	6	8	.569	-177.	3.	.635	0.	0.	.587	0.	0.	0.	0.
120	-7	8	.587	0.	0.	.587	3.	0.	.604	0.	0.	0.	.93.

CHANNEL REACH 11

121	-7	5	.301	-73291.	-61573.	.469	0.	0.	.566	10634.	0.	0.	0.
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VOLUME OF WATER ABOVE MSL @ 4120.4 MILLIONS OF CU FT  
(THE BEAARD RUNS THRU J# 2 ARE EXCLUDED)

## APPENDIX G

### IDENTIFICATION OF GAGES FOR SABINE-CALCASIEU HURRICANE CARLA VERIFICATION

Gages for Sabine-Calcasieu Hurricane Carla verification and time sequences of water level and flow at the identified gage for 60 hours are identified. Also included are listings of detailed channel output at 30 and 60 hours. For explanation of each column see Appendix C,7,b.

HURRICANE CARLA CALIBRATION FOR SABINE-CALCASIEU AREA

PERIOD OF RECORD- 0000 SEP 10 TO 0000 SEP 13, 1961

CALCULATIONS ALLOW FOR SUB-GRID SCALE CHANNELS AND BARRIERS

TIME SEQUENCES OF WATER LEVEL AND FLOW ARE SAVED FOR THE FOLLOWING PLACES-

GAGE 1 SABINE PASS, SOUTHWEST JETTY

GAGE 2 PORT ARTHUR, CE AREA OFFICE

GAGE 3 NORTH SABINE LAKE

GAGE 4 BEAUMONT, NECHES RIVER AND BRAMES BAYOU

GAGE 5 ORANGE NAVAL STATION, SABINE RIVER

GAGE 6 CAMERON, CALCASIEU PASS

GAGE 7 WEST END OF INTRACOASTAL WATERWAY

GAGE 8 SABINE PASS, COAST GUARD STATION

GAGE 9 LAKE CHARLES, CALCASIEU RIVER

FLOW 1 SABINE PASS INFLOW

FLOW 2 CALCASIEU PASS INFLOW

WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)

HOUR	1	2	3	4	5	6	7	8	9	1	2
0.0	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	0.00	0.00
1.0	4.83	3.04	3.16	3.27	3.25	4.49	3.55	4.54	3.04	103.46	92.63
2.0	5.17	3.42	3.15	3.30	3.24	4.50	3.75	4.41	3.14	144.22	128.51
3.0	5.50	3.54	3.59	3.32	3.21	4.67	3.84	4.84	2.97	132.54	171.44
4.0	5.73	3.65	3.80	3.41	3.19	4.79	3.94	5.03	2.86	160.98	199.86
5.0	5.97	3.74	3.95	3.53	3.17	4.93	4.04	5.20	2.89	177.44	216.70
6.0	6.20	3.91	4.14	3.60	3.20	5.08	4.19	5.43	2.83	179.64	233.63
7.0	6.13	4.09	4.36	3.83	3.20	5.09	4.34	5.51	2.90	169.05	235.17
8.0	6.07	4.33	4.57	3.96	3.22	5.03	4.53	5.57	2.84	154.33	233.57
9.0	6.00	4.43	4.73	4.12	3.24	4.97	4.70	5.59	2.91	143.54	231.81
10.0	5.93	4.64	4.92	4.24	3.31	4.91	4.90	5.65	2.94	128.34	229.76
11.0	5.87	5.02	5.07	4.44	3.39	4.84	4.97	5.04	2.92	111.24	225.84
12.0	5.80	5.01	5.21	4.62	3.44	4.79	5.30	5.72	2.94	90.74	223.02
13.0	5.93	5.14	5.34	4.82	3.62	4.81	5.33	5.85	2.92	79.31	229.31
14.0	6.07	5.23	5.43	4.99	3.77	4.86	5.52	6.00	2.89	64.62	239.04
15.0	6.20	5.34	5.56	5.11	3.92	4.93	5.71	6.15	2.85	55.37	248.26
16.0	6.30	5.45	5.69	5.14	4.02	4.95	5.87	6.27	2.80	46.64	257.14
17.0	6.40	5.54	5.83	5.23	4.15	4.95	5.94	6.37	2.79	42.11	269.44
18.0	6.50	5.64	5.95	5.30	4.32	4.99	6.05	6.47	2.79	42.56	280.16
19.0	6.40	5.72	6.04	5.36	4.44	4.93	6.16	6.44	2.82	34.14	282.00
20.0	6.30	5.80	6.19	5.42	4.54	4.85	6.24	6.44	2.91	23.24	280.14
21.0	6.20	5.85	6.24	5.47	4.77	4.74	6.37	6.40	3.09	3.39	278.90
22.0	6.13	5.90	6.30	5.57	4.94	4.64	6.44	6.37	3.14	-21.15	279.25
23.0	6.07	5.90	6.31	5.64	5.12	4.57	6.45	6.32	3.32	-42.14	284.54
24.0	6.00	5.92	6.30	5.71	5.23	4.51	6.44	6.20	3.44	-53.90	284.92
25.0	6.00	5.96	6.31	5.94	5.37	4.84	6.54	6.34	3.55	-27.87	293.20
26.0	6.00	6.04	6.40	6.14	5.63	5.00	6.81	6.70	3.64	16.03	312.52
27.0	7.20	6.35	6.03	6.17	5.89	5.24	7.07	7.07	3.82	74.94	330.24
28.0	7.13	6.56	6.42	6.32	6.04	5.36	7.24	7.14	3.94	93.92	328.14
29.0	7.07	6.73	6.94	6.57	6.08	5.35	7.40	7.16	4.10	99.49	322.74
30.0	7.00	6.84	6.94	6.67	6.20	5.33	7.57	7.10	4.14	92.34	317.63
31.0	6.90	6.93	7.01	7.22	6.42	5.15	7.67	6.90	4.24	62.47	309.45
32.0	6.80	6.97	7.04	7.34	6.63	4.94	7.71	6.70	4.34	21.24	278.01
33.0	5.80	6.94	7.02	7.47	6.77	4.73	7.74	6.44	4.48	-51.13	255.04
34.0	5.43	6.94	6.94	7.50	6.83	4.72	7.70	6.30	4.61	-93.79	246.04
35.0	6.07	7.00	6.92	7.65	6.83	4.79	7.64	6.54	4.64	-114.04	258.51
36.0	6.20	7.07	6.97	7.67	6.83	4.64	7.60	6.66	4.71	-120.99	249.57
37.0	6.33	7.11	7.05	7.71	6.89	4.94	7.59	6.74	4.72	-112.30	276.67
38.0	6.47	7.16	7.19	7.74	7.01	5.09	7.65	6.84	4.74	-94.54	246.54
39.0	6.60	7.32	7.34	7.90	7.23	5.14	7.71	6.90	4.84	-80.85	224.90
40.0	6.33	7.45	7.53	8.03	7.42	5.12	7.79	6.90	5.04	-90.35	290.04
41.0	6.07	7.54	7.60	8.17	7.51	5.03	7.92	6.81	5.24	-104.32	274.24
42.0	5.80	7.56	7.59	8.30	7.65	4.94	7.97	6.64	5.40	-141.29	247.21
43.0	5.30	7.57	7.59	8.42	7.74	4.74	8.00	6.47	5.44	-178.72	247.17
44.0	4.80	7.55	7.62	8.51	7.89	4.51	8.00	6.24	5.42	-223.71	229.47
45.0	4.30	7.52	7.62	8.61	7.96	4.24	7.94	6.07	5.41	-269.24	210.02
46.0	4.40	7.53	7.64	8.73	8.02	4.14	7.92	6.05	5.42	-243.10	208.72
47.0	4.50	7.55	7.61	8.84	8.06	4.13	7.82	6.11	5.42	-261.43	214.40
48.0	4.60	7.59	7.64	8.95	8.13	4.04	7.71	6.11	5.41	-240.30	221.50
49.0	4.87	7.62	7.73	9.07	8.24	4.22	7.57	6.23	5.41	-220.65	234.07
50.0	5.13	7.65	7.77	9.20	8.37	4.34	7.34	6.34	5.43	-190.00	255.91
51.0	5.44	7.70	7.87	9.31	8.56	4.56	7.21	6.44	5.47	-175.41	277.15
52.0	5.17	7.71	7.91	9.42	8.80	4.51	7.03	6.31	5.53	-142.30	274.94
53.0	4.93	7.69	7.91	9.52	8.82	4.43	6.94	6.14	5.63	-196.34	241.53
54.0	4.70	7.63	7.83	9.57	8.81	4.40	6.92	5.94	5.70	-210.66	242.74
55.0	4.20	7.57	7.80	9.53	8.72	4.24	6.84	5.82	5.79	-250.55	217.02
56.0	3.70	7.47	7.46	9.40	8.67	4.14	6.61	5.31	5.90	-247.80	177.37
57.0	3.20	7.34	7.39	9.27	8.69	3.93	6.25	5.05	5.13	-274.51	136.74
58.0	3.03	7.30	7.27	9.17	8.79	3.85	5.97	4.91	6.43	-264.63	102.29
59.0	2.87	7.14	7.04	9.06	8.85	3.91	5.84	4.74	6.69	-266.24	54.44



ALL H VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	HT	QVH	QXP	HT	QVH	QTP	HC	QKT	QIF	QVT	QVF
CHANNEL REACH 1													
1	-9	1	7.000	02300.	92785.	3.200	0.	0.	7.030	-25145.	-23044.	0.	0.
2	8	2	7.150	02795.	95512.	3.200	0.	0.	7.071	-26402.	-52865.	0.	0.
3	8	3	7.071	02612.	124170.	7.150	-115427.	-124170.	7.101	-52801.	-81003.	32920.	41853.
4	7	3	3.200	0.	0.	3.200	0.	0.	7.150	0.	0.	0.	0.
5	7	4	7.150	115427.	05525.	7.102	-81445.	-100131.	7.101	-55840.	-202.	-44157.	-24050.
6	6	4	3.200	0.	0.	3.200	0.	0.	7.102	0.	0.	0.	0.
7	6	5	7.102	81445.	70309.	7.107	0.	0.	7.135	-7279.	-36036.	0.	0.
8	6	6	7.135	70309.	67355.	7.107	-3909.	-7845.	7.071	-45607.	-44290.	10013.	19912.
9	6	7	7.071	50421.	63605.	7.090	0.	-3502.	6.950	-44478.	-44053.	30173.	38240.
10	7	7	3.200	0.	0.	0.950	-10847.	-15132.	6.900	0.	0.	0.	0.
11	6	7	3.200	0.	0.	6.910	-15132.	-15400.	6.857	0.	0.	0.	0.
12	6	8	0.857	-15400.	-15602.	3.390	0.	0.	6.877	0.	0.	0.	0.
13	9	8	3.200	0.	0.	6.877	-15602.	-10177.	6.832	0.	0.	0.	0.
14	9	9	0.832	-10177.	-10428.	3.200	0.	0.	6.847	0.	0.	0.	0.
15	9	10	0.847	-10428.	-7211.	3.200	0.	0.	6.803	0.	-9540.	0.	0.
16	9	11	0.803	-4290.	2074.	0.925	-8740.	-2074.	6.807	-92702.	-99306.	0.131.	91320.
17	8	11	3.200	0.	0.	6.954	-10485.	-8740.	6.925	0.	0.	0.	-5910.
18	7	11	3.200	0.	0.	3.200	0.	0.	6.954	0.	0.	0.	0.
19	7	12	6.954	10485.	10290.	6.909	-10910.	-10290.	6.974	-33927.	-37890.	25680.	25087.
20	6	12	3.200	0.	0.	6.907	-10035.	-10910.	6.900	0.	0.	0.	-920.
21	5	12	3.200	0.	0.	3.200	0.	0.	6.907	0.	0.	0.	0.
22	5	13	0.907	10035.	20304.	3.200	0.	0.	6.987	0.	-970.	0.	0.
23	5	14	6.987	20304.	22691.	6.901	-22057.	-22001.	6.976	-45761.	-60033.	30190.	38930.
24	4	14	3.200	0.	0.	3.200	0.	0.	6.981	0.	0.	0.	0.
25	4	15	6.981	22697.	20181.	3.200	0.	0.	6.874	0.	2270.	0.	0.
26	5	15	0.874	0.	0.	6.974	20181.	10747.	6.800	0.	0.	20302.	30322.
27	5	16	6.800	10747.	15432.	6.844	-10400.	-15432.	6.805	17510.	24070.	-92.	0.
28	4	16	6.874	0.	0.	3.200	0.	0.	6.844	0.	0.	0.	0.
29	4	17	6.844	10940.	024.	6.803	-312.	-560.	6.800	-14055.	0.	0.	0.
30	5	17	0.803	0.	0.	6.840	175.	-130.	6.811	0.	0.	0.	0.
31	5	18	6.811	-130.	-539.	6.839	0.	539.	6.810	0.	0.	0.	0.
32	4	18	0.840	0.	0.	6.800	0.	0.	6.830	0.	0.	0.	0.
33	-4	19	6.830	-800.	-1152.	3.200	0.	0.	6.847	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	19	3.200	0.	0.	3.200	0.	0.	6.863	0.	0.	0.	0.
35	3	18	0.863	312.	174.	0.892	-85.	-174.	6.846	0.	0.	0.	0.
36	-2	18	3.200	0.	0.	6.923	0.	-85.	6.807	0.	0.	0.	0.
CHANNEL REACH 3													
37	10	10	3.200	0.	0.	6.803	-2915.	-200.	6.795	0.	0.	53030.	51051.
38	11	10	3.200	0.	0.	6.795	-200.	2100.	6.720	0.	0.	24097.	22353.
39	12	10	3.200	0.	0.	6.720	2100.	4322.	6.617	0.	0.	39701.	37510.
40	12	11	6.617	4322.	6070.	6.721	5033.	8700.	6.618	-23017.	-25233.	-9157.	-12671.
41	13	11	3.200	0.	0.	6.618	10820.	10003.	6.430	0.	0.	20771.	20430.
42	14	11	3.200	0.	0.	6.430	10003.	17004.	6.721	0.	0.	00091.	97000.
43	14	12	0.721	-1300.	-1719.	6.725	653.	1700.	6.227	85475.	85711.	17005.	16320.
44	13	12	6.430	0.	0.	3.200	0.	0.	6.235	0.	0.	0.	0.
45	13	13	6.235	-643.	-252.	3.200	0.	0.	6.230	0.	-510.	0.	0.
46	14	13	6.222	0.	0.	6.230	-252.	200.	6.204	0.	0.	0.	0.
47	14	14	6.204	200.	-1000.	3.200	0.	0.	6.222	0.	1012.	0.	0.
48	15	14	3.200	0.	0.	6.222	-1000.	-2105.	6.191	0.	0.	10270.	10285.
49	15	15	6.191	-2105.	-1245.	6.235	930.	1205.	6.207	1101.	0.	470.	0.
50	14	15	6.222	0.	0.	3.200	0.	0.	6.235	0.	0.	0.	0.
51	14	16	6.235	-930.	-1113.	3.200	0.	0.	6.245	0.	0.	0.	0.
52	14	17	6.245	-1113.	-1325.	3.200	0.	0.	6.250	0.	0.	0.	0.
53	14	18	6.250	-1325.	-1440.	3.200	0.	0.	6.300	0.	0.	0.	0.
54	-14	19	6.300	-1440.	-1500.	3.200	0.	0.	6.373	0.	0.	0.	0.
CHANNEL REACH 4													
55	11	11	6.720	0.	0.	3.200	0.	0.	6.721	0.	0.	0.	0.
56	11	12	6.721	-6033.	-6000.	6.700	172.	0.	6.750	0.	-4400.	301.	0.
57	10	12	3.200	0.	0.	3.200	0.	0.	6.780	0.	0.	0.	0.
58	10	13	6.780	-172.	-82.	6.820	30.	82.	6.801	0.	0.	0.	0.
59	9	13	3.200	0.	0.	3.200	0.	0.	6.820	0.	0.	0.	0.
60	-9	14	6.820	-30.	0.	3.200	0.	0.	6.840	0.	0.	0.	0.
CHANNEL REACH 5													
61	15	11	3.200	0.	0.	6.221	10010.	17022.	5.953	0.	0.	0.	1290.
62	16	11	3.200	0.	0.	5.953	17022.	15000.	5.692	0.	0.	0.	1711.
63	17	11	3.200	0.	0.	5.692	15000.	15000.	5.437	0.	0.	0.	0.
64	18	11	3.200	0.	0.	5.437	15000.	15512.	5.182	0.	0.	0.	0.
65	19	11	4.720	-12400.	-15305.	5.182	15512.	15305.	4.927	0.	2942.	0.	0.
66	19	10	3.200	0.	0.	3.200	0.	0.	4.720	0.	0.	0.	0.
67	20	10	3.200	0.	0.	4.720	12400.	12400.	4.500	0.	0.	0.	117.
68	21	10	3.200	0.	0.	4.500	12400.	4017.	4.410	0.	0.	0.	7710.
69	22	10	3.200	0.	0.	4.410	4017.	4511.	4.327	0.	0.	0.	0.
70	23	10	4.102	4300.	5173.	4.327	4511.	7700.	4.200	0.	0.	0.	-3000.

CHANNEL REACH 6

71	-22	1	7.000	317027.	135032.	3.200	0.	0.	6.002	-10111.	-32523.	0.	0.
72	23	1	3.200	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
73	23	2	5.334	00023.	45244.	5.135	114307.	99823.	5.330	0.	0.	32677.	47784.
74	22	2	0.002	221275.	181244.	3.200	0.	0.	5.072	0250.	07210.	25334.	138703.
75	22	3	5.135	171010.	130749.	3.200	0.	0.	4.135	0.	0.	0.	0.
76	22	4	4.704	134700.	91254.	3.200	0.	0.	4.704	0.	0.	0.	0.
77	22	5	4.314	01404.	02018.	3.200	0.	0.	4.314	-27070.	19793.	0.	0.
78	23	5	3.200	0.	0.	0.	0.	0.	4.205	-121662.	-02300.	0.	0.
79	23	6	4.214	53052.	40600.	3.200	0.	0.	4.214	0.	0.	91177.	100122.
80	23	7	4.205	01400.	01430.	3.200	0.	0.	4.205	-0737.	0.	0.	0.
81	23	8	4.201	01030.	01314.	3.200	0.	0.	4.201	-5303.	0.	0.	0.
82	23	9	4.199	01310.	31210.	3.200	0.	0.	4.199	0.	0.	0.	0.
83	24	9	3.200	0.	0.	0.	0.	0.	4.192	-0460.	0.	0.	0.
84	24	10	4.104	10303.	11740.	4.200	12911.	18303.	4.104	0.	0.	-10730.	00813.
85	24	11	4.104	0333.	0403.	3.200	0.	0.	4.104	-34337.	-27670.	23952.	27297.
86	24	12	4.103	0003.	1307.	3.200	0.	0.	4.103	0.	0.	0.	0.
87	24	13	4.200	13307.	12907.	3.200	0.	0.	4.200	7710.	0.	0.	0.
88	25	13	3.200	0.	0.	0.	0.	0.	4.225	0.	0.	0.	0.
89	25	14	4.204	12004.	10000.	3.200	0.	0.	4.204	0.	0.	1807.	0.
90	26	14	3.200	0.	0.	0.	0.	0.	4.220	10608.	6910.	0.	0.
91	27	14	3.200	0.	0.	0.	0.	0.	4.190	0.	0.	-3520.	0.
92	27	15	4.154	-002.	-003.	3.200	0.	0.	4.180	0.	0.	-7162.	0.
93	-28	15	3.200	0.	0.	0.	0.	0.	4.155	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	3.200	0.	0.	0.	0.	0.	4.104	14000.	13275.	3.920	0.	0.	-1345.	0.
95	26	10	3.000	-0003.	-11320.	3.920	13275.	11320.	3.730	-11071.	-10231.	-1907.	0.	0.	0.	0.
96	26	9	3.200	0.	0.	0.	0.	0.	3.600	0.	0.	0.	0.	0.	0.	0.
97	27	9	3.450	-200.	-5035.	3.000	0.	0.	3.500	-5100.	0.	0.	13302.	17400.	0.	0.
98	27	8	3.200	0.	0.	0.	0.	0.	3.450	0.	0.	0.	0.	0.	0.	0.
99	-28	8	3.200	0.	0.	0.	0.	0.	3.307	0.	0.	0.	0.	0.	0.	0.

CHANNEL REACH 8

100	-1	4	3.200	0.	0.	0.	0.	0.	4713.	7.573	0.	0.	10051.	13905.	0.	0.	
101	2	4	3.200	0.	0.	0.	0.	0.	4713.	0901.	7.407	0.	0.	24550.	25200.	0.	0.
102	3	4	3.200	0.	0.	0.	0.	0.	7.407	05010.	5027.	7.357	0.	0.	25000.	25005.	
103	3	5	7.352	5027.	4021.	3.200	0.	0.	7.355	-25270.	-24300.	0.	0.	0.	0.	0.	
104	4	5	3.200	0.	0.	0.	0.	0.	7.355	4021.	3301.	7.209	0.	0.	22011.	23700.	
105	5	5	3.200	0.	0.	0.	0.	0.	7.209	3301.	-200.	7.107	0.	0.	32012.	30003.	
106	5	6	7.107	-200.	-3900.	3.200	0.	0.	7.107	-21252.	-17000.	0.	0.	0.	0.	0.	
107	5	7	7.107	0.	0.	0.	0.	0.	7.000	0.	0.	0.	0.	0.	0.	0.	
108	5	8	7.000	-002.	-5207.	7.200	0.	0.	7.153	20070.	30510.	0.	0.	0.	0.	0.	
109	4	8	3.200	0.	0.	0.	0.	0.	7.200	0.	0.	0.	0.	0.	0.	0.	
110	4	9	7.200	-0007.	-30010.	7.412	0.	0.	7.305	-32003.	-34002.	0.	0.	0.	0.	0.	
111	3	9	7.400	3507.	0915.	3.200	0.	0.	7.412	-07020.	-05530.	0.	0.	0.	0.	0.	
112	3	8	3.200	0.	0.	0.	0.	0.	7.400	220.	3557.	7.450	0.	0.	-35101.	-30030.	
113	2	8	3.200	0.	0.	0.	0.	0.	7.732	270.	220.	7.500	0.	0.	0.	0.	
114	-1	8	3.200	0.	0.	0.	0.	0.	7.001	0.	270.	7.732	0.	0.	5077.	5700.	

CHANNEL REACH 9

115	3	10	7.412	0072.	10000.	7.200	-5073.	-10000.	7.375	0.	-3003.	0.	0.	4150.	0.	0.
116	2	10	3.200	0.	0.	0.	0.	0.	7.200	0.	0.	0.	0.	0.	0.	0.
117	2	11	7.200	0073.	2157.	3.200	0.	0.	7.250	0.	3000.	0.	0.	0.	0.	0.
118	-2	12	7.250	2157.	0.	3.200	0.	0.	7.200	0.	2002.	0.	0.	0.	0.	0.

CHANNEL REACH 10

119	6	8	6.000	70150.	00077.	7.153	0.	0.	6.020	12000.	20123.	0.	0.	0.	0.	0.
120	-7	8	0.000	0.	0.	0.003	00077.	70001.	3.300	0.	0.	0.	0.	3720.	-07070.	0.

CHANNEL REACH 11

121	-7	5	7.101	-30000.	-53070.	7.135	0.	0.	7.230	-00070.	-75015.	0.	0.	0.	0.	0.
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VOLUME OF WATER ABOVE SL = 102254.0 MILLIONS OF CU FT  
(T-1 STATION AND R-15 THROUGH 2 ARE EXCLUDED)

ALL H VALUES IN FEET, ALL G VALUES IN CFS

K	I	J	HE	G24	GAP	NY	G24	G2P	PC	G2T	G2F	G2T	G2F
CHANNEL REACH 1													
1	-8	1	2.700	-277231.	-276172.	3.200	0.	0.	3.501	0.	0.	0.	0.
2	8	2	3.511	-376172.	-375206.	3.200	0.	0.	4.223	0.	0.	0.	0.
3	8	3	4.223	-475206.	-474537.	5.502	303734.	318517.	4.714	0.	45900.	0.	-15033.
4	7	3	3.200	0.	0.	3.200	0.	0.	4.062	0.	0.	0.	0.
5	7	4	5.733	-117952.	-117910.	5.733	117962.	127117.	5.500	27000.	23900.	24313.	15000.
6	6	4	3.200	0.	0.	3.200	0.	0.	5.733	0.	0.	0.	0.
7	6	5	5.733	-117952.	-117910.	6.330	0.	0.	6.024	28900.	20412.	0.	0.
8	6	6	6.024	-117952.	-117910.	6.000	4356.	12370.	6.423	39551.	3081.	-5385.	-13573.
9	6	7	6.423	-117952.	-117910.	7.273	21370.	25057.	6.925	47027.	52300.	-13725.	-17461.
10	7	7	3.200	0.	0.	6.925	-30614.	-30000.	6.971	0.	0.	0.	0.
11	8	7	3.200	0.	0.	6.971	-30098.	-29033.	7.013	0.	0.	0.	0.
12	8	8	7.013	-29033.	-29177.	7.000	0.	0.	7.000	0.	0.	0.	0.
13	8	9	3.200	0.	0.	7.000	-29177.	-28708.	7.120	0.	0.	0.	0.
14	8	10	3.200	0.	0.	3.200	0.	0.	7.180	0.	0.	0.	0.
15	9	10	7.180	-28708.	-28223.	3.200	0.	0.	7.200	0.	0.	0.	0.
16	9	11	7.200	-28223.	-28050.	7.774	40861.	46150.	7.400	190213.	191724.	-71043.	-72000.
17	8	11	3.200	0.	0.	7.774	43023.	44861.	7.627	0.	0.	0.	-1670.
18	7	11	3.200	0.	0.	3.200	0.	0.	7.770	0.	0.	0.	0.
19	7	12	7.770	-43023.	-43137.	6.992	41731.	43137.	7.957	33941.	30291.	-29236.	-30488.
20	6	12	3.200	0.	0.	6.992	42201.	41731.	8.092	0.	0.	0.	0.
21	5	12	3.200	0.	0.	3.200	0.	0.	8.191	0.	0.	0.	0.
22	5	13	8.191	-42201.	-41917.	3.200	0.	0.	8.310	0.	-293.	0.	0.
23	5	14	8.310	-41917.	-40610.	6.513	37275.	40010.	8.430	44351.	42500.	-39092.	-41098.
24	4	14	3.200	0.	0.	3.200	0.	0.	8.513	0.	0.	0.	0.
25	4	15	8.513	-37275.	-36230.	8.010	0.	0.	8.619	0.	-960.	0.	0.
26	5	15	8.619	-36230.	-36050.	8.010	-36200.	-37260.	8.902	0.	0.	-68802.	-67580.
27	5	16	8.902	-36050.	-36050.	9.190	36010.	36050.	9.055	-25000.	-25190.	-35003.	-36100.
28	4	16	8.910	0.	0.	3.200	0.	0.	9.190	0.	0.	0.	0.
29	4	17	9.190	-36010.	-36740.	9.447	50000.	8221.	9.360	7441.	8000.	0.	-3200.
30	5	17	9.005	0.	0.	9.360	-24527.	-25022.	9.495	0.	0.	-20998.	-23500.
31	5	18	9.495	-24527.	-16770.	9.685	11582.	18770.	9.570	6904.	0.	3805.	-3257.
32	4	18	9.300	0.	0.	9.670	0.	0.	9.605	0.	0.	0.	0.
33	-8	19	9.605	-11582.	-1812.	3.200	0.	0.	9.652	4927.	-4720.	0.	0.
CHANNEL REACH 2													
34	3	17	3.200	0.	0.	3.200	0.	0.	9.407	0.	0.	0.	0.
35	3	18	9.407	-4000.	-2513.	9.405	70.	2513.	9.476	12650.	10235.	-1780.	-4181.
36	-2	18	3.200	0.	0.	9.409	0.	70.	9.465	0.	0.	0.	0.
CHANNEL REACH 3													
37	10	10	3.200	0.	0.	7.200	-215.	-3040.	7.207	0.	0.	-162320.	-159300.
38	11	10	3.200	0.	0.	7.207	-3040.	-6470.	7.300	0.	0.	-145500.	-141907.
39	12	10	3.200	0.	0.	7.300	-6470.	-10325.	7.600	0.	0.	12054.	16321.
40	12	11	7.600	-10325.	-13600.	7.400	11813.	9015.	7.702	-29432.	-25050.	13707.	15093.
41	13	11	3.200	0.	0.	7.702	-9200.	-5030.	7.672	0.	0.	8481.	10050.
42	14	11	3.200	0.	0.	7.672	-5030.	-6000.	7.951	0.	0.	-95100.	-93700.
43	14	12	7.951	-16500.	-10300.	8.310	17062.	16300.	8.101	-20000.	-20900.	-19415.	-16013.
44	13	12	7.872	0.	0.	3.200	0.	0.	8.310	0.	0.	0.	0.
45	13	13	8.310	-17062.	-17630.	3.200	0.	0.	8.527	0.	0.	0.	0.
46	14	13	8.101	0.	0.	8.527	-17630.	-18012.	8.721	0.	0.	36300.	37310.
47	14	14	8.721	-18012.	-18770.	3.200	0.	0.	8.830	44231.	44700.	0.	0.
48	15	14	3.200	0.	0.	8.830	-18770.	-17002.	8.963	0.	0.	-53785.	-54500.
49	15	15	8.963	-17002.	-15000.	9.200	14570.	15000.	9.111	-26370.	-26300.	-19700.	-20701.
50	14	15	8.830	0.	0.	3.200	0.	0.	9.200	0.	0.	0.	0.
51	14	16	9.200	-14570.	-13770.	3.200	0.	0.	9.590	0.	0.	-1070.	0.
52	14	17	9.590	-13770.	-8131.	3.200	0.	0.	9.572	0.	0.	-3710.	0.
53	14	18	9.472	-8131.	-2872.	3.200	0.	0.	9.930	0.	0.	-2750.	0.
54	-14	19	9.930	-2872.	-2000.	3.200	0.	0.	10.620	0.	0.	0.	0.
CHANNEL REACH 4													
55	11	11	7.300	0.	0.	3.200	0.	0.	7.600	0.	0.	0.	0.
56	11	12	7.600	-11813.	-12407.	7.600	8970.	12407.	7.822	314.	2201.	-4352.	-5257.
57	10	12	3.200	0.	0.	3.200	0.	0.	7.780	0.	0.	0.	0.
58	10	13	7.780	-4070.	-600.	8.070	39.	60.	7.900	0.	-7001.	0.	0.
59	9	13	3.200	0.	0.	3.200	0.	0.	8.070	0.	0.	0.	0.
60	-9	14	8.070	-39.	0.	3.200	0.	0.	8.100	0.	0.	0.	0.
CHANNEL REACH 5													
61	15	11	3.200	0.	0.	7.951	9688.	12720.	7.968	0.	0.	0.	-2920.
62	16	11	3.200	0.	0.	7.968	12720.	17038.	7.677	0.	0.	0.	-5000.
63	17	11	3.200	0.	0.	7.777	17038.	21112.	7.720	0.	0.	-17330.	-20577.
64	18	11	3.200	0.	0.	7.720	21112.	27935.	7.463	0.	0.	-10400.	-23000.
65	19	11	6.711	-10001.	-20000.	7.603	27935.	28000.	7.093	0.	9100.	0.	0.
66	19	10	3.200	0.	0.	3.200	0.	0.	7.111	0.	0.	0.	0.
67	20	10	3.200	0.	0.	6.711	19001.	14542.	6.550	0.	0.	0.	5122.
68	21	10	3.200	0.	0.	6.550	14542.	12030.	6.000	0.	0.	0.	2013.
69	22	10	3.200	0.	0.	6.000	12030.	12000.	6.310	0.	0.	0.	-11.
70	23	10	6.000	-30000.	-30750.	6.310	12000.	15070.	6.150	0.	0.	0.	-2853.

CHANNEL REACH 6

71	-22	1	3.475	-16093.	-15020.	3.200	0.	0.	3.856	0.	0.	0.	0.
72	23	1	3.200	0.	0.	3.456	33800.	-253.	0.007	0.	0.	0.	30910.
73	23	2	4.007	-2453.	-28350.	4.196	49708.	20350.	4.182	32354.	60717.	4972.	20711.
74	27	2	3.456	0.0000.	-64333.	3.200	0.	0.	4.196	0.	20408.	0.	0.
75	22	3	4.196	-13701.	-17587.	3.200	0.	0.	4.534	0.	-4476.	0.	0.
76	22	4	4.534	-14070.	-16507.	3.200	0.	0.	4.071	5752.	1513.	0.	0.
77	22	5	5.071	-14070.	-16507.	3.200	0.	0.	5.200	130440.	137361.	0.	0.
78	23	4	3.200	0.	0.	5.200	-15501.	-105477.	5.400	0.	0.	-103730.	-103290.
79	23	6	5.400	-10000.	-10000.	3.200	0.	0.	4.028	-415.	0.	0.	0.
80	23	7	5.400	-10000.	-10000.	3.200	0.	0.	4.000	5720.	0.	0.	0.
81	23	8	5.400	-10000.	-10000.	3.200	0.	0.	5.953	14151.	0.	0.	0.
82	23	9	5.953	-60700.	-60700.	3.200	0.	0.	6.000	21081.	0.	0.	0.
83	24	0	3.200	0.	0.	0.000	-25001.	-25000.	0.112	0.	0.	-117002.	-118253.
84	24	10	4.112	-25205.	-19000.	0.159	-20203.	-13077.	0.100	-70100.	-75321.	-65700.	-50227.
85	24	11	0.100	-21455.	-7600.	3.200	0.	0.	0.201	-34402.	-47002.	0.	0.
86	24	12	0.201	-9800.	14907.	3.200	0.	0.	0.300	22855.	0.	0.	0.
87	24	13	0.300	10907.	15121.	3.200	0.	0.	0.457	0.	0.	0.	0.
88	25	13	3.200	0.	0.	0.457	15121.	15260.	0.510	0.	0.	0.	0.
89	25	14	0.510	15260.	13100.	3.200	0.	0.	0.622	0.	2100.	0.	0.
90	26	10	3.200	0.	0.	0.622	13100.	0.000.	0.670	0.	0.	-7210.	0.
91	27	10	3.200	0.	0.	0.670	0.000.	-1312.	0.700	0.	0.	-7400.	0.
92	27	15	0.700	-1312.	-1310.	3.200	0.	0.	0.750	0.	0.	0.	0.
93	-20	15	3.200	0.	0.	0.750	-1310.	-1310.	0.770	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	3.200	0.	0.	0.100	-12200.	-4707.	0.275	0.	0.	29720.	20205.
95	26	10	0.200	1240.	4076.	0.275	-4707.	-4170.	0.300	-00305.	-00303.	-3020.	-0250.
96	26	0	3.200	0.	0.	3.200	0.	0.	0.207	0.	0.	0.	0.
97	27	0	0.271	103.	-130.	0.257	-1200.	130.	0.300	-351.	0.	40003.	45335.
98	27	0	3.200	0.	0.	3.200	0.	0.	0.271	0.	0.	0.	0.
99	-20	0	3.200	0.	0.	0.271	-103.	0.	0.205	0.	0.	0.	0.

CHANNEL REACH 8

100	-1	4	3.200	0.	0.	5.751	0.	-5407.	5.002	0.	0.	-5551.	-950.
101	2	4	3.200	0.	0.	5.002	-5407.	-9010.	5.001	0.	0.	-0305.	-4700.
102	3	4	3.200	0.	0.	5.001	-9010.	-11435.	5.002	0.	0.	-11431.	-9011.
103	3	5	5.002	-10935.	-4703.	3.200	0.	0.	0.173	14330.	13270.	0.	0.
104	4	5	3.200	0.	0.	0.173	-9703.	-6408.	0.205	0.	0.	-10300.	-21073.
105	5	4	3.200	0.	0.	0.205	-6408.	-2200.	0.330	0.	0.	-07002.	-51030.
106	5	4	0.330	-2200.	0.350.	3.200	0.	0.	0.400	31000.	25302.	0.	0.
107	5	7	0.400	0.	0.	3.200	0.	0.	7.293	0.	0.	0.	0.
108	5	8	7.293	-21320.	-18000.	7.000	10077.	10005.	7.305	-13001.	-10030.	-00501.	-09705.
109	6	4	3.200	0.	0.	7.000	0.	0.	7.000	0.	0.	0.	0.
110	6	0	7.000	-10077.	-13001.	7.000	12000.	13001.	7.000	41371.	40000.	-40013.	-40000.
111	3	0	7.000	-10077.	-3700.	3.200	0.	0.	7.000	07007.	00205.	0.	0.
112	3	0	3.200	0.	0.	7.000	10077.	-10072.	7.000	0.	0.	40001.	53500.
113	2	0	3.200	0.	0.	7.000	12000.	18000.	7.000	0.	0.	15.	-4007.
114	-1	0	3.200	0.	0.	7.000	0.	1221.	7.000	0.	0.	18.	-10000.

CHANNEL REACH 9

115	3	10	7.000	-10077.	-10700.	0.000	11100.	10700.	0.522	550.	2070.	0.	-7070.
116	2	10	3.200	0.	0.	3.200	0.	0.	0.000	0.	0.	0.	0.
117	2	11	0.000	-11100.	-0372.	3.200	0.	0.	0.007	9900.	5130.	0.	0.
118	-2	12	0.007	-0372.	0.	3.200	0.	0.	0.002	0.	-0300.	0.	0.

CHANNEL REACH 10

119	6	0	0.000	-0300.	-30075.	7.305	0.	0.	7.171	-31300.	-42017.	0.	0.
120	6	0	0.001	0.	0.	7.171	-30075.	-33130.	7.000	0.	0.	-00500.	-00032.

CHANNEL REACH 11

121	6	5	5.500	-15000.	-15000.	0.000	0.	0.	5.700	13702.	10001.	0.	0.
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VOLUME OF WATER AVAILABLE IN MILLIONS OF CU FT  
(THE SUB-AREA CODE 1-11 IS NOT BE EXCLUDED)

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